

ABBREVIATIONS ANCHOR BOLT ABOVE ADJACENT ABOVE FINISH FLOC GGR AGGREGATE T ALTERNATE UM ALUMINUM PROX APPROXIMATELY BOARD BUILDING K'G BLOCKING BEAM BOUNDARY NAILIN BOTTOM OF DTT BOTTOM 6 BEARING WN BETWEEN AM CAMBER ANT'L CANTILEVERED R CENTERED CENTERLINE R CLEAR MU CONC MASONRY UN OL COLUMN ONC CONCRETE ONN CONNECTION ONST CONSTRUCTION PENNY DIA DIAMETER DOUBLE PT DEPARTMEN DOUGLAS FIR LARCH DIAGONA DETAIL DIAMETER DIMENSION DOWN VG DRAWING VL DOWEL EACH EXPANSION JOINT EV ELEVATION C ELECTRIC EDGE NAILING VCL ENCLOSED R ENGINEER OF RECOR EQUAL 2019 EQUIPMENT IST EXISTING P EXPANSION T EXTERIOR TD EXTENDED FLOOR DRAIN ID/EDN FOUNDATION FINISH FLOOR FINISH GRADE FINISH R FLOOR RMG FRAMING FOOT OR FEE FOOTING FURRING GAUGE _B GLU-LAM BEAM RND GROUND GIRDER TRUSS GYPSUM WALLBOA HEADER HANGER HT HEIGH ORIZ HORIZONTAL R HIP TRUSS NSULATION INTERIOR LAMINATE T LATERAL POUNDS LAM STRAND LUMBER LIGHT LAM VENEER LUMBER LONG WAY AS MASONRY AX MAXIMUM ECH MECHANICAL FR MANUFACTURER MANUFACTURE MINIMUM MISCELLANEOUS NUMBER COMMON NAIL S NOT TO SCALE OVER ON CENTER OPPOSITE HANE OPENING FA POST FROM ABOVE PLATE YWD PLYWOOD L PANEL REFAB PREFABRICATED PARALLEL STRAND LBS PER SQ INCH POST-TENSIONED POUND RAD RADIUS REFERENCE REINFORCING O'D REQUIRED SINKER NAIL CHED SCHEDULE SQUARE FEET CT SECTION SHEATHING SIMILAR SPECIFICATIONS SQUARE D STANDARD G'D STAGGERED STEEL STRUCTURAL M SYMMETRICAL MI SHEAR WALL PN SOLE PLATE NAILING SHORT WAY G TONGUE AND GROOV B TOP AND BOTTOM THICK RIMM TRIMMER TRUSS TUBE STEEL LSL TIMBERSTRAND BEAM TYPICAL INO UNLESS NOTED C VERTICAL

/ W/O WITH WITHOUT

WOOD SCREW

WEIGHT

GENERAL

- ALL WORK SHALL CONFORM TO THE MINIMUM STANDARDS OF THE FOLLOWING CODE: THE INTERNATIONAL BUILDING AND RESIDENTIAL CODE, 2018 EDITION, OTHER REGULATING AGENCIES WHICH HAVE AUTHORITY OVER ANY PORTION OF THE WORK, AND THOSE CODES AND STANDARDS LISTED IN THESE NOTES AND SPECIFICATIONS.
- 2. THE STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING, SHORING FOR LOADS DUE TO CONSTRUCTION EQUIPMENT, ETC. OBSERVATION VISITS TO THE SITE BY THE STRUCTURAL ENGINEER SHALL NOT INCLUDE INSPECTIONS OF THE ABOVE ITEMS.
- THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS PRIOR TO STARTING CONSTRUCTION. THE ARCHITECT SHALL BE NOTIFIED OF ANY DISCREPANCIES OR INCONSISTENCIES.
- 4. DIMENSIONS SHALL TAKE PRECEDENCE OVER THE SCALE SHOWN ON DRAWINGS.
- NOTES AND DETAILS ON PLANS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS.
- 6. SEE ARCHITECTURAL PLANS FOR THE FOLLOWING UNO:
- SIZE AND LOCATION OF ALL DOOR AND WINDOW OPENINGS SIZE AND LOCATION OF ALL INTERIOR AND EXTERIOR NON-BEARING PARTITIONS.
- SIZE AND LOCATION OF ALL CONCRETE CURBS, FLOOR DRAINS, SLOPES, DEPRESSED AREAS, CHANGES IN LEVEL, CHAMFER, GROOVES, INSERTS, ETC.
- SIZE AND LOCATION OF FLOOR AND ROOF OPENINGS.
- FLOOR AND ROOF FINISHES.
- STAIR FRAMING AND DETAILS. G. DIMENSIONS NOT SHOWN ON STRUCTURAL PLANS.
- 7. SEE MECHANICAL, PLUMBING, AND ELECTRICAL PLANS FOR THE FOLLOWING: A. PIPE RUNS, SLEEVES, HANGERS, TRENCHES, WALL AND SLAB OPENINGS, ETC., (EXCEPT AS SHOWN OR NOTED).
 - ELECTRICAL CONDUIT RUNS, BOXES, OUTLETS IN WALLS AND SLABS.
 - CONCRETE INSERTS FOR ELECTRICAL, MECHANICAL, OR PLUMBING FIXTURES. SIZE AND LOCATION OF MACHINE EQUIPMENT BASES, OR ANCHOR BOLTS FOR MOUNTS. SIZE AND LOCATION OF ALL MECHANICAL UNITS.
- OPENINGS, POCKETS, ETC. LARGER THAN 6 INCHES SHALL NOT BE PLACED IN SLABS, DECKS, BEAMS, JOISTS, COLUMNS, WALLS, ETC., UNLESS SPECIFICALLY DETAILED ON THE STRUCTURAL PLANS.
- 9. ASTM SPECIFICATIONS NOTED SHALL BE THE LATEST REVISION
- 10. THE CONTRACTOR SHALL INVESTIGATE THE SITE DURING CLEARING AND EARTHWORK OPERATIONS FOR FILLED EXCAVATIONS OR BURIED STRUCTURES SUCH AS CESSPOOLS, CISTERNS, FOUNDATIONS, ETC. IF ANY SUCH STRUCTURES ARE FOUND, THE ENGINEER OF RECORD SHALL BE NOTIFIED IMMEDIATELY.
- 11. CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON FLOORS OR ROOF. LOAD SHALL NOT EXCEED THE DESIGN LIVE LOAD PER SQUARE FOOT. PROVIDE ADEQUATE SHORING AND/OR BRACING WHERE STRUCTURE HAS NOT ATTAINED DESIGN STRENGTH.
- 12. WHERE THE LONGEST HORIZONTAL CEILING DIMENSION IS EQUAL TO OR GREATER THAN 20'-0", IT IS RECOMMENDED THAT RESILIENT CHANNEL BE USED TO HELP LIMIT DRYWALL CRACKING.

DESIGN CRITERIA

	LIVE LOAD	DEAD LOAI	C			
FLOOR LOAD	40 PSF	15 PSF				
ROOF LOAD	20 PSF	22 PSF				
ROOF SNOW LOAD	10 PSF	22 PSF				
STAIR & EXIT LOAD						
STORAGE LOAD						
* SEE FRAMING NOTES FOR SPE	ECIAL LOADING CONDITIONS.					
BASIC WIND SPEED			115 MPH			
WIND RISK CATEGORY			II			
WIND EXPOSURE CATEGORY	(С			
INTERNAL PRESSURE COEFFICIENT			±0.18			
SEISMIC RISK CATEGORY & IMPORTANCE		Π	I _E = 1	.0		
MAPPED SPECTRAL RESPONSE ACCELERATIONS	S _S	S _S = 0.498 & S ₁ = 0.162				
SPECTRAL RESPONSE COEF	FICIENTS	S _{DS} =	= 0.432 & S _{D1} = 0.1	62		
SITE CLASS			С			
LATERAL FORCE RESISTING	SYSTEM	LIGHT FRAME SHEAR P	WALLS WITH ANELS	R = 6.5		
SEISMIC RESPONSE COEFFI	CIENT, C _S		0.066			
DESIGN BASE SHEAR		Cs	x DEAD WEIGHT(V	V)		
ANALYSIS PROCEDURE		EQUIVA	LENT LATERAL FO	DRCE		
SEISMIC DESIGN CATEGORY			С			

FOUNDATION

- 1. THIS IS TO CERTIFY THE FOUNDATION DEPICTED HEREIN HAS BEEN DESIGNED IN ACCORDANCE WITH RECOGNIZED ENGINEERING PRACTICE FOR CONDITIONS AS CLASSIFIED BY THE PROJECT GEOTECHNICAL REPORT:
 - CONSULTANT: DUPONT ENGINEERING, INC PROJECT NO: 18-0414 DATED: SEPTEMBER 29, 2018
 - UPDATED: MARCH 5, 2020
- 2. FOOTINGS ARE DESIGNED BASED ON AN ALLOWABLE BEARING PRESSURE OF 2000 PSF PER SOIL REPORT
- SOILS PREPARATION AND FOUNDATION CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF THE GEOTECHNICAL REPORT
- 4. SEE ARCHITECTURAL AND STRUCTURAL DRAWINGS FOR EXACT LOCATION OF BULKHEADS AND OPENINGS, ETC.
- 5. THE CONTRACTOR SHALL PROVIDE FOR PROPER DE-WATERING OF EXCAVATIONS FROM SURFACE WATER, GROUND WATER, SEEPAGE, ETC. (FOOTINGS SHALL NOT BE PLACED UNDER WATER).
- FOOTINGS SHALL BE PLACED ACCORDING TO DEPTHS SHOWN ON THE STRUCTURAL PLANS. ALL ABANDONED FOOTINGS, UTILITIES, ETC. THAT INTERFERE WITH NEW CONSTRUCTION SHALL BE REMOVED.
- 7. CONCRETE PLACEMENT SHALL BE IN ONE CONTINUOUS OPERATION UNLESS OTHERWISE SPECIFIED AND SLAB SURFACE SHALL BE CURED WITH HUNTS COMPOUND OR EQUAL.
- 8. FOOTING BACKFILL AND UTILITY TRENCH BACKFILL WITHIN BUILDING AREA SHALL BE PER THE REQUIREMENTS OF THE GEOTECHNICAL REPORT. FLOODING WILL NOT BE PERMITTED.
- STOOPS, PORCHES, OR OTHER ATTACHMENTS SHALL BE CAST INDEPENDENT OF THE CONCRETE FOUNDATION SLAB, UNO.

FOUNDATION HARDWARE

- 1. THICKEN SLAB AS REQUIRED FOR CONCRETE COVERAGE AT ANCHOR BOLTS PER ACI 318 WHERE OCCURS. THE CONCRETE CONTRACTOR SHALL VERIFY LOCATION OF ALL BOLTS, TIE-DOWNS, POST-ANCHORS, ETC. WITH THE ARCHITECTURAL PLANS PRIOR TO COMMENCING WORK AND BE RESPONSIBLE FOR SAME.
- 2. UNO BOLT SILL PLATES TO THE FOUNDATIONS WITH MIN 1/2" NOMINAL DIA ANCHOR BOLT WITH PLATE WASHERS. BOLTS SHALL BE SPACED NOT MORE THAN 72" O.C. THERE SHALL BE A MIN OF (2) BOLTS PER PIECE. ONE BOLT SHALL BE LOCATED NOT MORE THAN 12" AND NOT LESS THAN 4" FROM EACH END, OR FROM EACH SIDE OF A NOTCH GREATER THAN 1/2 THE WIDTH OF THE PLATE. EMBED BOLTS AT LEAST 7" INTO REINFORCED MASONRY OR CONCRETE. SILL PLATE ANCHOR BOLTS MAY BE "WET SFT"
- 3. PROVIDE 3" X 3" X 0.229" PLATE WASHERS ON ALL ANCHOR BOLTS AT SHEAR WALLS. PLATE WASHERS SHALL EXTEND TO WITHIN 1/2" OF THE EDGE OF THE SILL PLATE ON THE SIDE(S) WITH SHEATHING. PROVIDE STANDARD CUT WASHERS UNDER BOLT HEADS AND NUTS WHEN SLOTTED PLATE WASHERS ARE USED. PROVIDE CUT OR SLOTTED WASHERS AGAINST WOOD AT ALL REMAINING WALLS.
- 4. ALL 1/2" DIA. ANCHOR BOLTS MAY BE REPLACED WITH: A. 1/2" DIA THREADED ROD EPOXY DOWELED TO FOUNDATION WITH SIMPSON SET-XP EPOXY (ICC REPORT ESR-2508) IN 5/8" DIA HOLE. PROVIDE A MIN. OF 4" EMBEDMENT AND 1 3/4" EDGE DISTANCE FROM ANY SLAB EDGE
 - B. 1/2" DIA SIMPSON TITEN HD HIGH STRENGTH THREADED ANCHOR (ICC ESR-2713). PROVIDE A MIN. OF 4" EMBEDMENT AND 1 3/4" EDGE DISTANCE FROM ANY SLAB EDGE.
 - C. SIMPSON MASA/MASAP MUDSILL ANCHORS (ICC ESR-2555). INSTALLATION OF THE MUDSILL ANCHOR SHALL BE PER MANUFACTURER'S TYPICAL OR ALTERNATE INSTALLATION METHOD. DO NOT WET-SET THE MUDSILL ANCHOR, INSTALL WITH ONE LEG UP OR INSTALL OVER PLYWOOD OR OSB. FOLLOW ALL MANUFACTURER'S SPECIFICATIONS AND RECOMMENDATIONS DURING INSTALLATION. DO NOT US WHEN ANCHOR SPACING IS < 9 INCHES.
- 5. AT INTERIOR NON-BEARING AND NON-SHEARWALLS, 1/2" DIA ANCHOR BOLTS MAY BE REPLACED WITH MINIMUM 0.145" DIA X 2 7/8" POWDER DRIVEN FASTENERS AT A MAXIMUM SPACING OF 24" O.C. (HILTI ESR-2379 OR SIMPSON ESR-2138 OR EQUIVALENT)
- 6. PROVIDE REINFORCING BAR FOR HOLDOWNS PER DETAILS SHEET SD-1, UNO.
- HOLDOWNS SPECIFIED ON PLANS MAY BE POST INSTALLED (RETRO-FITTED) AS NOTED BELOW. DRILL COMPLETELY THRU THE FOOTING AND PROVIDE THREADED ROD WITH BEARING PLATE, WASHER AND NUT. BACKFILL ALL AROUND THE ROD, WASHER AND NUT TO PROVIDE A MINIMUM OF 3" COVER USING LEAN CONCRETE MIX. THREADED ROD AND BEARING PLATE SHALL BE PER THE TABLE PROVIDED. SEE RETROFIT DETAIL SHEET SD-1.1. THE USE OF POST INSTALLED ANCHORS UNLESS NOTED OTHERWISE E.G. EXPANSION ANCHORS OR EPOXY ANCHORS, IS NOT ALLOWED WITHOUT PRIOR REVIEW AND AUTHORIZATION BY THE ENGINEER OF RECORD. CONTACT THE ENGINEER OF RECORD FOR REQUEST OF ALTERNATE ANCHORAGE SOLUTIONS.

REINFORCING STEEI

- 1. REINFORCING BARS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A-615 GRADE 60.
- 2. WELDED REINFORCING BARS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A-706 GRADE 60.
- 3. ALL REINFORCING BAR BENDS SHALL BE MADE COLD.
- 4. WELDED WIRE REINFORCEMENT (WWR) SHALL BE PER ASTM A185 AND SHALL BE SUPPLIED IN SHEETS. WWR FRM ROLLS SHALL NOT BE USED.
- 5. MINIMUM LAP OF WELDED WIRE FABRIC SHALL BE 6 INCHES OR ONE AND ONE HALF SQUARES, WHICHEVER IS GREATER.
- 6. ALL BARS SHALL BE MARKED SO THEIR IDENTIFICATION CAN BE MADE WHEN THE FINAL IN-PLACE INSPECTION IS MADE.
- 7. REBAR SPLICES ARE TO BE CLASS "B" (UNO). MAINTAIN 2 BAR DIA CLEAR SPACE BETWEEN ADJACENT SPLICES.
- 8. REINFORCING SPLICES SHALL BE MADE ONLY WHERE INDICATED ON THE DRAWINGS.
- 9. DOWELS BETWEEN FOOTINGS AND WALLS OR COLUMNS SHALL BE THE SAME GRADE, SIZE AND SPACING OR NUMBER AS THE VERTICAL REINFORCING, RESPECTIVELY, UNO.

	REBAR SIZE	3	4	5	6	7	8	9	10	11
	MINIMUM LAP LENGTH (INCHES)	22	29	36	43	68	78	88	98	107
	STD HOOK LENGTH (INCHES)	4 1/2	6	7 1/2	9	10 1/2	12	13 1/2	15 1/2	16 1/2
•	LAP LENGTH	*			*		<u>-</u>			

STRUCTURAL GENERAL NOTES

CONCRETE

- 1. ALL PHASES OF WORK PERTAINING TO THE CONCRETE CONSTRUCTION SHALL CONFORM TO THE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" (ACI 318 LATEST ADOPTED EDITION), WITH MODIFICATIONS AS NOTED IN THE DRAWINGS AND SPECIFICATIONS.
- 2. REINFORCED CONCRETE DESIGN IS BY THE "ULTIMATE STRENGTH DESIGN METHOD". SCHEDULE OF STRUCTURAL CONCRETE 28-DAY STRENGTHS AND TYPES:
- LOCATION IN STRUCTURE STRENGTH 2500 PSI SLABS ON GRADE FOOTINGS 2500 PSI



- DESIGN BASED ON 2500 PSI, 28-DAY STRENGTH, THEREFORE SPECIAL INSPECTION IS NOT REQUIRED.
- 4. CONCRETE MIX DESIGN SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR APPROVAL WITH THE FOLLOWING REQUIREMENTS:
- A. COMPRESSIVE STRENGTH AT AGE 28 DAYS AS SPECIFIED ABOVE.
- B. LARGE AGGREGATE-HARDROCK: 3/4" MAXIMUM SIZE CONFORMING TO ASTM C-33. CEMENT: ASTM C-150, TYPE V PORTLAND CEMENT.
- MAXIMUM SLUMP: 5 INCHES. NO ADMIXTURES, EXCEPT FOR ENTRAINED AIR, AND AS APPROVED BY THE ENGINEER. F. WATER/CEMENT RATIO SHALL BE AS FOLLOWS:
- a. fc = 4500 PSI.....w/c = 0.45
- b. f'c = 4000 PSI.....w/c = 0.5 c. fc < 4000 PSI....w/c = PER MIX DESIGN
- G. FLY ASH: ASTM C618
- H. SLAG: ASTM C989 I. SILICA FUME: ASTM C1240
- LIGHTWEIGHT AGGREGATE: ASTM C330
- AIR ENTRAINING ADMIXTURE: ASTMC260 L. WATER REDUCERS: ASTM C494, TYPE A OR F
- 5. CONCRETE MIXING OPERATIONS, ETC, SHALL CONFORM TO ASTM C-94.
- 6. PLACEMENT OF CONCRETE SHALL CONFORM TO ACI STANDARD 614 AND PROJECT SPECIFICATIONS.
- 7. CLEAR COVERAGE OF CONCRETE OVER OUTER REINFORCING BARS SHALL BE AS FOLLOWS: A. CONCRETE POURED DIRECTLY AGAINST EARTH: 3 INCHES CLEAR B. STRUCTURAL SLABS: 3/4 INCHES CLEAR, TOP AND BOTTOM (2" TOP; 3/4" BOTTOM IN CORROSIVE ENVIRONMENTS)
 - C. FORMED CONCRETE WITH EARTH BACKFILL: 2 INCHES CLEAR
- 8. ALL REINFORCING BARS, HOLD DOWN BOLTS AND STRAPS, AND OTHER CONCRETE INSERTS SHALL BE WELL SECURED IN POSITION PRIOR TO PLACING CONCRETE.
- 9. PROVIDE SLEEVES FOR PLUMBING AND ELECTRICAL OPENINGS IN CONCRETE BEFORE PLACING. DO NOT CUT ANY REINFORCING WHICH MAY CONFLICT. CORING IN CONCRETE IS NOT PERMITTED EXCEPT AS SHOWN. NOTIFY THE STRUCTURAL ENGINEER IN ADVANCE OF CONDITIONS NOT SHOWN ON THE DRAWINGS.
- 10. CONDUIT OR PIPE SIZE (O.D.) SHALL NOT EXCEED 30% OF SLAB THICKNESS AND SHALL BE PLACED BETWEEN THE TOP AND BOTTOM REINFORCING, UNLESS SPECIFICALLY DETAILED OTHERWISE. CONCENTRATIONS OF CONDUITS OR PIPES SHALL BE AVOIDED EXCEPT WHERE DETAILED OPENINGS ARE PROVIDED.
- 11. MODULUS OF ELASTICITY OF CONCRETE, WHEN TESTED IN ACCORDANCE WITH ASTM C-460, SHALL BE AT LEAST THE VALUE GIVEN BY THE EQUATIONS IN SECTION 8.5.1 OF ACI 318 FOR THE SPECIFIED 28-DAY STRENGTH.
- 12. SEE FOUNDATION DETAILS FOR REINFORCEMENT REQUIRED AT CORNERS AND INTERSECTIONS OF CONCRETE WALLS, CONVENTIONAL FOOTINGS AND GRADE BEAMS.

WOOD

- FRAMING LUMBER, UNO: A. 2X AND 4X TO BE DOUGLAS FIR LARCH (DFL) NO. 2 GRADE. SEE FRAMING PLANS AND NOTES FOR WALL STUD REQUIREMENTS.
- B. 6X TO BE DOUGLAS FIR LARCH NO. 1 GRADE. C. ALL LUMBER SHALL HAVE A MOISTURE CONTENT OF LESS THAN 19%.
- 2. BOLT HOLES SHALL BE 1/16" (MAXIMUM) LARGER THAN THE BOLT SIZE. RE-TIGHTEN ALL NUTS PRIOR TO CLOSING IN.
- 3. ALL SILLS OR PLATES RESTING ON CONCRETE OR MASONRY SHALL BE PRESSURE TREATED DOUGLAS FIR USING BORON BASED PRESERVATIVES OR LSL TREATED WITH ZINC BORATE.
- 4. DO NOT NOTCH JOISTS, RAFTERS OR BEAMS, EXCEPT WHERE SHOWN IN DETAILS. OBTAIN ENGINEER'S APPROVAL FOR ANY HOLES OR NOTCHES NOT DETAILED. (i.e. ALL POSTS TO BE CONTINUOUS UNTIL FULLY SUPPORTED BY BEAM OR FOUNDATION BELOW.)
- WHERE A POST OCCURS AT AN UPPER LEVEL, ADD THE SAME POST DIRECTLY BELOW IT ON THE LOWER LEVELS AND IN BETWEEN FLOOR SHEATHING AND LOWER LEVEL WALL TOP PLATES. ALL POSTS TO BE CONTINUOUS UNTIL FULLY SUPPORTED BY BEAM OR FOUNDATION BELOW.
- 6. FACE NAIL EACH PLY OF MULTIPLE 2X POSTS WITH 16D SINKERS AT 6" O.C.
- 7. PROVIDE MULTIPLE 2X POST AT ALL GIRDER TRUSS AND BEAM BEARING LOCATIONS, WIDTH TO MATCH BEAM OR TRUSS, MIN. (2) 2X UNO.
- 8. CONNECTION HARDWARE SHALL BE SIMPSON OR EQUAL AND MUST BE I.C.C. APPROVED.
- 9. FOR CONNECTIONS NOT DETAILED, PROVIDE FASTENERS PER TABLE 2304.9.1 OF THE LATEST ADOPTED EDITION OF THE INTERNATIONAL BUILDING CODE.
- 10. DO NOT NOTCH TOP PLATES OR STUDS EXCEPT AS SHOWN IN DETAILS. OBTAIN ENGINEER'S APPROVAL FOR ANY HOLES OR NOTCHES NOT DETAILED.
- 11. NON-BEARING WALLS SHALL HAVE STUDS SPACED AT 24" O.C. (MAX). REFER TO ARCHITECTURAL DRAWINGS FOR SIZE. TOP PLATES SHALL BE SUCH THAT A 1/2" GAP BETWEEN THE TOP OF THE PLATES AND THE BOTTOM OF THE TRUSSES AND/OR BLOCKING PANELS EXISTS AFTER THE ROOF IS LOADED.
- 12. TYPICAL FASTENER SIZE NOTED IN DRAWINGS: 8d COMMON NAILS = 0.131" DIA X 2 1/2", 10d COMMON NAILS = 0.148" DIA 16d COMMON NAILS = 0.162" DIA 16d SINKER NAILS = 0.148" DIA X3 1/4".
- 13. 16d SINKERS NOTED IN THESE DRAWINGS MAY BE REPLACED WITH 0.131 X 3 1/4" GUN NAILS PER THE FOLLOWING TABLES:

# OF 16d SINKERS	1	2	3	4	5	6	7	8	9	10	11	12
# OF 0.131 X 3 1/4" GUN NAILS	2	3	4	5	6	7	8	9	11	12	13	14
SPACING OF 16d SINKERS	2" C	С	4"	00	6" (OC	8" OC	;	10" 00	2	12" (C
SPACING OF 0.131 X 3 1/4" GUN NAILS	1 1/2"	00	3 1/2	2" OC	5" (OC	7" OC	: 8	8 1/2" C)C	10 1/2	" OC

MANUFACTURED BEAMS

- 1. GLUE LAMINATED BEAMS (GLB) A. GLB SHALL BE 24F-V4, (CONTINUOUS AND CANTILEVERED GLB SHALL BE 24F-V8) AND HAVE THE FOLLOWING MINIMUM PROPERTIES: FB=2400 PSI, FV=240 PSI FC (PERPENDICULAR) = 650 PSI
- E=1,800,000 PSI B. ALL BEAMS SHALL BE FABRICATED USING EXTERIOR GLUE. FABRICATION AND HANDLING PER LATEST AITC AND WCCA STANDARDS. BEAMS TO BEAR GRADE STAMP AND AITC STAMP AND
- CERTIFICATE . MOISTURE CONTENT SHALL BE LIMITED TO A MAXIMUM OF 12%.
- D. ALL GLB SHALL HAVE STANDARD CAMBER, UNO. ON PLANS.

LAMINATED STRAND LUMBER (1.3E OR 1.5E LSL)

- LSL BEAMS SHALL HAVE I.C.C APPROVAL AND HAVE THE FOLLOWING MINIMUM PROPERTIES: 1) LSL (1.3E) - E = 1,300,000 PSI, FB = 1700 PSI, FV = 400 PSI FC (PERPENDICULAR) = 680 PSI, FC (PARALLEL) = 1400 PSI
- 2) LSL (1.5E) E = 1,500,000 PSI, FB = 2250 PSI, FV = 400 PSI
 - FC (PERPENDICULAR) = 750 PSI, FC (PARALLEL) = 1950 PSI B. ALL MULTI-PLY LSL MEMBERS SPECIFIED ON PLANS MAY BE REPLACED WITH SOLID MEMBERS OF EQUAL OR GREATER PROPERTIES WITH EQUAL OR GREATER WIDTH AND DEPTH WITHOUT FURTHER REVIEW.

LAMINATED VENEER LUMBER (LVL

- A. LVL BEAMS SHALL HAVE I.C.C APPROVAL AND HAVE THE FOLLOWING MINIMUM PROPERTIES: 1) LVL (1.7E) - E = 1,700,000 PSI, FB = 2650 PSI, FV = 285 PSI,
- FC (PERPENDICULAR) = 750 PSI, FC (PARALLEL) = 3000 PSI
- 2) LVL (1.9E) E = 1,900,000 PSI, FB = 2600 PSI, FV = 285 PSI,
- FC (PERPENDICULAR) = 750 PSI, FC (PARALLEL) = 2510 PSI 3) LVL (2.0E) - E = 2,000,000 PSI, FB = 2800 PSI, FV = 285 PSI,
- FC (PERPENDICULAR) = 750 PSI, FC (PARALLEL) = 3000 PSI
- B. MULTIPLE-PLY LVL BEAMS SHALL BE NAILED TOGETHER AS FOLLOWS: 1) PROVIDE (2) ROWS OF 16D SINKERS AT 12" O.C. FOR BEAMS 11 7/8" DEEP.
- 2) PROVIDE (3) ROWS OF 16D SINKERS AT 12" O.C.
- FOR BEAMS > 11 7/8" DEEP. C. ALL MULTI-PLY LVL MEMBERS SPECIFIED ON PLANS MAY BE REPLACED WITH SOLID MEMBERS
- OF EQUAL OR GREATER PROPERTIES WITH EQUAL OR GREATER WIDTH AND DEPTH WITHOUT FURTHER REVIEW.
- 4. PARALLEL STRAND LUMBER (PSL)
- A. PSL BEAMS SHALL HAVE I.C.C APPROVAL AND HAVE THE FOLLOWING MINIMUM PROPERTIES: E = 2,000,000 PSI, FB = 2900 PSI, FV = 290 PSI
 - FC (PERPENDICULAR) = 750 PSI, FC (PARALLEL) = 2900 PSI
- PREFABRICATED WOOD TRUSSES
- 1. PREFABRICATED WOOD ROOF TRUSSES SHALL BE AS DESIGNED BY THE TRUSS MANUFACTURER (INCLUDING BRIDGING SIZE AND SPACING) UNO. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS, ERECTION DRAWINGS, AND DESIGN CALCULATIONS, SEALED BY AN ENGINEER, REGISTERED IN THE STATE OF THE GOVERNING JURISDICTION, FOR REVIEW PRIOR TO MANUFACTURE. CALCULATIONS AND SHOP DRAWINGS SHALL SHOW ANY SPECIAL DETAILS REQUIRED AT BEARING POINTS. ALL CONNECTORS SHALL HAVE CURRENT I.C.C. APPROVAL.
- 2. TRUSS MANUFACTURER TO DESIGN TRUSSES FOR LATERAL LOAD (LAT. = XXXX) IN POUNDS, AS SHOWN ON PLANS.
- 3. UNLESS OTHERWISE NOTED, TRUSSES TO BE DESIGNED FOR LOADS INDICATED IN THE DESIGN CRITERIA AND AS FOLLOWS: A. TOP CHORD DEAD LOAD = XXX + ADDITIONAL 5 PSF AT OVERFRAMING.
 - B. BOTTOM CHORD DEAD LOAD = XXX. C. WIND LOAD: WHEN CALCULATING NET UPLIFT REACTIONS, USE MAXIMUM RESISTING DEAD LOAD = XXX PSF ON TOP CHORD AND XXX PSF ON BOTTOM CHORD
- 4. TRUSS MANUFACTURER TO DESIGN TRUSSES TO SUPPORT MECHANICAL EQUIPMENT AS REQUIRED. ADDITIONAL TRUSSES MAY BE SUPPLIED AS REQUIRED
- 5. ALL TRUSS TO TRUSS CONNECTORS PER TRUSS MANUFACTURER.
- 6. WHERE POST OCCURS ABOVE A MANUFACTURED TRUSS, A VERTICAL WEB SHALL BE PROVIDED UNDER THE POST. THE WEB SECTION AREA SHALL BE EQUAL TO OR LARGER THAN POST SECTION AREA. MULTIPLE 2X4S MAY BE USED FOR THE VERTICAL WEB.
- 7. THE TOP CHORD OF ALL TRUSSES SHALL HAVE A SPECIES WITH SPECIFIC GRAVITY EQUAL TO OR GREATER THAN 0.42.
- 8. TRUSS MANUFACTURER SHALL LIMIT TOTAL LOAD DEFLECTIONS TO LESS THAN L/240 AND LIVE LOAD DEFLECTIONS TO LESS THAN L/360. DEFLECTION SHALL BE LIMITED SO AS NOT TO CREATE A BEARING CONDITION AT NON-BEARING WALLS. REFER ALSO TO NOTE 11 OF THE WOOD SECTION.
- 9. CONNECTION OF MANUFACTURED TRUSSES FOR UPLIFT SHALL BE PER THE TABLES ON SHEET S1.1.
- 10. TRUSSES SHALL BE DESIGNED TO SUPPORT A 250 POUND CONCENTRATED LOAD AT ANY LOCATION ALONG THE BOTTOM CHORD.

	STANDARD TRUSS	ES ⁽²⁾
REACTIO	NS TO THE BEARING V	VALL OR BEAM BELOW:
TRUSS TYPE & TOTAL LENGTH	HARDWARE REQUIRED	NAILING REQUIRED
JACK TRUSSES 10'-0"	NONE REQUIRED	UPLIFT IS RESISTED BY EXISTING NAILED CONNECTION OF TRUSS TO PLATE ⁽¹⁾
HIP/COMMON TRUSSES 30'-0"	NONE REQUIRED	UPLIFT IS RESISTED BY EXISTING NAILED CONNECTION OF TRUSS TO PLATE ⁽¹⁾
HIP/COMMON TRUSS 30'-0" L 50'-0"	H1 OR H2.5	(6) 8d x 1 1/2" INTO TRUSS (4) 8d x 1 1/2" INTO PLATE
ALL HIP/COMMON DRAG TRUSSES L 50'-0"	H1 OR H2.5	(6) 8d x 1 1/2" INTO TRUSS (4) 8d x 1 1/2" INTO PLATE
HIP/COM. GIRDER 8FT SETBACK L 30'-0"	H10 OR (2) H2.5	(8) 8d x 1 1/2" INTO TRUSS (8) 8d x 1 1/2" INTO PLATE
HIP/COM. GIRDER 8FT SETBACK L 40'-0"	HTS20	(12) 10d x 1 1/2" INTO TRUSS (12) 10d x 1 1/2" INTO PLATE & STUD BLW
HIP/COM. GIRDER 8FT SETBACK L 50'-0"	(2) HTS20	(12) 10d x 1 1/2" INTO TRUSS (12) 10d x 1 1/2" INTO PLATE & STUD BLW
ALL OTHER NON-DRAG GIRDER TR L 40'-0"	H1 OR H2.5	(6) 8d x 1 1/2" INTO TRUSS (4) 8d x 1 1/2" INTO PLATE
ALL OTHER GIRDER TRUSSES L 50'-0"	H10 OR (2) H2.5	(8) 8d x 1 1/2" INTO TRUSS (8) 8d x 1 1/2" INTO PLATE
AT TRUSSES WHERE THE HARDWARE ABO	VE CANNOT BE INSTALLE	D, PROVIDE AN HTS20 STRAP
GAE	BLE END WALL TRU	JSSES
HARDWARE		NAILING REQUIRED
LTP4 AT 32" OC UNO	6) 8d x 1 1/2" INTO TRUSS (4) 8d x 1 1/2" INTO PLATE
NOTES:		

- (1) SEE DETAIL 1/SD-3 FOR STANDARD NAILING REQUIREMENTS (2) SEE FRAMING PLANS FOR NON-STANDARD UPLIFT HARDWARE

STRUCTURAL SHEATHING

SHALL HAVE ALL EDGES BLOCKED.

SHEAR WALL SHEATHIN

SHOP DRAWING

PER SHEAR WALL SCHEDULE

BY ARCHITECTURAL SPECIFICATIONS.

3. VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

RED-LINED OR FLAGGED BY SUBMITTING PARTIES.

THE DESIGNING OR SUBMITTING AUTHORITY.

REQUIRED BY THE PERMITTING AGENCY.

ITEMS REQUIRING SPECIAL INSPECTION:

DEFERRED SUBMITTAL ITEMS

1. MFR WOOD TRUSSES.

ARCHITECT/STRUCTURAL ENGINEER FOR REVIEW.

A. POST INSTALLED CONCRETE ANCHORS

STRUCTURAL ENGINEER.

SPECIAL INSPECTIONS

SHT #	SHEET NAME
S1	GENERAL NOTES
S1.1	SCHEDULES AND NOTES
S2	FOUNDATION PLAN
S3	ROOF FRAMING PLANS
S4	SHEAR WALL PLANS
SD-1	STRUCTURAL DETAILS
SD-1.1	STRUCTURAL DETAILS
SD-2	STRUCTURAL DETAILS
SD-2.1	STRUCTURAL DETAILS
SD-3	STRUCTURAL DETAILS
WSW1	SIMPSON STRONG-WALL DETAILS
WSW2	SIMPSON STRONG-WALL DETAILS
WSW4	SIMPSON STRONG-WALL DETAILS

1. ALL STRUCTURAL SHEATHING SHALL BE C-D INTERIOR SHEATHING WITH EXTERIOR GLUE OR ORIENTED

ROOF SHEATHING AT UNBLOCKED DIAPHRAGM (STANDARD, UNO)

3. ROOF SHEATHING AT BLOCKED DIAPHRAGM (WHERE NOTED ON PLANS)

4. FLOOR SHEATHING AT UNBLOCKED DIAPHRAGM (STANDARD, UNC

O.C. AT ALL BOUNDARIES AND SUPPORTED EDGES, 12" O.C. FIELD, UNO ON PLANS.

SAME AS SHOWN FOR 10D COMMON NAILS PER I.C.C. REPORT ESR-1472.

5. FLOOR SHEATHING AT BLOCKED DIAPHRAGM (WHERE NOTED ON PLANS)

SHOWN FOR 10D COMMON NAILS PER I.C.C. REPORT ESR-1472.

STRAND BOARD (OSB), EXPOSURE I, AND SHALL BEAR THE STAMP OF AN APPROVED TESTING AGENCY.

LAY SHEATHING WITH LONG DIMENSION PERPENDICULAR TO SUPPORTS. STAGGER JOINTS AND NAILS.

5/32" WOOD STRUCTURAL PANEL: PANEL INDEX = 32/16, UNBLOCKED UNO W/ 8D COMMON NAILS AT 6"

O.C. AT ALL BOUNDARIES AND SUPPORTED EDGES, 12" O.C. FIELD. PANELS LESS THAN 2'-0" IN WIDTH

15/32" WOOD STRUCTURAL PANEL: PANEL INDEX = 32/16, BLOCKED. NAIL W/ 8D COMMON NAILS AT 4"

23/32" WOOD STRUCTURAL PANEL: T AND G, PANEL INDEX 48/24, UNBLOCKED (UNO.). NAIL WITH 10D

PLANS. PANELS LESS THAN 2'-0" IN WIDTH SHALL HAVE ALL EDGES BLOCKED. OPTION: QUIK DRIVE #8

23/32" WOOD STRUCTURAL PANEL: T AND G, PANEL INDEX = 48/24, BLOCKED. NAIL WITH 16D SINKERS

AT 4" O.C. AT ALL BOUNDARIES AND SUPPORTED EDGES, 12" O.C. FIELD UNO ON PLANS. OPTION: QUIK

DRIVE #8 X 2 1/4" SCREWS MAY BE USED IN LIEU OF 16D SINKERS ABOVE. SPACING SHALL BE SAME AS

1. SHOP DRAWINGS SHALL BE SUBMITTED FOR ALL STRUCTURAL ITEMS IN ADDITION TO ITEMS REQUIRED

4. ANY CHANGES, SUBSTITUTIONS, OR DEVIATIONS FROM ORIGINAL STRUCTURAL DRAWINGS SHALL BE

5. THE STRUCTURAL ENGINEER HAS THE RIGHT TO APPROVE OR DISAPPROVE ANY CHANGES TO THE

6. THE SHOP DRAWINGS DO NOT REPLACE THE ORIGINAL STRUCTURAL DRAWINGS. ITEMS OMITTED OR

7. THE ADEQUACY OF ENGINEERING DESIGNS AND LAYOUT PERFORMED BY THE OTHERS RESTS WITH

8. REVIEWING IS INTENDED ONLY AS AN AID TO THE CONTRACTOR IN OBTAINING CORRECT SHOP

DRAWINGS. RESPONSIBILITY FOR CORRECTNESS SHALL REST WITH THE CONTRACTOR.

9. THE CONTRACTOR SHALL PROVIDE (1) FILE COPY OF ALL APPROVED SHOP DRAWINGS TO THE

1. PROVIDE SPECIAL INSPECTIONS IN ACCORDANCE WITH SECTION 1705 OF THE BUILDING CODE, AS

2. WHERE SPECIAL INSPECTION IS REQUIRED, IT SHALL BE PERFORMED BY A REGISTERED DEPUTY

OF THE INSPECTION REPORTS SHALL BE SUBMITTED TO THE BUILDING DEPARTMENT AND

INSPECTOR EMPLOYED BY THE OWNER AND APPROVED BY THE GOVERNING JURISDICTION. COPIES

CODE SECTION

SHOWN INCORRECTLY BUT NOT FLAGGED BY THE STRUCTURAL ENGINEER OR ARCHITECT ARE NOT TO

ORIGINAL DRAWINGS AT ANY TIME, BEFORE OR AFTER SHOP DRAWING REVIEW.

BE CONSIDERED CHANGES TO THE ORIGINAL CONTRACT DRAWING.

2. THE CONTRACTOR SHALL REVIEW ALL SHOP DRAWINGS PRIOR TO SUBMITTAL. ITEMS NOT IN

ACCORDANCE WITH CONTRACT DRAWINGS SHALL BE FLAGGED FOR REVIEW.

COMMON NAILS AT 6" O.C. AT ALL BOUNDARIES AND SUPPORTED EDGES, 12" O.C. FIELD UNO ON

WSNTL WOOD SCREWS MAY BE USED IN LIEU OF 10D COMMON NAILS ABOVE. SPACING SHALL BE

DESIGNED BY: SLB DRAWN BY: ACM ISSUED FOR:

JOB NO: 1939-006-201

CONSTRUCTION DOCUMENTS DATE: 07-09-20

STRUCTURAL GENERAL NOTES

REVISIONS



SHEET INDEX

SHEARWALL ^(7, 17) MARKMATERIAL ^(4, 14) NAILINGP13/8" APA SHEATHING ⁽¹⁵⁾ 8dN AT 6" OC EDGES, 12" FIELP23/8" APA SHEATHING ⁽¹⁵⁾ 8dN AT 4" OC EDGES, 12" FIELP33/8" APA SHEATHING ⁽¹⁵⁾ 8dN AT 3" OC EDGES, 12" FIELP43/8" APA SHEATHING ^(9, 15) 8dN AT 2" OC EDGES, 12" FIELP43/8" APA SHEATHING ^(9, 15) 8dN AT 2" OC EDGES, 12" FIELP515/32" APA SHEATHING ⁽¹⁰⁾ 10dN AT 2" OC EDGES, 12" FIELP515/32" APA SHEATHING ⁽¹⁰⁾ 10dN AT 2" OC EDGES, 12" FIEL(1)SEE DETAIL 7/SD-2 FOR ADDT'L INFO AT UA APPLY TO SINGLE STORY PLANS. SEE PL (2)(2)OPENINGS IN SHEARWALL SHEATHING S SUM OF ANY TWO OR MORE OPENINGS OF GREATER THAN 8 INCHES DO NOT REQU FOR REQUIREMENTS AT OPENINGS NOT (3)(3)MINIMUM (2) 1/2" DIA ANCHORS PER SHE SHEET, S1. ALL ANCHOR BOLTS SHALL F (4)(4)APA RATED (STRUCTURAL II) PLYWOOD C (5)(5)SEE DETAIL 11/SD-2 WHERE WALL FRAMI (6)(6)FOR SHEAR PANELS ON TWO SIDES OF V CONNECTION AND ANCHOR BOLT SPACI (7)(7)DOUBLE SIDED SHEARWALLS SHALL HAV 3" NOMINAL STUDS (MIN) AT JOINTS. AT	ONE SII TWO SID UPPER FLOOR SILL PL CONN ^(1, 18) 16dS AT 4" OC (STG'D) UNO 16dS AT 3" OC, (STG'D) UNO 16dS AT 3" OC, (STG'D) UNO 1/4x6 SCREWS AT 4" OC (STG'D), UNO 1/4x6 SCREWS AT 4" OC (STG'D), UNO 1/4x6 SCREWS AT 4" OC (STG'D), UNO 1/4x6 SCREWS AT 4" OC (STG'D), UNO 1/4x6 SCREWS AT 3" OC (STG'D), UNO JPPER FLOOR SILL PLATE CONNE AN FOR SILL PLATE AND SHEAR (HALL NOT EXCEED 8 INCHES IN A ON COMMON OR OVERLAPPING V IRE BLOCKING AROUND THE PEN OTHERWISE DETAILED. AR WALL. SEE SECTION FOUNDA 1AVE 3" x 3" x 0.229" PLATE WASHE OR OSB. NG STEPS OR PERPENDICULAR W VALL, USE ONE-HALF THE SPACIN NG, UNO.	DED SHEARWALL ED SHEARWALL ⁽⁶⁾ ANCHOR BOLTS ⁽³⁾ (UNO ON FOUNDATION PLAN) 1/2" DIA X 10" AT 32" OC 1/2" DIA X 10" AT 32" OC 1/2" DIA X 10" AT 24" OC 1/2" DIA X 10" AT 16" OC 1/2" DIA X 10" AT 16" OC 1/2" DIA X 10" AT 12" OC 1/2" DIA X 10" AT 9" OC 1/2" DIA X 10" AT 9" OC 1/2" DIA X 10" AT 9" OC CC CC IION FOR MULTI-STORY PLANS CONNECTIONS AT EXTERIOR WAL NY DIRECTION FOR A SINGLE OP FERTICAL OR HORIZONTAL LINES. ETRATION. CONTACT THE ENGIN TION HARDWARE, NOTE 2 ON THI ERS.	SEISMIC ALLON SH 260#/FT 350#/FT 490#/FT 600#/FT 600#/FT 770#/FT 0NLY. DOES LS. ENING OR TH OPENINGS IEER OF REC E GENERAL I	WIND WABLE EAR 365#/FT 532#/FT 685#/FT 895#/FT 1078#/FT 1078#/FT S NOT HE NOT CORD NOTES
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P43/8" APA SHEATHING ^(9, 15) 8dN AT 2" OC EDGES, 12" FIELP515/32" APA SHEATHING ⁽¹⁰⁾ 10dN AT 2" OC EDGES, 12" FIELSCHEDULE NOTES:10dN AT 2" OC EDGES, 12" FIEL(1)SEE DETAIL 7/SD-2 FOR ADDT'L INFO AT U APPLY TO SINGLE STORY PLANS. SEE PL (2)(2)OPENINGS IN SHEARWALL SHEATHING S SUM OF ANY TWO OR MORE OPENINGS OF FOR REQUIREMENTS AT OPENINGS NOT (3)(3)MINIMUM (2) 1/2" DIA ANCHORS PER SHE SHEET, S1. ALL ANCHOR BOLTS SHALL F (4)(4)APA RATED (STRUCTURAL II) PLYWOOD OF CONNECTION AND ANCHOR BOLT SPACI (5)(5)SEE DETAIL 11/SD-2 WHERE WALL FRAMI (6)(6)FOR SHEAR PANELS ON TWO SIDES OF V CONNECTION AND ANCHOR BOLT SPACI (7)(7)DOUBLE SIDED SHEARWALLS SHALL HAV 3" NOMINAL STUDS (MIN) AT JOINTS. AT	1/4x6 SCREWS AT 4" OC (STG'D), UNO.D(16)1/4x6 SCREWS AT 3" OC (STG'D), UNO.D(16)1/4x6 SCREWS AT 3" OC (STG'D), UNOJPPER FLOOR SILL PLATE CONNE .AN FOR SILL PLATE AND SHEAR (.HALL NOT EXCEED 8 INCHES IN A ON COMMON OR OVERLAPPING V IRE BLOCKING AROUND THE PEN OTHERWISE DETAILED. AR WALL. SEE SECTION FOUNDA .AVE 3" x 3" x 0.229" PLATE WASHE DR OSB. NG STEPS OR PERPENDICULAR W VALL, USE ONE-HALF THE SPACIN NG, UNO.	1/2" DIA X 10" AT 12" OC 1/2" DIA X 10" AT 9" OC 1/2" DIA X 10" AT 9" OC CONNECTIONS AT EXTERIOR WAL NY DIRECTION FOR A SINGLE OP ERTICAL OR HORIZONTAL LINES. ETRATION. CONTACT THE ENGIN TION HARDWARE, NOTE 2 ON THE ERS. VALL INTERSECTS SHEAR WALL. IG GIVEN IN THE SCHEDULE FOR	600#/FT 770#/FT ONLY. DOE: LS. ENING OR TI OPENINGS IEER OF REC E GENERAL I	895#/FT 1078#/FT S NOT HE NOT CORD NOTES
 P5 15/32" APA SHEATHING⁽¹⁰⁾ 10dN AT 2" OC EDGES, 12" FIEL SCHEDULE NOTES: (1) SEE DETAIL 7/SD-2 FOR ADDT'L INFO AT L APPLY TO SINGLE STORY PLANS. SEE PL (2) OPENINGS IN SHEARWALL SHEATHING S SUM OF ANY TWO OR MORE OPENINGS (GREATER THAN 8 INCHES DO NOT REQU FOR REQUIREMENTS AT OPENINGS NOT (3) MINIMUM (2) 1/2" DIA ANCHORS PER SHEL SHEET, S1. ALL ANCHOR BOLTS SHALL F (4) APA RATED (STRUCTURAL II) PLYWOOD (5) SEE DETAIL 11/SD-2 WHERE WALL FRAMI (6) FOR SHEAR PANELS ON TWO SIDES OF V CONNECTION AND ANCHOR BOLT SPACI (7) DOUBLE SIDED SHEARWALLS SHALL HAV 3" NOMINAL STUDS (MIN) AT JOINTS. AT 	Definition of the second secon	1/2" DIA X 10" AT 9" OC CTION FOR MULTI-STORY PLANS CONNECTIONS AT EXTERIOR WAL NY DIRECTION FOR A SINGLE OP ERTICAL OR HORIZONTAL LINES. ETRATION. CONTACT THE ENGIN TION HARDWARE, NOTE 2 ON THI ERS. VALL INTERSECTS SHEAR WALL. IG GIVEN IN THE SCHEDULE FOR	770#/FT ONLY. DOE: LS. ENING OR TI OPENINGS IEER OF REC E GENERAL I SILL PLATE	1078#/FT S NOT HE NOT CORD NOTES
 SCHEDULE NOTES: (1) SEE DETAIL 7/SD-2 FOR ADDT'L INFO AT U APPLY TO SINGLE STORY PLANS. SEE PL (2) OPENINGS IN SHEARWALL SHEATHING S SUM OF ANY TWO OR MORE OPENINGS (GREATER THAN 8 INCHES DO NOT REQU FOR REQUIREMENTS AT OPENINGS NOT (3) MINIMUM (2) 1/2" DIA ANCHORS PER SHE SHEET, S1. ALL ANCHOR BOLTS SHALL F (4) APA RATED (STRUCTURAL II) PLYWOOD (2) SEE DETAIL 11/SD-2 WHERE WALL FRAMI (6) FOR SHEAR PANELS ON TWO SIDES OF V CONNECTION AND ANCHOR BOLT SPACI (7) DOUBLE SIDED SHEARWALLS SHALL HAV 3" NOMINAL STUDS (MIN) AT JOINTS. AT 	JPPER FLOOR SILL PLATE CONNE LAN FOR SILL PLATE AND SHEAR HALL NOT EXCEED 8 INCHES IN A ON COMMON OR OVERLAPPING V IRE BLOCKING AROUND THE PEN OTHERWISE DETAILED. AR WALL. SEE SECTION FOUNDA HAVE 3" x 3" x 0.229" PLATE WASHE OR OSB. NG STEPS OR PERPENDICULAR W VALL, USE ONE-HALF THE SPACIN NG, UNO.	CTION FOR MULTI-STORY PLANS CONNECTIONS AT EXTERIOR WAL NY DIRECTION FOR A SINGLE OP ERTICAL OR HORIZONTAL LINES. ETRATION. CONTACT THE ENGIN TION HARDWARE, NOTE 2 ON THI ERS. VALL INTERSECTS SHEAR WALL. IG GIVEN IN THE SCHEDULE FOR	ONLY. DOE: LS. ENING OR TI OPENINGS IEER OF REC E GENERAL I SILL PLATE	S NOT HE NOT CORD NOTES
 SEE DETAIL 7/SD-2 FOR ADDT'L INFO AT U APPLY TO SINGLE STORY PLANS. SEE PL OPENINGS IN SHEARWALL SHEATHING S SUM OF ANY TWO OR MORE OPENINGS O GREATER THAN 8 INCHES DO NOT REQU FOR REQUIREMENTS AT OPENINGS NOT MINIMUM (2) 1/2" DIA ANCHORS PER SHE SHEET, S1. ALL ANCHOR BOLTS SHALL F APA RATED (STRUCTURAL II) PLYWOOD O SEE DETAIL 11/SD-2 WHERE WALL FRAMI FOR SHEAR PANELS ON TWO SIDES OF V CONNECTION AND ANCHOR BOLT SPACI DOUBLE SIDED SHEARWALLS SHALL HAV 3" NOMINAL STUDS (MIN) AT JOINTS. AT 	JPPER FLOOR SILL PLATE CONNE AN FOR SILL PLATE AND SHEAR HALL NOT EXCEED 8 INCHES IN A ON COMMON OR OVERLAPPING V IRE BLOCKING AROUND THE PEN OTHERWISE DETAILED. AR WALL. SEE SECTION FOUNDA HAVE 3" x 3" x 0.229" PLATE WASHE OR OSB. NG STEPS OR PERPENDICULAR W VALL, USE ONE-HALF THE SPACIN NG, UNO.	CTION FOR MULTI-STORY PLANS CONNECTIONS AT EXTERIOR WAL NY DIRECTION FOR A SINGLE OP ERTICAL OR HORIZONTAL LINES. ETRATION. CONTACT THE ENGIN TION HARDWARE, NOTE 2 ON THI ERS. VALL INTERSECTS SHEAR WALL. IG GIVEN IN THE SCHEDULE FOR	ONLY. DOE: LS. ENING OR TI OPENINGS IEER OF REC E GENERAL I SILL PLATE	S NOT HE NOT CORD NOTES
 EACH SIDE SHALL BE STAGGERED. (8) ALL SHEARWALLS REQUIRE DOUBLE 2X T PROVIDE DEFLECTION CLEARANCE. (9) P2, P3 AND P4 SHEARWALLS SHALL REQUIND A. STAGGER NAILING ALONG PLYWOOD B. SINGLE 3" NOMINAL MEMBERS AT ALL NOMINAL MEMBERS AT SINGLE SIDE W/ (2) ROWS OF 16d SINKERS AT 4" (2) (10) P5 SHEARWALLS SHALL REQUIRE SINGLE ABUTTING PANELS (MIN). STAGGER JOIN (11) ALL SHEARWALL LENGTHS NOTED ON PL (12) SHEATHING MAY BE PLACED ON EITHER (13) ALLOWABLE SHEAR CAPACITIES ARE IN A INCLUDED. (14) APA SHEATHING AND GYPSUM SHEATHING FRAMING MUST BE 16" ON CENTER (MAX PERPENDICULAR TO THE FRAMING ALL F (15) 3/8" SHEATHING MAY BE REPLACED WITH (16) ALL VERTICAL AND HORIZONTAL PANELS 	 ⁷E VERTICAL PANEL JOINTS OFFS THE ENDS OF THE SHEARWALL, ⁴ ⁷FOP PLATES, U.N.O. AT NON-BEA ⁷JIRE THE FOLLOWING: ¹D JOINTS AND SILL PLATES. ¹L FRAMING MEMBERS RECEIVING ⁵D SHEARWALL MAY BE CONSTRUCC. ⁵ 3" NOMINAL MEMBERS AT ALL F ⁵T AND SILL PLATE NAILING. ⁵AN ARE MINIMUM REQUIRED ANI ⁵FACE OF DESIGNATED WALL, UN ⁵ACCORDANCE WITH AF&PA SDPW ⁵NG MAY BE INSTALLED WITH THE ⁵INSTALLED WITH THE SHORT I ⁵. WHERE GYPSUM SHEATHING I ⁵PANEL EDGES MUST BE BLOCKED ⁴7/16" OR 15/32" SHEATHING WITH 	ET TO FALL ON DIFFERENT STUD X NOMINAL MEMBERS ARE REQU RING SHEAR WALLS, SHORTEN S G EDGE NAILING FROM ABUTTING JCTED W/ (2) 2X MEMBERS FASTE RAMING MEMBERS RECEIVING EE D MAY BE INCREASED WITHOUT R O. /S TABLE 4.3A WITH APPLICABLE (LONG OR SHORT DIRECTION PER DIRECTION PERPENDICULAR TO T S INSTALLED WITH THE SHORT DI AND NAILED. HOUT ADDITIONAL REVIEW. FD.	S OR USE SI JIRED. NAILS TUDS 1/4 INC PANELS. 3" NED TOGETI OGE NAILING EVIEW. OMEGA FAC PENDICULA THE FRAMING RECTION	NGLE S ON CH TO HER S FROM TORS R TO G, THE
 (17) STUDS SHALL BE 24" OC (MAX). FOR 3/8" SPACING IS GREATER THAN 16" OC. (18) SILL PLATE CONNECTORS SHALL BE PER 	AND 7/16" SHEATHING, FIELD NA	ILING SHALL BE REDUCED TO 6" (IFIED ON PLANS.	DC WHERE S	STUD

HOLDOWN/S	STRAP SCHEDULE ^(1, 2)
MBED AT FND AND / DR ANCHOR BOLT	CONN TO (2) 2X STUD, UNO ^(3, 6, 10)
I/A	EXTEND STRAP 16" MIN. EA. END W/ (13) 8dN TO (2) 2X STUD ABOVE AND BELOW FLOOR FRAMING
I/A	EXTEND STRAP 16" MIN. EA. END W/ (13) 8dN TO (2) 2X STUD ABOVE AND BELOW FLOOR FRAMING
I/A	EXTEND STRAP 25" MIN. EA. END W/ (28) 16d SINKERS TO (2) 2X STUD ABOVE AND BELOW FLOOR FRAMING
I/A	EXTEND STRAP 32" MIN. EA. END W/ (33) 16dN TO (3) 2X STUD ABOVE AND BELOW FLOOR FRAMING
I/A	EXTEND STRAP 40" MIN. EA. END W/ (42) 16dN TO (3) 2X STUD ABOVE AND BELOW FLOOR FRAMING
" EMBED	(20) 16d SINKERS
0" EMBED	(24) 16d SINKERS
4" EMBED	(30) 16d SINKERS
STB24 W/ 21" EMBED ⁽¹²⁾	(26) 16dN X 2 1/2" NAILS
STB24 W/ 21" EMBED ⁽¹²⁾	(6) SDS 1/4 X 2 1/2 SCREWS W/ MIN (2) 2X POSTS
STB24 W/ 21" EMBED ⁽¹²⁾	(10) SDS 1/4 X 2 1/2 SCREWS W/ MIN (2) 2X POSTS
STB24 W/ 21" EMBED ⁽¹²⁾	(14) SDS 1/4 X 2 1/2 SCREWS W/ MIN (2) 2X POSTS
STB34 W/ 29" EMBED ⁽¹³⁾	(20) SDS 1/4 X 2 1/2 SCREWS W/ MIN (3) 2X POSTS
" DIA AB W/ 9" MIN EMBED W/ /IN 28" SQ x 14" DEEP FTG ⁽⁸⁾	(30) SDS 1/4 X 2 1/2 SCREWS W/ MIN 4X8 POST ⁽⁹⁾
" DIA AB W/ 10" MIN EMBED W/ /IN 30" SQ X 15" DEEP FTG ⁽⁷⁾	(36) SDS 1/4 X 2 1/2 SCREWS W/ MIN 4X8 POST ⁽⁹⁾

SCHEDULE NOTES:

HD/STRAF

(2) CS16

CMSTC16

CMST14

CMST12

LSTHD8^{(4,11}

STHD10^{(4,1*}

STHD14^{(4,1}

HTT5

HDU2

HDU4

HDU5

HDU8

HDU11

HDU14

CS16

 HD/STRAP SHALL BE SIMPSON OR EQUAL W/ ICC APPROVAL. ALL SUBSTITUTES SHALL BE REVIEWED BY THE ENGINEER OF RECORD BEFORE INSTALLATION.
 FIXED-LENGTH STRAPS SHALL BE INSTALLED WITH AN EQUAL LENGTH OVERLAPPING CONNECTED MEMBERS AND AN EQUAL NUMBER OF FASTENERS IN EACH MEMBER.
 STITCH NAIL EACH STUD AT MULTIPLE 2x STUDS TOGETHER WITH 16d SINKERS AT:

4" OC FOR P3 AND P4 SHEAR WALLS

6" OC FOR ALL OTHER SHEAR WALLS(4) FOR CONCRETE SPALLS LESS THAN 4", THERE IS NO LOAD REDUCTION AND NO FURTHER

REVIEW BY EOR IS REQUIRED.

(5) SEE DETAIL 4/SD-2 FOR ADDL CRITERIA AT UPPER FLOOR STRAPS (WHERE OCCURS).
(6) EDGE NAIL SHT'G TO EA MEMBER OF MULTPLE POST, OFFSET 1/2 SPACING BTWN MEMBERS.
(7) ASTM F1554-55 BOLT W/ HEAVY SQUARE NUT OR 1/4 X 1 3/4 X 1 3/4 PLATE WASHER REQUIRED FOR FULL LOAD. REDUCE ALLOWABLE LOAD TO 13180 LBS FOR ASTM GRADE 36 BOLT.

MINIMUM EMBEDMENT IS FROM TOP OF FOOTING.

(8) ASTM GRADE 36 BOLT W/ SQUARE OR HEAVY HEX HEAD OR NUT REQUIRED. MINIMUM EMBEDMENT IS FROM TOP OF FOOTING.

(9) PROVIDE 6X8 POST AT 2X6 WALLS, MULTIPLE STUDS NOT ALLOWED.

(10) END POST TO BE FULL HEIGHT MEMBERS, UNO.

(11) STRAPS MAY BE PLACED ON EITHER FACE OF DESIGNATED WALL AND ARE NOT REQUIRED TO OCCUR ON SAME FACE AS SHEATHING, UNO.

(12) AT GARAGE STEMWALL LOCATIONS USE SSTBL.

 (12) WHEN SLAB AND FOOTINGS ARE PLACED AS A MONO-POUR, SSTB28 WITH 25" EMBEDMENT MAY BE SUBSTITUTED FOR THE SSTB34 SPECIFIED.

FOUNDATION NOTES:

A. AT INTERIOR BEARING WALLS, WITHOUT DEEPENED FOOTINGS, USE 1/2" DIA TITEN HD HIGH STRENGTH SCREW ANCHORS IN LIEU OF ANCHOR BOLTS.

- B. SEE DETAIL 1/SD-1 FOR PERIMETER FOOTING EMBEDMENT DEPTH.
- C. VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

JOB NO: 1939-006-201 DESIGNED BY: SLB

DRAWN BY: ACM

ISSUED FOR:

CONSTRUCTION DOCUMENTS

DATE: 07-09-20 SHEET TITLE: SCHEDULES

AND NOTES

REVISIONS:

	INTE STU AND	RIOR BEARING AND SH D GRADE, UNO. SEE F SPACING.	IEAR WALLS TO RAMING PLANS	D BE MIN 2x4 A S FOR NON-TYF	T 16" O.C. DFL PICAL STUD SIZE
2.	TRIN	IMER / KING STUD SCH	IEDULE, UNO.		
		OPENING SPAN (L)	TRIMMERS	KING STUDS	
	S	L < 6'-0"	1	1	
	WALL	6'-0" L < 10'-0"	2	2	
	2x4	10'-0" L < 18'-0"	2	3	
	S	L < 8'-0"	1	1	
	WALI	8'-0" L < 12'-0"	2	2	
	2x6	12'-0" L < 20'-0"	2	3	
3.		BLOCKED DIA SHEET S1.	APHRAGM - SEE	E STRUCTURAL	. GENERAL NOTES
4.	FOR DRA	TYPICAL OVERFILL FRA WINGS, SEE DETAILS 4	AMING WHERE /SD-3 OR 5/SD-	REQUIRED BY 3.	TRUSS SHOP
5.			ARING WALLS		
6.	BEA LAR	M AND HEADER SIZES GER SIZES OR HIGHER	INDICATED ON GRADE LUMBE	THIS PLAN AR ER MAY BE SUE	E MINIMUM. BSTITUTED.
7.	TOP	PLATE SPLICES PER D	ETAIL 3/SD-2, U	JNO.	
8.	SEE	DETAIL 8/SD-2 FOR AD	DITIONAL FRAM	MING REQUIRE	MENTS.

ALL EXTERIOR WALLS TO BE MIN 2x6 AT 16" OC DFL STUD GRADE AND

FRAMING NOTES:

|--|

JNLE	ESS	OTHERWISE I	NOTED 2X LEDGEF	RS WHERE DETAILED SHALL BE AS FO	DLLOWS:
		TRU SPAN (L)	JSS SPACING (MAX)	LEDGER AND NAILING ^(1,2,6,10,11)	MIN HANGER, UNO ^(3,4,5,11)
			16" O.C.	2X6 W/ (3) 16d AT 16"OC	LUS24/JUS24
		L 8'-0"	24" O.C.	2X8 W/ (5) 16d AT 24"OC	LUS24/JUS24
DAD				2X8 W/ (5) 16d AT 16"OC	LUS26/JUS26
ΓU		L 16'-0"	24" O.C.	2X12 W/ (8) 16d AT 24"OC	LUS26/JUS26
00				2X12 W/ (8) 16d AT 16"OC	LUS28/JUS28
∞ L 24'-0"		L 24'-0"	24" O.C.	2X12 W/ 2 COLUMNS OF (8) 16d AT 24"OC	HUS28/HUS28
			16" O.C.	2X8 W/ (5) 16d AT 16"OC	LUS46/JUS46
OAD	BER	L 10'-0"	24" O.C.	2X10 W/ 2 COLUMNS OF (6) 16d AT 24"OC	LUS46/JUS46
LOOR L	IX MEMI	1 20.0"	24" O.C.	2X10 W/ 2 COLUMNS OF (6) 16d AT 16"OC	LUS48/JUS48
Ē			24" O.C.	2X12 W/ 2 COLUMNS OF (8) 16d AT 24"OC	HUS48/HUS48
			16" O.C.	2X8 W/ (5) 16d AT 16"OC	LUS26/JUS26
OAD	BER	L 10'-0"	24" O.C.	2X10 W/ 2 COLUMNS OF (6) 16d AT 24"OC	LUS26/JUS26
LOOR L	ex MEMI	1 20.0"	24" O.C.	2X10 W/ 2 COLUMNS OF (6) 16d AT 16"OC	LUS28/JUS28
LL.		L 20-0	24" O.C.	2X12 W/ 2 COLUMNS OF (8) 16d AT 24"OC	HUS28/HUS28
			16" O.C.	2X10 W/ (2) 1/4"x3" SCREWS AT 16" OC	LUS26/JUS26
_OAD	BER	L 10'-0"	24" O.C.	2X10 W/ 2 COLUMNS OF (2) 1/4"x3" SCREWS AT 24" OC	LUS26/JUS26
DECK	2× MEM		24" O.C.	2X10 W/ 2 COLUMNS OF (2) 1/4"x3" SCREWS AT 16" OC	LUS28/JUS28
		L 15-0"	24" O.C.	2X10 W/ 2 COLUMNS OF (3) 1/4"x3" SCREWS AT 24" OC	HUS28/HUS28

SCHEDULE NOTES:

 TWO COLUMNS OF FASTENERS REQUIRE MIN SUPPORTING MEMBER RECEIVING FASTENERS OF 3" OR (2)2X IN WIDTH (ENDS OF 4X2 OR DOUBLE TRUSSES ARE ACCEPTABLE). SPACE FASTENERS MIN 1" APART IN EACH DIRECTION.
 SPACING SHOWN EQUALS SPACING OF FRAMING MEMBERS RECEIVING FASTENERS.

(3) HANGERS LISTED IN ORDER ARE BY SIMPSON STRONG-TIE AND USP, RESPECTIVELY.
(4) LISTED HANGERS ARE MINIMUM HANGERS REQUIRED WHERE NOT OTHERWISE NOTED ON THE STRUCTURAL DRAWINGS.

(5) SPACING SHOWN FOR HANGERS IS SPACING OF THE FRAMING MEMBER SUPPORTED BY THE HANGER.
(6) LEDGER MATERIAL SHALL BE DFL #2 OR BETTER UNO. LEDGER AT DECK SHALL BE

PRESSURE-PRESERVATIVE TREATED OR NATURALLY DURABLE WOOD.
(7) PROVIDE THA218 (MIN) HANGER FOR ROOF TRUSS TO BEAM CONNECTIONS FOR ALL BEAM DEDTHS. CREATED THAN 10 INCHES.

BEAM DEPTHS GREATER THAN 10 INCHES.
 (8) PROVIDE THA418 (MIN) HANGER FOR FLOOR TRUSS TO BEAM CONNECTIONS FOR ALL BEAM DEPTHS GREATER THAN 10 INCHES.

 (9) PROVIDE THA422 (MIN) HANGER FOR FLOOR TRUSS TO BEAM CONNECITONS FOR ALL BEAM DEPTHS GREATER THAN 18 INCHES.
 (10) ALL SIMPSON SCREWS NOTED SHALL BE STRONG DRIVE SDS SCREWS

(10) ALL SIMPSON SCREWS NOTED SHALL BE STRONG DRIVE SDS SCREWS. ALL USP SCREWS NOTED SHALL BE WS SCREWS.

 (11) HANGERS AND FASTENERS WITH EXTERIOR EXPOSURE SHALL BE STAINLESS STEEL, UNLESS NOTED OTHERWISE.





FOUNDATION PLAN



SIZE TO MATCH BOX COLUMN + 6" EA. SIDE



ISSUED FOR:

CONSTRUCTION DOCUMENTS

DATE: 07-09-20

SHEET TITLE: FOUNDATION PLAN

REVISIONS:





S2

SEE SHEET S1.1 FOR FOUNDATION NOTES AND HOLDOWN SCHEDULE





JOB NO: 1939-006-201 DESIGNED BY: SLB DRAWN BY: ACM

ISSUED FOR:

CONSTRUCTION DOCUMENTS DATE: 07-09-20

SHEET TITLE: FRAMING PLAN

REVISIONS:

SEE SHEET S1.1 FOR FRAMING NOTES



SHEAR WALL PLAN

3/16"=1'-0"

JOB NO: 1939-006-201 DESIGNED BY: SLB DRAWN BY: ACM ISSUED FOR: CONSTRUCTION DOCUMENTS DATE: 07-09-20 SHEET TITLE: SHEAR WALL PLAN **REVISIONS**: ASSURED DEVELOPMENT OT 2 DN, NV HENDERSON, ATHENS







SEE SHEET S1.1 FOR SHEAR WALL AND HOLDOWN SCHEDULES



HIGH ROOF SHEAR WALL PLAN

3/16"=1'-0"









NTS

(12) BOX COLUMN SILL PLATE

(4) INTERIOR CONTINUOUS FTG



NTS









INTERIOR NON-BEARING WALLS





DEVELOPMEN \sim Z OT Ζ Ο THENS S \propto NDEI \square SURE \checkmark S I \triangleleft GINEER Daven LOTHO BAREN Exp. 12-31-2020 CIVIL SD-3

DRAWN BY: ACM ISSUED FOR: CONSTRUCTION DOCUMENTS DATE: 07-09-20 SHEET TITLE: STRUCTURAL DETAILS

JOB NO: 1939-006-201

DESIGNED BY: SLB

REVISIONS:



		_	13,100	28	10	17,100	33	11
	CRACKED	HIGH	25,200	41	14	32,700	48	16
SEISMIC		STRENGTH	27,100	43	15	35,300	51	17
SEISMIC			12,000	22	8	16,300	27	9
		STANDARD	13,100	24	8	17,100	28	10
	UNCRACKED	HIGH	25,300	36	12	32,700	42	14
		STRENGTH	27,100	38	13	35,300	44	15
			5,000	13	6	5,600	14	6
		STANDARD	8,800	19	7	10,200	21	7
WIND	CRACKED		13,100	25	9	17,100	30	10
		D HIGH STRENGTH	15,700	28	10	20,100	33	11
			19,200	32	11	25,300	38	13
			23,200	36	12	32,300	44	15
			27,100	40	14	35,300	47	16
	UNCRACKED	STANDARD	5,500	12	6	6,200	13	6
			8,500	16	6	12,800	21	7
			13,100	22	8	17,100	26	9
		NCRACKED HIGH STRENGTH	16,600	25	9	21,800	30	10
			19,700	28	10	25,200	33	11
			24,000	32	11	31,700	38	13
			07 100	05	10	25.200		

			WSW-AE	87/8 ANCHOR	BOLT	WSW-A	B1 ANCHOR E	BOLT
DESIGN CRITERIA	CONCRETE CONDITION	ETE ANCHOR ION STRENGTH	ASD ALLOWABLE TENSION (lb.)	W (in.)	d _e (in.)	ASD ALLOWABLE TENSION (lb.)	W (in.)	d _e (in.
			12,600	23	8	16,000	27	9
		STANDARD	13,100	24	8	17,100	29	10
CRACKED	HIGH	24,800	36	12	32,100	42	14	
		STRENGTH	27,100	38	13	35,300	45	15
SEISMIC			12,700	20	7	15,700	23	8
		STANDARD	13,100	21	7	17,100	25	9
	CRACKED	HIGH	24,600	31	11	32,500	37	13
		STRENGTH	27,100	34	12	35,300	39	13
		STANDARD	5,400	12	6	6,800	14	6
			8,300	16	6	11,600	20	7
			13,100	22	8	17,100	26	9
		RACKED HIGH STRENGTH	15,300	24	8	21,400	30	10
			19,300	28	10	25,800	34	12
			23,600	32	11	31,000	38	13
			27,100	36	12	35,300	42	14
WIND			6,800	12	6	6,800	12	6
		STANDARD	9,400	15	6	12,400	18	6
	UNCRACKED		13,100	19	7	17,100	23	8
		RACKED	16,800	22	8	21,600	26	9
			20,300	25	9	26,700	30	10
		STRENGTH	24,100	28	10	32,200	34	12
			27,100	31	11	35,300	36	12



