

REVISIONS:

SHEARWALL SCHEDULE ^(2, 4, 8, 11, 12, 13)						
SHEARWALL ^(7, 17)			ONE SIDED SHEARWALL TWO SIDED SHEARWALL ⁽⁶⁾		SEISMIC	WIND
MARK	MATERIAL ^(4, 14)	NAILING	UPPER FLOOR SILL PL CONN ^(1, 18)	ANCHOR BOLTS ⁽³⁾ (UNO ON FOUNDATION PLAN)	ALLOWABLE SHEAR	
P1	3/8" APA SHEATHING ⁽¹⁵⁾	8dN AT 6" OC EDGES, 12" FIELD ⁽¹⁶⁾	16dS AT 4" OC (STG'D) UNO	1/2" DIA X 10" AT 32" OC	260#/FT	365#/FT
P2	3/8" APA SHEATHING ⁽¹⁵⁾	8dN AT 4" OC EDGES, 12" FIELD ⁽¹⁶⁾	16dS AT 3" OC, (STG'D) UNO	1/2" DIA X 10" AT 24" OC	350#/FT	532#/FT
P3	3/8" APA SHEATHING ^(9, 15)	8dN AT 3" OC EDGES, 12" FIELD ⁽¹⁶⁾	1/4x6 SCREWS AT 4" OC (STG'D), UNO	1/2" DIA X 10" AT 16" OC	490#/FT	685#/FT
P4	3/8" APA SHEATHING ^(9, 15)	8dN AT 2" OC EDGES, 12" FIELD ⁽¹⁶⁾	1/4x6 SCREWS AT 4" OC (STG'D), UNO	1/2" DIA X 10" AT 12" OC	600#/FT	895#/FT
P5	15/32" APA SHEATHING ⁽¹⁰⁾	10dN AT 2" OC EDGES, 12" FIELD ⁽¹⁶⁾	1/4x6 SCREWS AT 3" OC (STG'D), UNO	1/2" DIA X 10" AT 9" OC	770#/FT	1078#/FT

SCHEDULE NOTES:

- SEE DETAIL 7/SD-2 FOR ADDTL INFO AT UPPER FLOOR SILL PLATE CONNECTION FOR MULTI-STORY PLANS ONLY. DOES NOT APPLY TO SINGLE STORY PLANS. SEE PLAN FOR SILL PLATE AND SHEAR CONNECTIONS AT EXTERIOR WALLS.
- OPENINGS IN SHEARWALL SHEATHING SHALL NOT EXCEED 8 INCHES IN ANY DIRECTION FOR A SINGLE OPENING OR THE SUM OF ANY TWO OR MORE OPENINGS ON COMMON OR OVERLAPPING VERTICAL OR HORIZONTAL LINES. OPENINGS NOT GREATER THAN 8 INCHES DO NOT REQUIRE BLOCKING AROUND THE PENETRATION. CONTACT THE ENGINEER OF RECORD FOR REQUIREMENTS AT OPENINGS NOT OTHERWISE DETAILED.
- MINIMUM (2) 1/2" DIA ANCHORS PER SHEAR WALL. SEE SECTION FOUNDATION HARDWARE, NOTE 2 ON THE GENERAL NOTES SHEET, S1. ALL ANCHOR BOLTS SHALL HAVE 3" x 3" x 0.229" PLATE WASHERS.
- APA RATED (STRUCTURAL II) PLYWOOD OR OSB.
- SEE DETAIL 11/SD-2 WHERE WALL FRAMING STEPS OR PERPENDICULAR WALL INTERSECTS SHEAR WALL.
- FOR SHEAR PANELS ON TWO SIDES OF WALL, USE ONE-HALF THE SPACING GIVEN IN THE SCHEDULE FOR SILL PLATE CONNECTION AND ANCHOR BOLT SPACING, UNO.
- DOUBLE SIDED SHEARWALLS SHALL HAVE VERTICAL PANEL JOINTS OFFSET TO FALL ON DIFFERENT STUDS OR USE SINGLE 3" NOMINAL STUDS (MIN) AT JOINTS. AT THE ENDS OF THE SHEARWALL, 4X NOMINAL MEMBERS ARE REQUIRED. NAILS ON EACH SIDE SHALL BE STAGGERED.
- ALL SHEARWALLS REQUIRE DOUBLE 2X TOP PLATES, U.N.O. AT NON-BEARING SHEAR WALLS, SHORTEN STUDS 1/4 INCH TO PROVIDE DEFLECTION CLEARANCE.
- P2, P3 AND P4 SHEARWALLS SHALL REQUIRE THE FOLLOWING:
 - STAGGER NAILING ALONG PLYWOOD JOINTS AND SILL PLATES.
 - SINGLE 3" NOMINAL MEMBERS AT ALL FRAMING MEMBERS RECEIVING EDGE NAILING FROM ABUTTING PANELS. 3" NOMINAL MEMBERS AT SINGLE SIDED SHEARWALL MAY BE CONSTRUCTED W/ (2) 2X MEMBERS FASTENED TOGETHER W/ (2) ROWS OF 16d SINKERS AT 4" OC.
- P5 SHEARWALLS SHALL REQUIRE SINGLE 3" NOMINAL MEMBERS AT ALL FRAMING MEMBERS RECEIVING EDGE NAILING FROM ABUTTING PANELS (MIN), STAGGER JOINT AND SILL PLATE NAILING.
- ALL SHEARWALL LENGTHS NOTED ON PLAN ARE MINIMUM REQUIRED AND MAY BE INCREASED WITHOUT REVIEW.
- SHEATHING MAY BE PLACED ON EITHER FACE OF DESIGNATED WALL, UNO.
- ALLOWABLE SHEAR CAPACITIES ARE IN ACCORDANCE WITH AF&PA SDPWS TABLE 4.3A WITH APPLICABLE OMEGA FACTORS INCLUDED.
- APA SHEATHING AND GYPSUM SHEATHING MAY BE INSTALLED WITH THE LONG OR SHORT DIRECTION PERPENDICULAR TO THE FRAMING. WHERE APA SHEATHING IS INSTALLED WITH THE SHORT DIRECTION PERPENDICULAR TO THE FRAMING, THE FRAMING MUST BE 16" ON CENTER (MAX). WHERE GYPSUM SHEATHING IS INSTALLED WITH THE SHORT DIRECTION PERPENDICULAR TO THE FRAMING ALL PANEL EDGES MUST BE BLOCKED AND NAILED.
- 3/8" SHEATHING MAY BE REPLACED WITH 7/16" OR 15/32" SHEATHING WITHOUT ADDITIONAL REVIEW.
- ALL VERTICAL AND HORIZONTAL PANEL EDGES TO BE BLOCKED AND NAILED.
- STUDS SHALL BE 24" OC (MAX). FOR 3/8" AND 7/16" SHEATHING, FIELD NAILING SHALL BE REDUCED TO 6" OC WHERE STUD SPACING IS GREATER THAN 16" OC.
- SILL PLATE CONNECTORS SHALL BE PER SHEAR TRANSFER DETAIL SPECIFIED ON PLANS.

HOLDOWN/STRAP SCHEDULE ^(1, 2)		
HD/STRAP	EMBED AT FND AND / OR ANCHOR BOLT	CONN TO (2) 2X STUD, UNO ^(3, 6, 10)
CS16	N/A	EXTEND STRAP 16" MIN. EA. END W/ (13) 8dN TO (2) 2X STUD ABOVE AND BELOW FLOOR FRAMING
(2) CS16	N/A	EXTEND STRAP 16" MIN. EA. END W/ (13) 8dN TO (2) 2X STUD ABOVE AND BELOW FLOOR FRAMING
CMSTC16	N/A	EXTEND STRAP 25" MIN. EA. END W/ (28) 16d SINKERS TO (2) 2X STUD ABOVE AND BELOW FLOOR FRAMING
CMST14	N/A	EXTEND STRAP 32" MIN. EA. END W/ (33) 16dN TO (3) 2X STUD ABOVE AND BELOW FLOOR FRAMING
CMST12	N/A	EXTEND STRAP 40" MIN. EA. END W/ (42) 16dN TO (3) 2X STUD ABOVE AND BELOW FLOOR FRAMING
LSTHD6 ^(4, 11)	8" EMBED	(20) 16d SINKERS
STHD10 ^(4, 11)	10" EMBED	(24) 16d SINKERS
STHD14 ^(4, 11)	14" EMBED	(30) 16d SINKERS
HTT5	SSTB24 W/ 21" EMBED ⁽²⁾	(26) 16dN X 2 1/2" NAILS
HU2	SSTB24 W/ 21" EMBED ⁽²⁾	(6) SDS 1/4 X 2 1/2 SCREWS W/ MIN (2) 2X POSTS
HU4	SSTB24 W/ 21" EMBED ⁽²⁾	(10) SDS 1/4 X 2 1/2 SCREWS W/ MIN (2) 2X POSTS
HU5	SSTB24 W/ 21" EMBED ⁽²⁾	(14) SDS 1/4 X 2 1/2 SCREWS W/ MIN (2) 2X POSTS
HU8	SSTB34 W/ 29" EMBED ⁽²⁾	(20) SDS 1/4 X 2 1/2 SCREWS W/ MIN (3) 2X POSTS
HU11	1" DIA AB W/ 9" MIN EMBED W/ MIN 28" SQ X 14" DEEP FTG ⁽⁹⁾	(30) SDS 1/4 X 2 1/2 SCREWS W/ MIN 4X8 POST ⁽⁹⁾
HU14	1" DIA AB W/ 10" MIN EMBED W/ MIN 30" SQ X 15" DEEP FTG ⁽⁷⁾	(36) SDS 1/4 X 2 1/2 SCREWS W/ MIN 4X8 POST ⁽⁹⁾

SCHEDULE NOTES:

- HD/STRAP SHALL BE SIMPSON OR EQUAL W/ ICC APPROVAL. ALL SUBSTITUTES SHALL BE REVIEWED BY THE ENGINEER OF RECORD BEFORE INSTALLATION.
- FIXED-LENGTH STRAPS SHALL BE INSTALLED WITH AN EQUAL LENGTH OVERLAPPING CONNECTED MEMBERS AND AN EQUAL NUMBER OF FASTENERS IN EACH MEMBER.
- STITCH NAIL EACH STUD AT MULTIPLE 2x STUDS TOGETHER WITH 16d SINKERS AT: 4" OC FOR P3 AND P4 SHEAR WALLS. 6" OC FOR ALL OTHER SHEAR WALLS.
- FOR CONCRETE SPALLS LESS THAN 4", THERE IS NO LOAD REDUCTION AND NO FURTHER REVIEW BY EOR IS REQUIRED.
- SEE DETAIL 4/SD-2 FOR ADDL CRITERIA AT UPPER FLOOR STRAPS (WHERE OCCURS).
- EDGE NAIL SHT'G TO EA MEMBER OF MULTIPLE POST, OFFSET 1/2 SPACING BTWN MEMBERS.
- ASTM F1554-55 BOLT W/ HEAVY SQUARE NUT OR 1/4 X 1 3/4 X 1 3/4 PLATE WASHER REQUIRED FOR FULL LOAD. REDUCE ALLOWABLE LOAD TO 13180 LBS FOR ASTM GRADE 36 BOLT. MINIMUM EMBEDMENT IS FROM TOP OF FOOTING.
- ASTM GRADE 36 BOLT W/ SQUARE OR HEAVY HEX HEAD OR NUT REQUIRED. MINIMUM EMBEDMENT IS FROM TOP OF FOOTING.
- PROVIDE 6x8 POST AT 2x6 WALLS, MULTIPLE STUDS NOT ALLOWED.
- END POST TO BE FULL HEIGHT MEMBERS, UNO.
- STRAPS MAY BE PLACED ON EITHER FACE OF DESIGNATED WALL AND ARE NOT REQUIRED TO OCCUR ON SAME FACE AS SHEATHING, UNO.
- AT GARAGE STEMWALL LOCATIONS USE SSTBL.
- WHEN SLAB AND FOOTINGS ARE PLACED AS A MONO-POUR, SSTB28 WITH 25" EMBEDMENT MAY BE SUBSTITUTED FOR THE SSTB34 SPECIFIED.

FOUNDATION NOTES:

- AT INTERIOR BEARING WALLS, WITHOUT DEEPEENED FOOTINGS, USE 1/2" DIA TITEN HD HIGH STRENGTH SCREW ANCHORS IN LIEU OF ANCHOR BOLTS.
- SEE DETAIL 1/SD-1 FOR PERIMETER FOOTING EMBEDMENT DEPTH.
- VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

FRAMING NOTES:

- ALL EXTERIOR WALLS TO BE MIN 2x6 AT 16" OC DFL STUD GRADE AND INTERIOR BEARING AND SHEAR WALLS TO BE MIN 2x4 AT 16" O.C. DFL STUD GRADE, UNO. SEE FRAMING PLANS FOR NON-TYPICAL STUD SIZE AND SPACING.
- TRIMMER / KING STUD SCHEDULE, UNO.

	OPENING SPAN (L)	TRIMMERS	KING STUDS
2x4 WALLS	L < 6'-0"	1	1
	6'-0" L < 10'-0"	2	2
	10'-0" L < 18'-0"	2	3
2x6 WALLS	L < 8'-0"	1	1
	8'-0" L < 12'-0"	2	2
	12'-0" L < 20'-0"	2	3
- BLOCKED DIAPHRAGM - SEE STRUCTURAL GENERAL NOTES SHEET S1.
- FOR TYPICAL OVERFILL FRAMING WHERE REQUIRED BY TRUSS SHOP DRAWINGS, SEE DETAILS 4/SD-3 OR 5/SD-3.
- INTERIOR BEARING WALLS
- BEAM AND HEADER SIZES INDICATED ON THIS PLAN ARE MINIMUM. LARGER SIZES OR HIGHER GRADE LUMBER MAY BE SUBSTITUTED.
- TOP PLATE SPLICES PER DETAIL 3/SD-2, UNO.
- SEE DETAIL 8/SD-2 FOR ADDITIONAL FRAMING REQUIREMENTS.

LEDGER & HANGER SCHEDULE:

UNLESS OTHERWISE NOTED 2X LEDGERS WHERE DETAILED SHALL BE AS FOLLOWS:

	SPAN (L)	TRUSS		LEDGER AND NAILING ^(1, 2A, 10, 11)	MIN HANGER, UNO ^(3, 4, 5, 11)
		SPACING (MAX)			
ROOF LOAD	L 8'-0"	16' O.C.	2x6 W/ (3) 16d AT 16" OC	LUS24/JUS24	
		24" O.C.	2x8 W/ (5) 16d AT 24" OC	LUS24/JUS24	
ROOF LOAD	L 16'-0"	24" O.C.	2x8 W/ (5) 16d AT 16" OC	LUS26/JUS26	
			2x12 W/ (8) 16d AT 24" OC	LUS26/JUS26	
ROOF LOAD	L 24'-0"	24" O.C.	2x12 W/ (8) 16d AT 16" OC	LUS28/JUS28	
			2x12 W/ 2 COLUMNS OF (8) 16d AT 24" OC	HUS28/HUS28	
FLOOR LOAD	L 10'-0"	16' O.C.	2x8 W/ (5) 16d AT 16" OC	LUS46/JUS46	
		24" O.C.	2x10 W/ 2 COLUMNS OF (6) 16d AT 24" OC	LUS46/JUS46	
FLOOR LOAD	L 20'-0"	24" O.C.	2x10 W/ 2 COLUMNS OF (6) 16d AT 16" OC	LUS48/JUS48	
		24" O.C.	2x12 W/ 2 COLUMNS OF (8) 16d AT 24" OC	HUS48/HUS48	
FLOOR LOAD	L 10'-0"	16' O.C.	2x8 W/ (5) 16d AT 16" OC	LUS26/JUS26	
		24" O.C.	2x10 W/ 2 COLUMNS OF (6) 16d AT 24" OC	LUS26/JUS26	
FLOOR LOAD	L 20'-0"	24" O.C.	2x10 W/ 2 COLUMNS OF (6) 16d AT 16" OC	LUS28/JUS28	
		24" O.C.	2x12 W/ 2 COLUMNS OF (8) 16d AT 24" OC	HUS28/HUS28	
DECK LOAD	L 10'-0"	16' O.C.	2x10 W/ (2) 1/4"x3" SCREWS AT 16" OC	LUS26/JUS26	
		24" O.C.	2x10 W/ 2 COLUMNS OF (2) 1/4"x3" SCREWS AT 24" OC	LUS26/JUS26	
DECK LOAD	L 15'-0"	24" O.C.	2x10 W/ 2 COLUMNS OF (2) 1/4"x3" SCREWS AT 16" OC	LUS28/JUS28	
		24" O.C.	2x10 W/ 2 COLUMNS OF (3) 1/4"x3" SCREWS AT 24" OC	HUS28/HUS28	

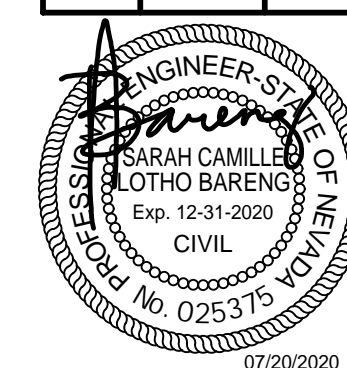
SCHEDULE NOTES:

- TWO COLUMNS OF FASTENERS REQUIRE MIN SUPPORTING MEMBER RECEIVING FASTENERS OF 3" OR (2)2X IN WIDTH (ENDS OF 4X2 OR DOUBLE TRUSSES ARE ACCEPTABLE). SPACE FASTENERS MIN 1" APART IN EACH DIRECTION.
- SPACING SHOWN EQUALS SPACING OF FRAMING MEMBERS RECEIVING FASTENERS.
- HANGERS LISTED IN ORDER ARE BY SIMPSON STRONG-TIE AND USP, RESPECTIVELY.
- LISTED HANGERS ARE MINIMUM HANGERS REQUIRED WHERE NOT OTHERWISE NOTED ON THE STRUCTURAL DRAWINGS.
- SPACING SHOWN FOR HANGERS IS SPACING OF THE FRAMING MEMBER SUPPORTED BY THE HANGER.
- LEDGER MATERIAL SHALL BE DFL #2 OR BETTER UNO. LEDGER AT DECK SHALL BE PRESSURE-PRESERVATIVE TREATED OR NATURALLY DURABLE WOOD.
- PROVIDE THA218 (MIN) HANGER FOR ROOF TRUSS TO BEAM CONNECTIONS FOR ALL BEAM DEPTHS GREATER THAN 10 INCHES.
- PROVIDE THA418 (MIN) HANGER FOR FLOOR TRUSS TO BEAM CONNECTIONS FOR ALL BEAM DEPTHS GREATER THAN 10 INCHES.
- PROVIDE THA422 (MIN) HANGER FOR FLOOR TRUSS TO BEAM CONNECTIONS FOR ALL BEAM DEPTHS GREATER THAN 18 INCHES.
- ALL SIMPSON SCREWS NOTED SHALL BE STRONG DRIVE SDS SCREWS.
- ALL USP SCREWS NOTED SHALL BE WS SCREWS.
- HANGERS AND FASTENERS WITH EXTERIOR EXPOSURE SHALL BE STAINLESS STEEL, UNLESS NOTED OTHERWISE.

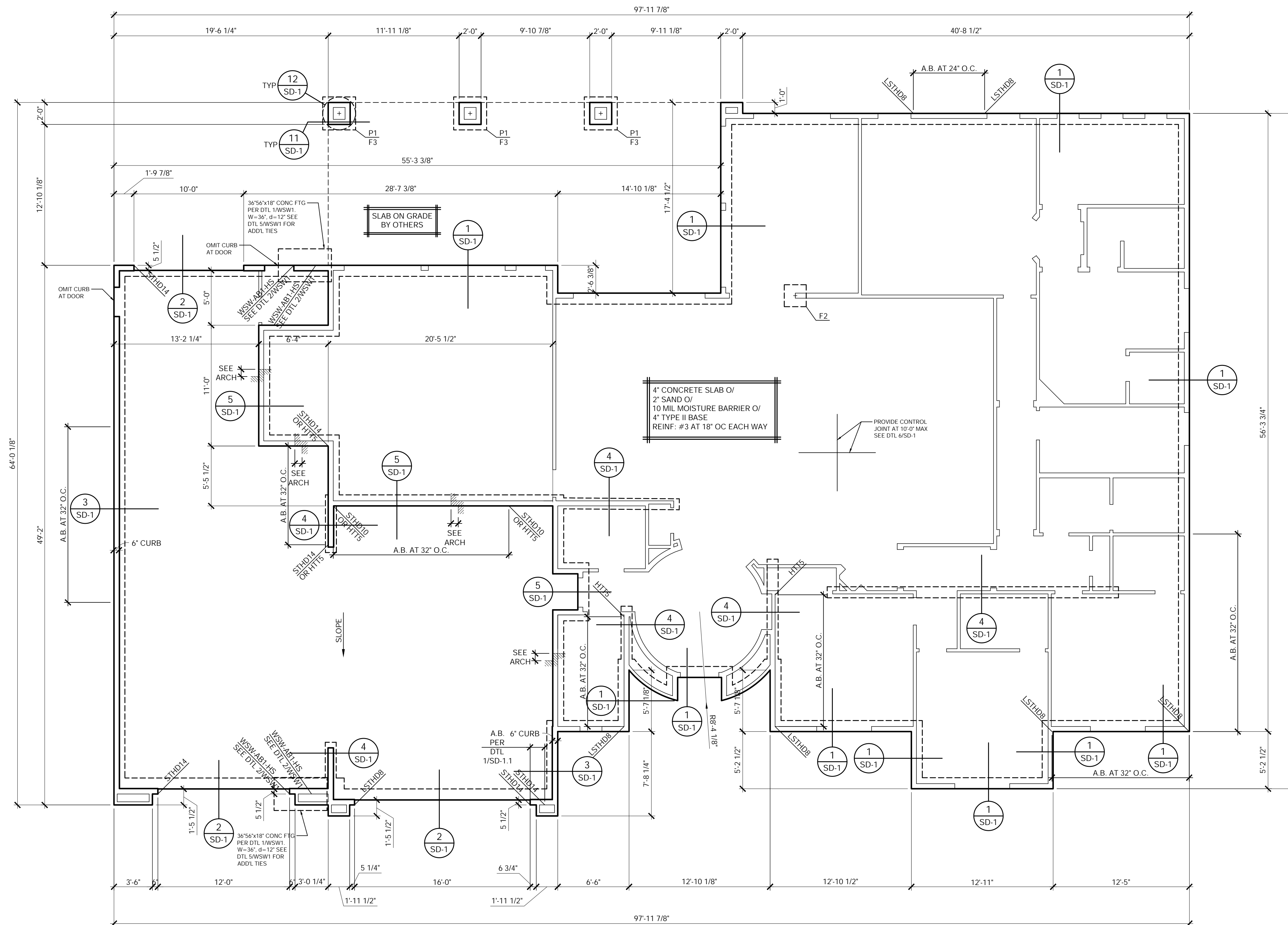
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CIVIL FORENSICS PLANNING STRUCTURAL SURVEY

ASSURED DEVELOPMENT
 ATHENS LOT 2
 HENDERSON, NV



07/20/2020
S1.1



FOOTING AND PIER SCHEDULE			
F.G. = FINISHED GRADE C.G. = COMPACTED SUB-GRADE			
MARK	SIZE L x W x D	REINFORCEMENT	REMARKS
P1	NOTE 1	(4) #4 DOWELS W/ 6\"/>	NOTE 2
F1	L x W x 1'-0"	#4 AT 12" O.C. EA WAY BOTT	NOTE 6
F2	2'-0" SQ x 1'-0"	(2) #4 EA WAY, BOTT	
F2.5	2'-6" SQ x 1'-0"	(3) #4 EA WAY, BOTT	
F3	3'-0" SQ x 1'-0"	(3) #4 EA WAY, BOTT	
F4	4'-0" SQ x 1'-0"	(5) #4 EA WAY, BOTT	

FOOTING AND PIER NOTES:

1. SIZE TO MATCH ARCH'L BOXED COLUMN (SEE DETAIL 11/SD-1)
2. TOP OF PIER = FINISHED FLOOR
3. TIES SHALL HAVE 1 1/2" COVER & BE LOCATED 2" FROM TOP OF PIER
4. EMBEDMENT BELOW COMPACTED SUB-GRADE PER DETAIL 1/SD-1
5. SEE DETAIL 4/SD-1 FOR MINIMUM EMBEDMENT DEPTH FROM TOP OF SLAB OR LOWEST ADJACENT COMPACTED SUBGRADE
6. SIZE TO MATCH BOX COLUMN + 6" EA. SIDE

JOB NO: 1939-006-201
 DESIGNED BY: SLB
 DRAWN BY: ACM
 ISSUED FOR:
 CONSTRUCTION DOCUMENTS
 DATE: 07-09-20
 SHEET TITLE:
 FOUNDATION PLAN
 REVISIONS:

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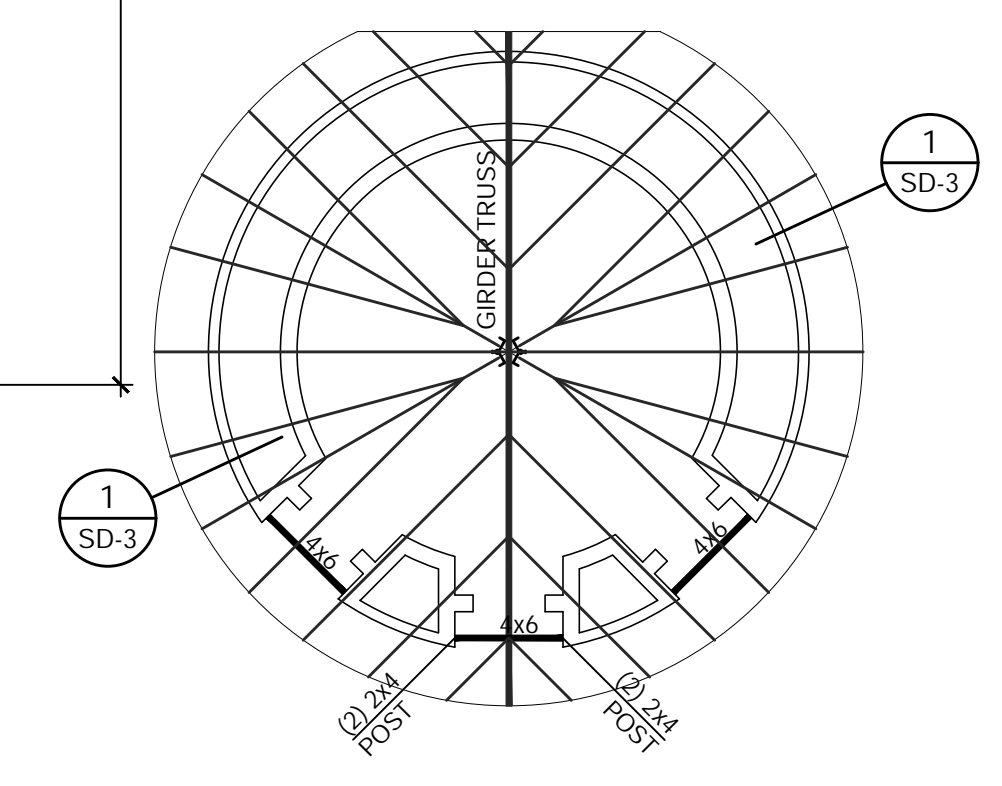
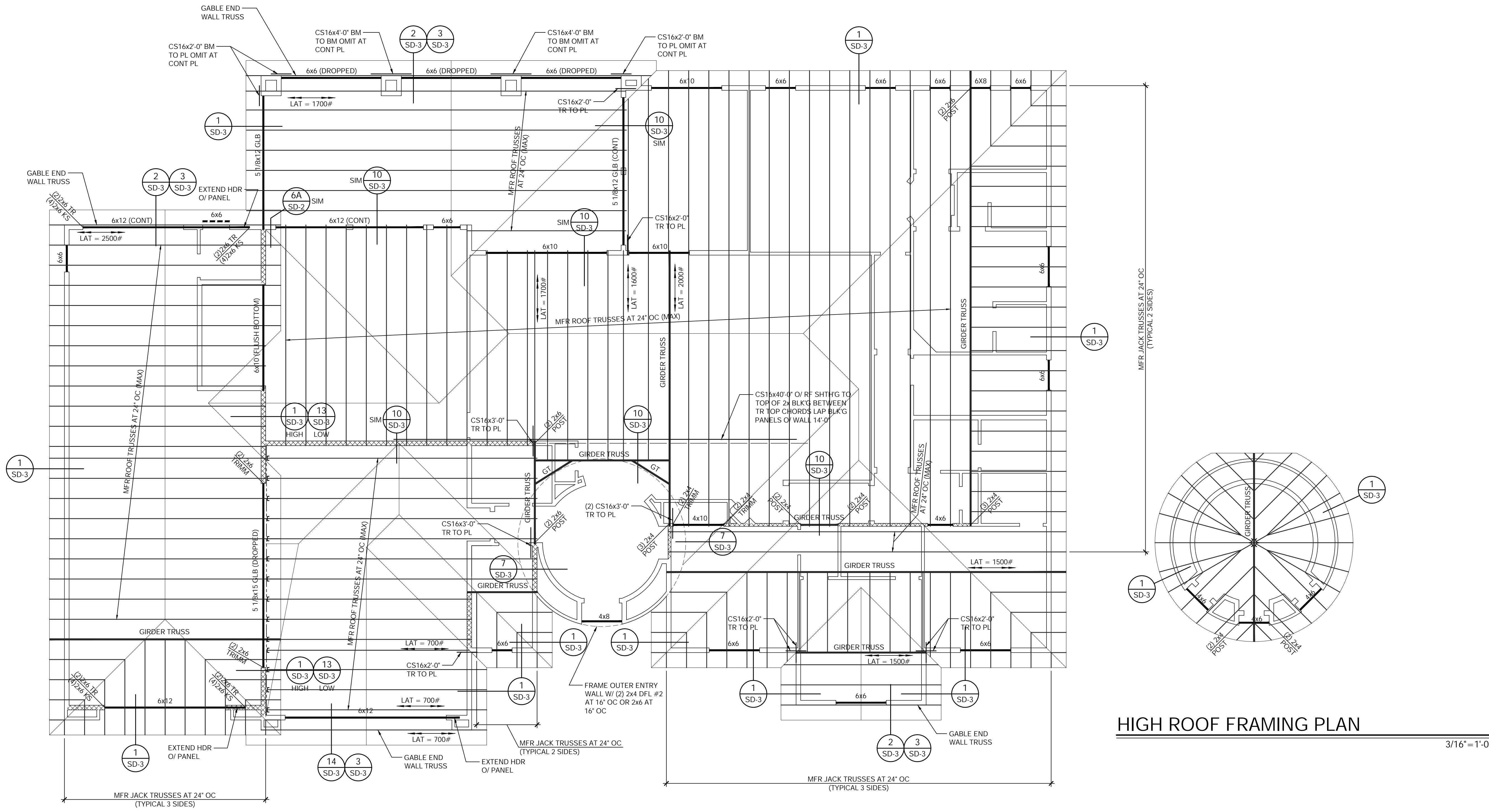
ASSURED DEVELOPMENT
 ATHENS LOT 2
 HENDERSON, NV



FOUNDATION PLAN

3/16" = 1'-0"

SEE SHEET S1.1 FOR FOUNDATION NOTES AND HOLDOWN SCHEDULE



ROOF FRAMING PLAN
 3/16" = 1'-0"

HIGH ROOF FRAMING PLAN
 3/16" = 1'-0"

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ASSURED DEVELOPMENT
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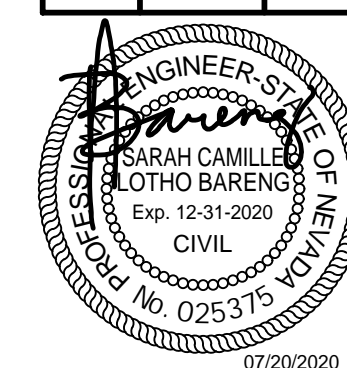
S3

SEE SHEET S1.1 FOR FRAMING NOTES

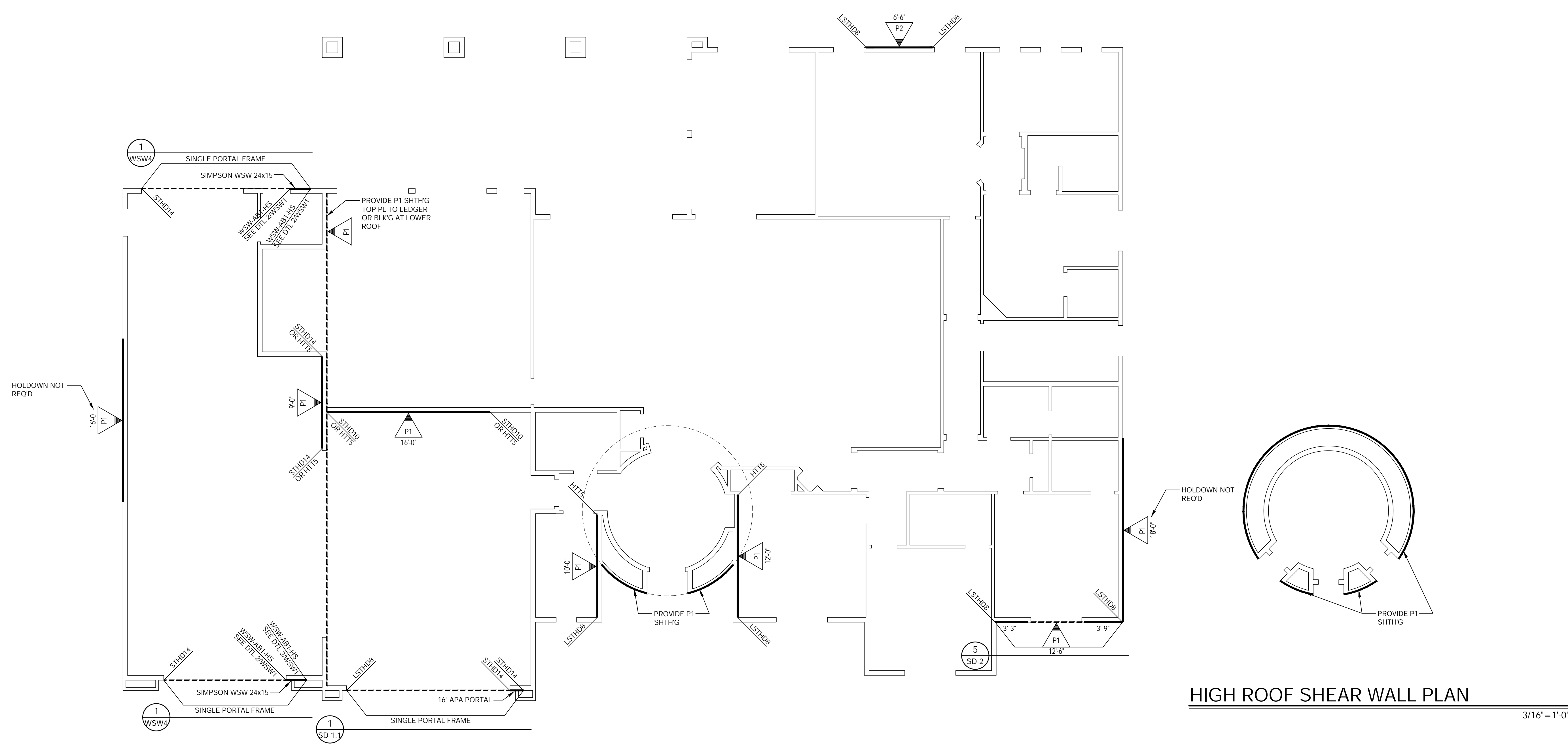
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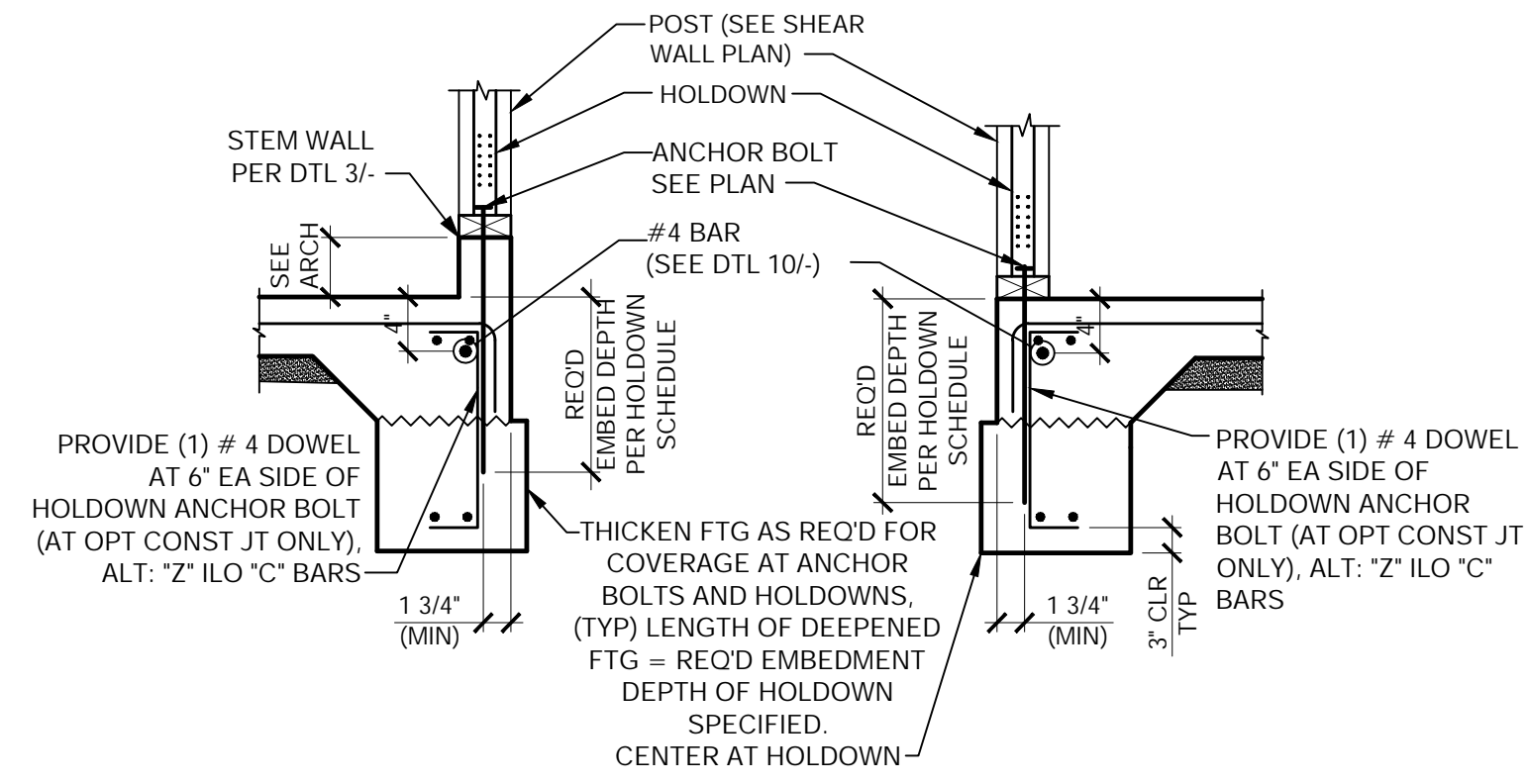
S4



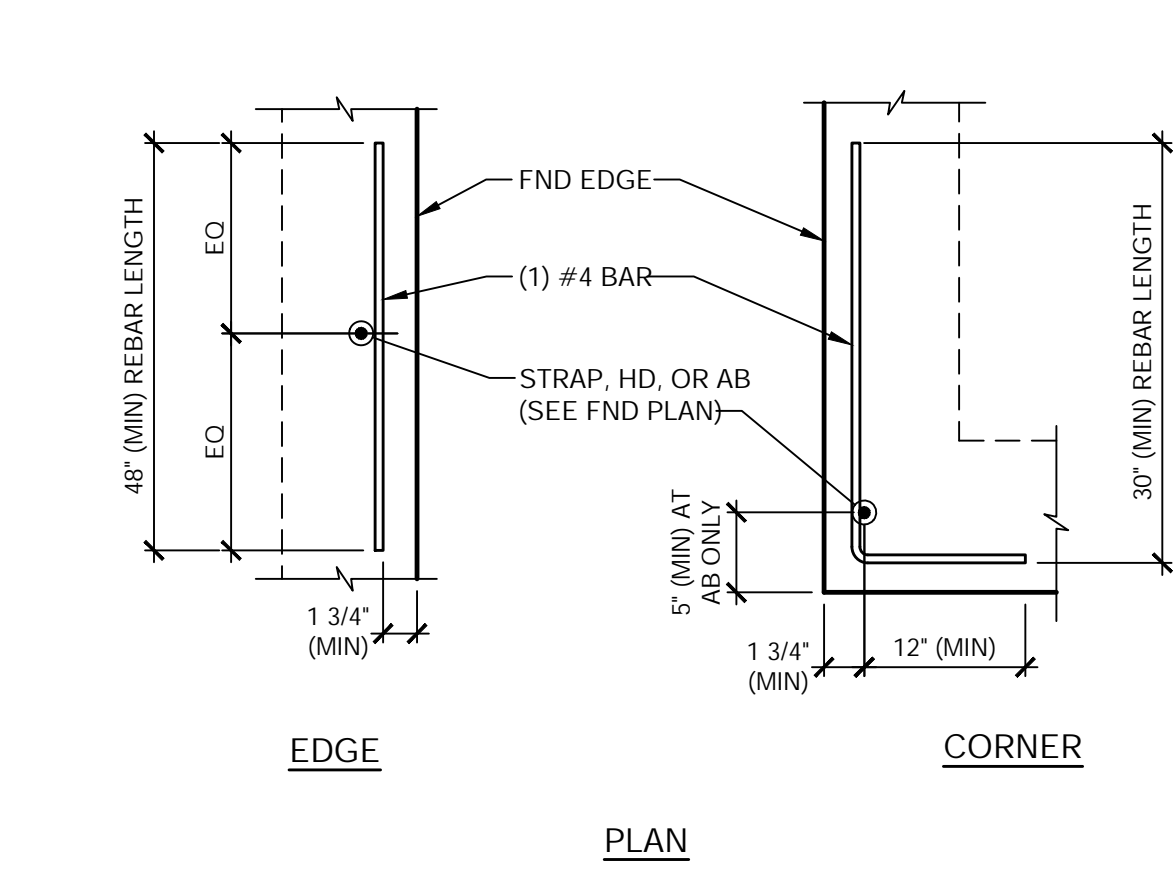
SHEAR WALL PLAN
 3/16" = 1'-0"

HIGH ROOF SHEAR WALL PLAN
 3/16" = 1'-0"

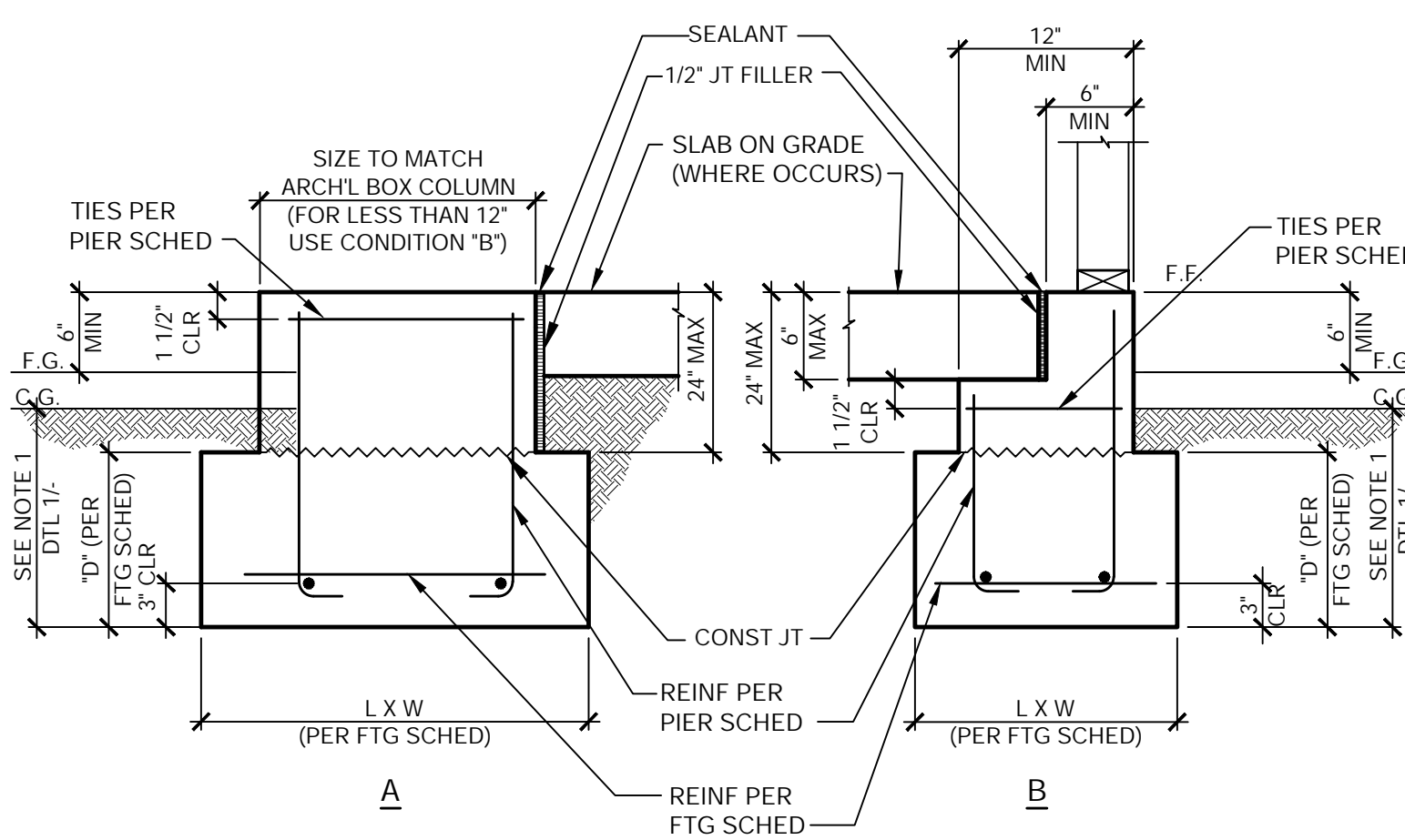
SEE SHEET S1.1 FOR SHEAR WALL AND HOLDOWN SCHEDULES



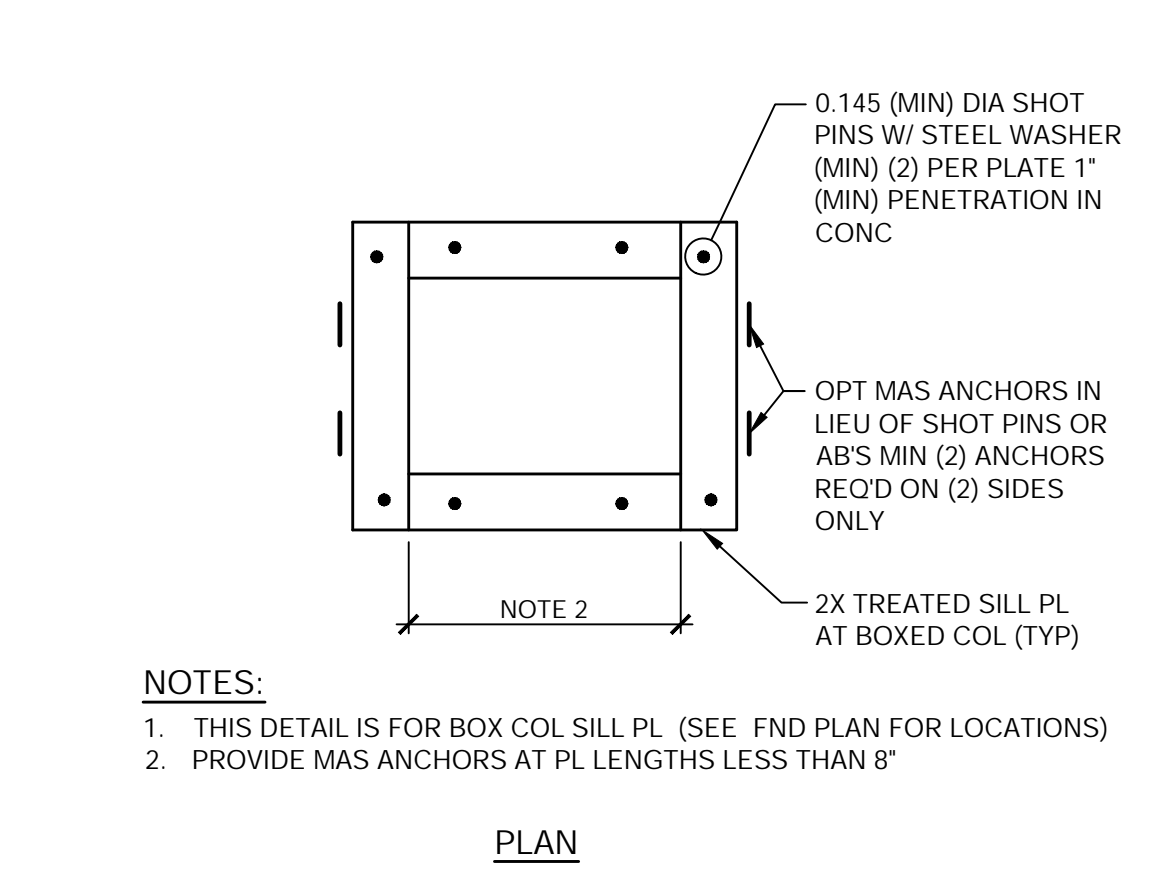
SEE DTL 1/- FOR INFORMATION NOT SHOWN
9 HOLDOWN W/ ANCHOR BOLT (TYPICAL) NTS



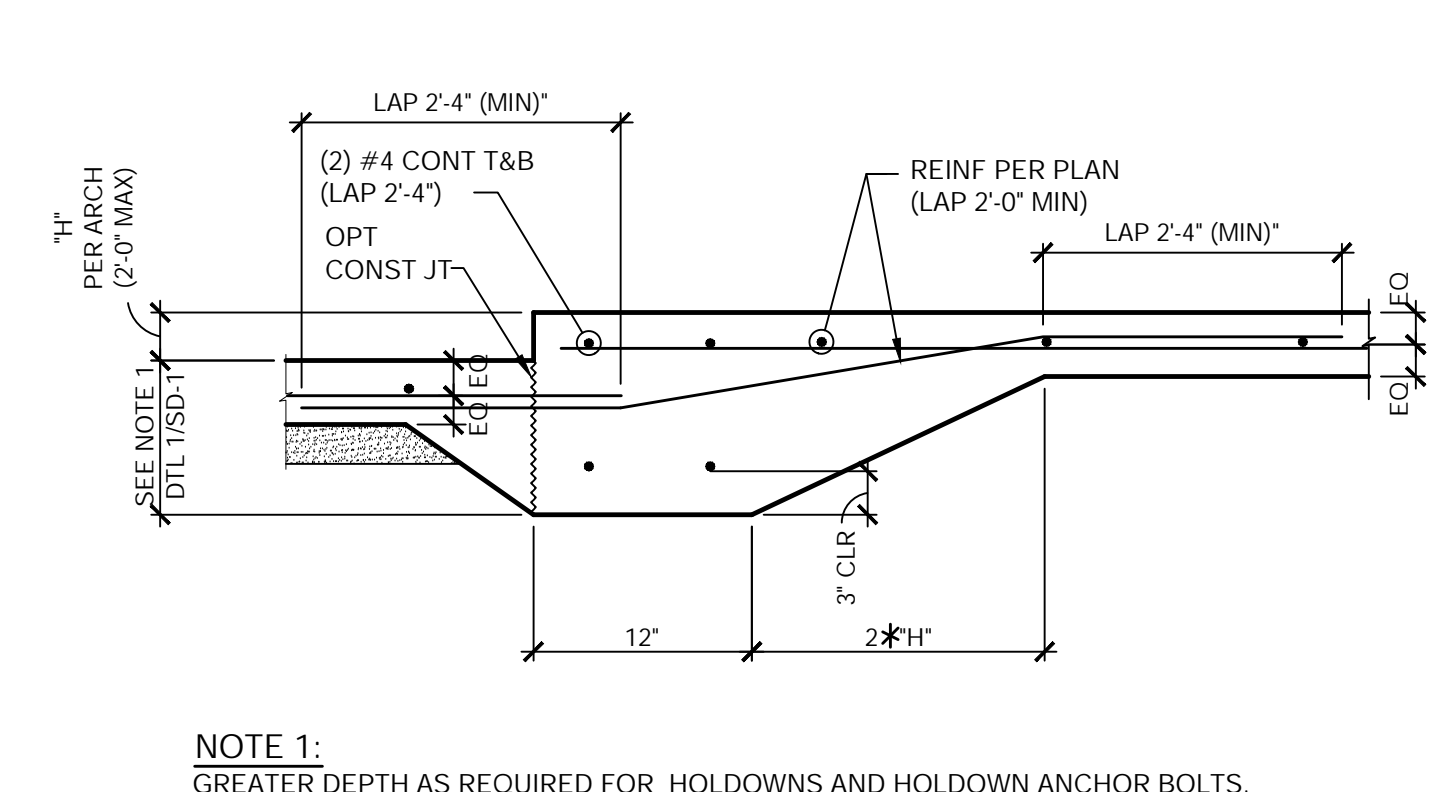
10 REINFORCING AT HOLDOWNS NTS



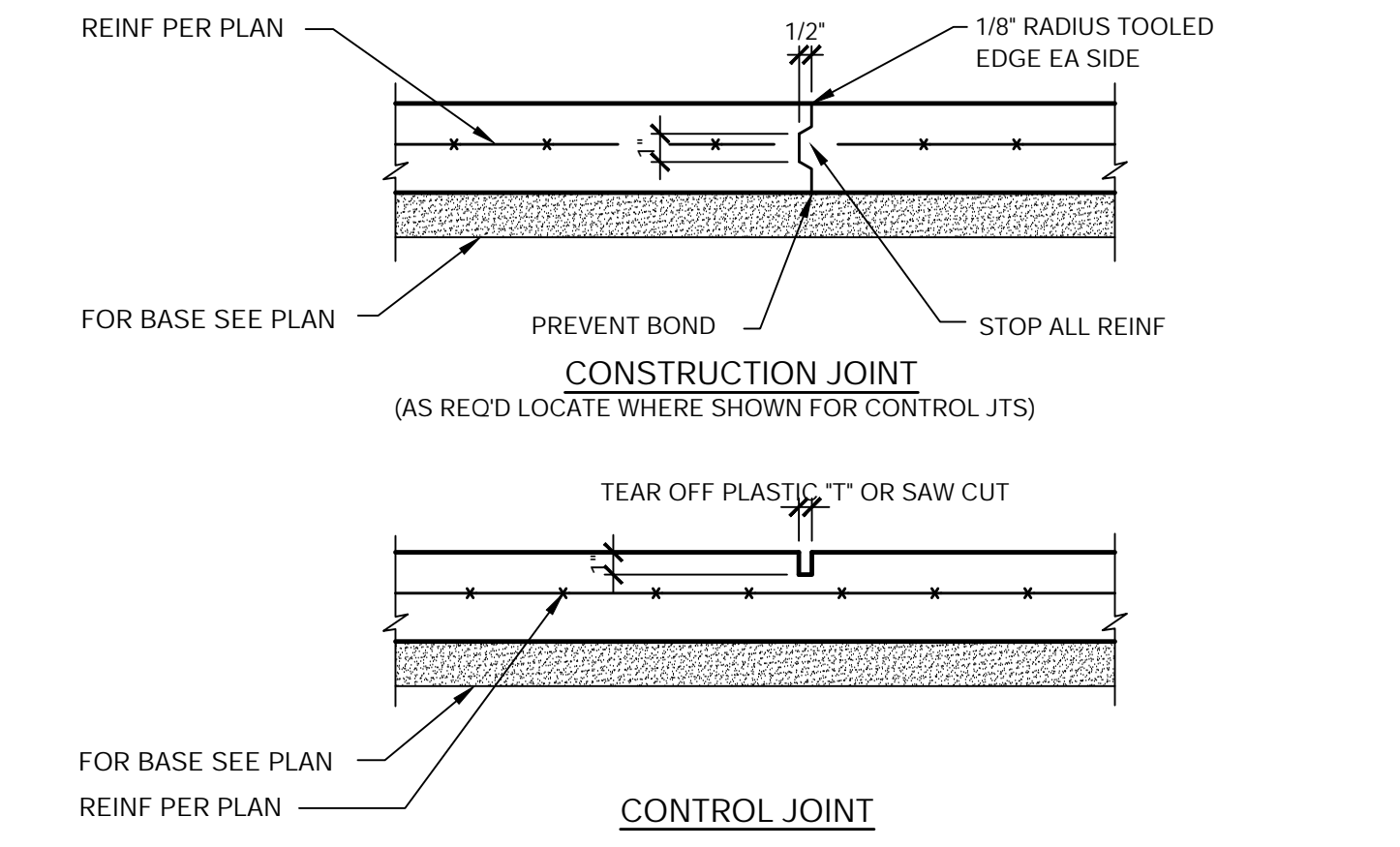
11 CONCRETE PIER AND FOOTING NTS



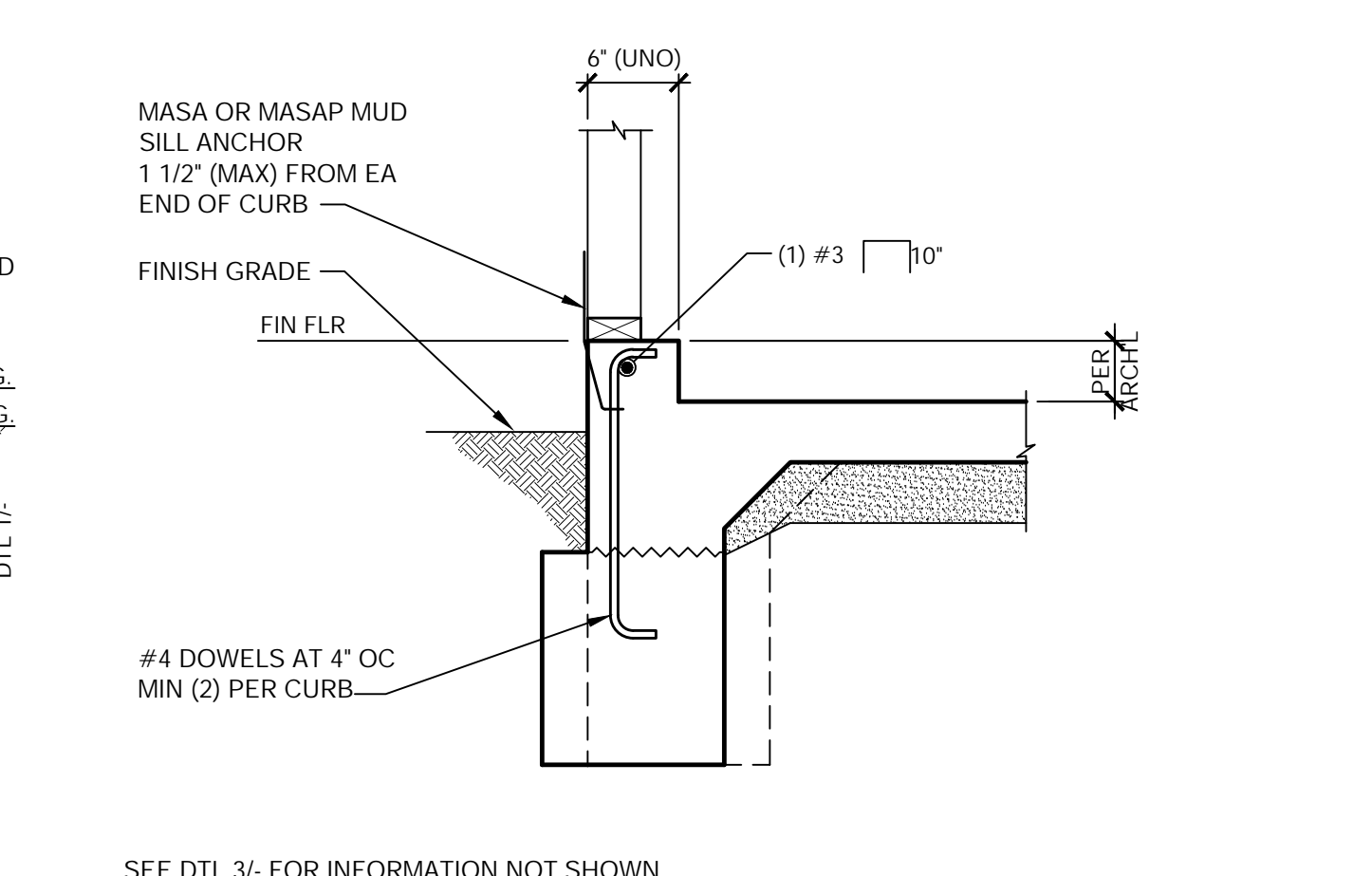
NOTES:
 1. THIS DETAIL IS FOR BOX COL SILL PL (SEE FND PLAN FOR LOCATIONS)
 2. PROVIDE MAS ANCHORS AT PL LENGTHS LESS THAN 8'
12 BOX COLUMN SILL PLATE NTS



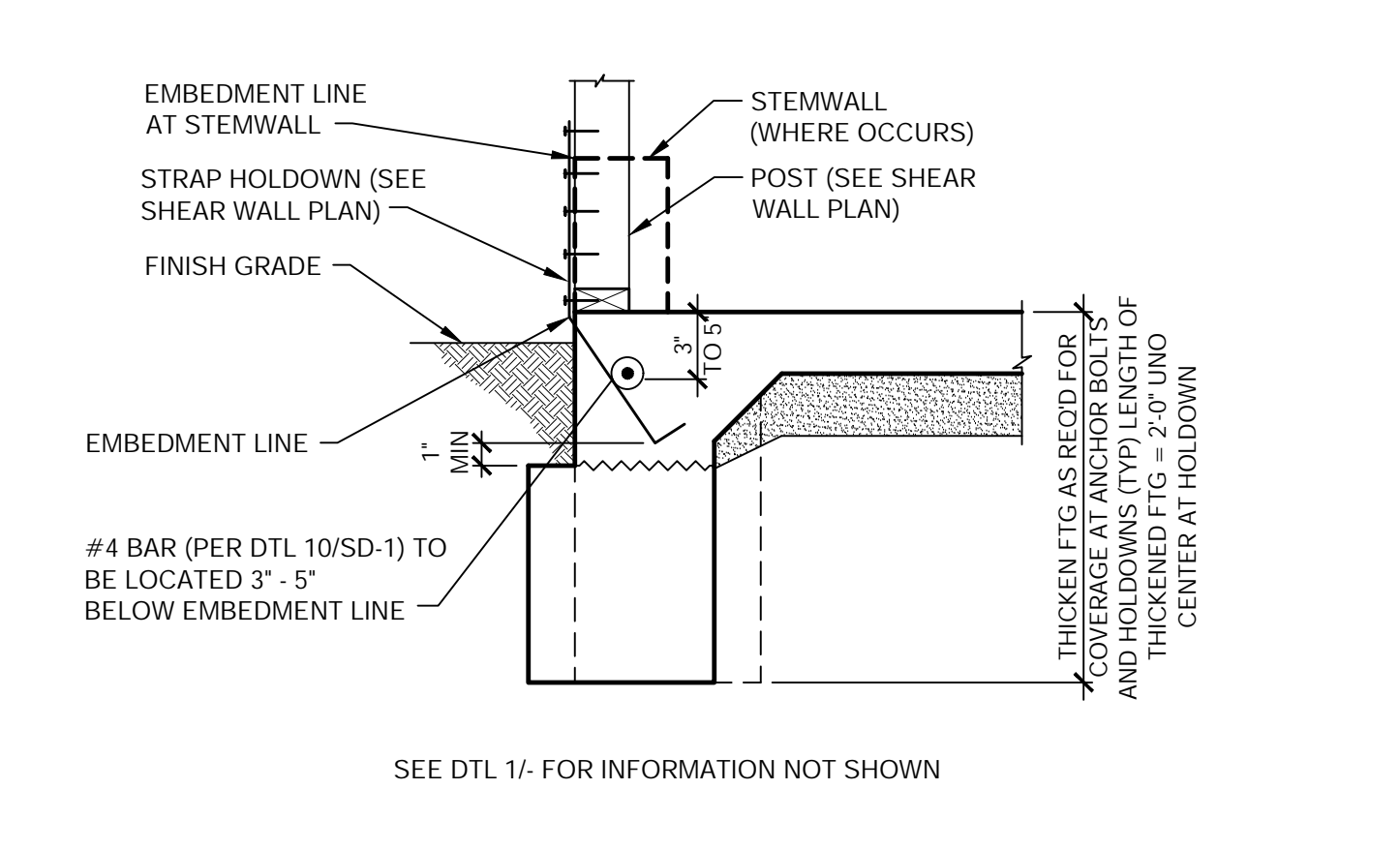
NOTE 1:
 GREATER DEPTH AS REQUIRED FOR HOLDOWNS AND HOLDOWN ANCHOR BOLTS.
5 STEP IN SLAB NTS



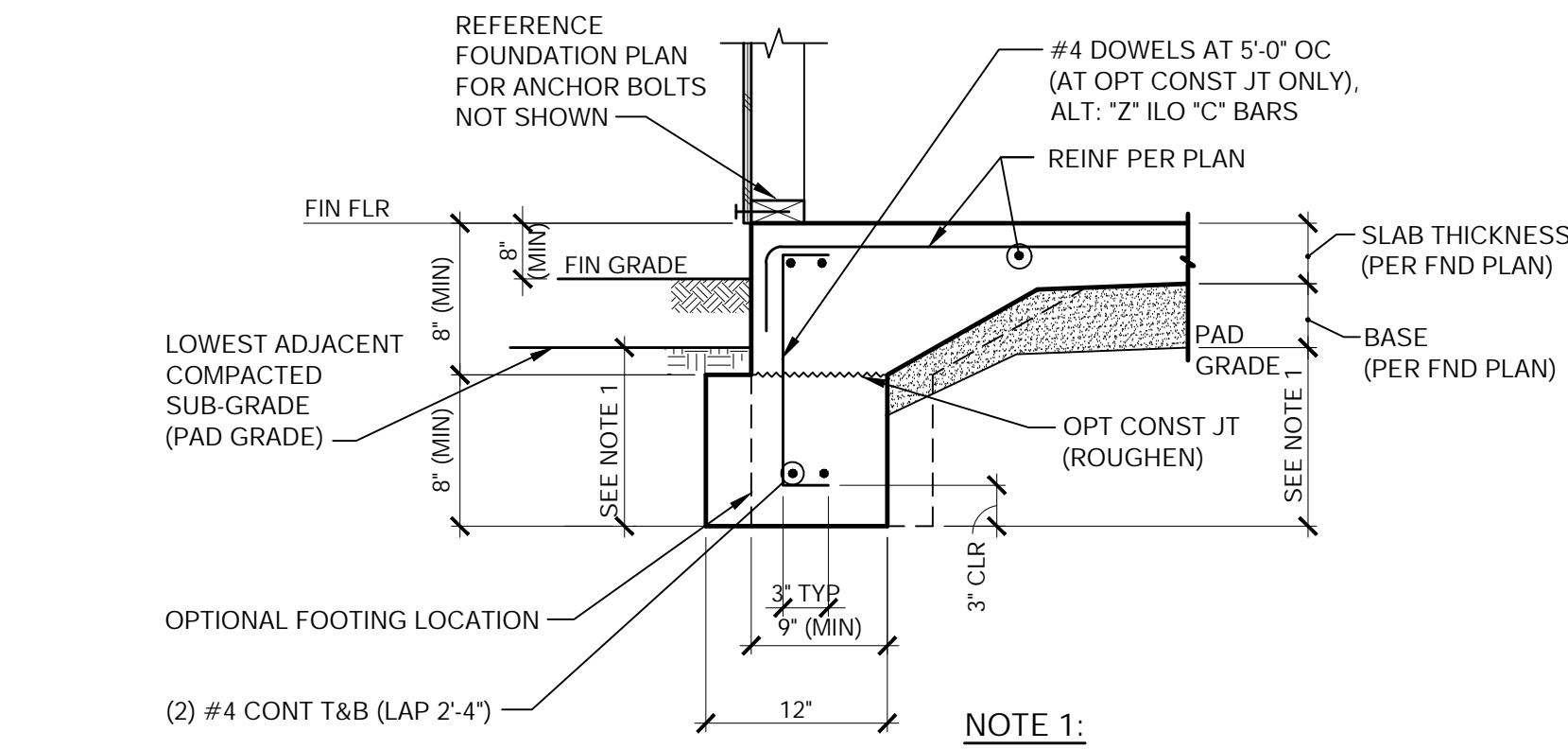
6 TYP SLAB JOINTS NTS



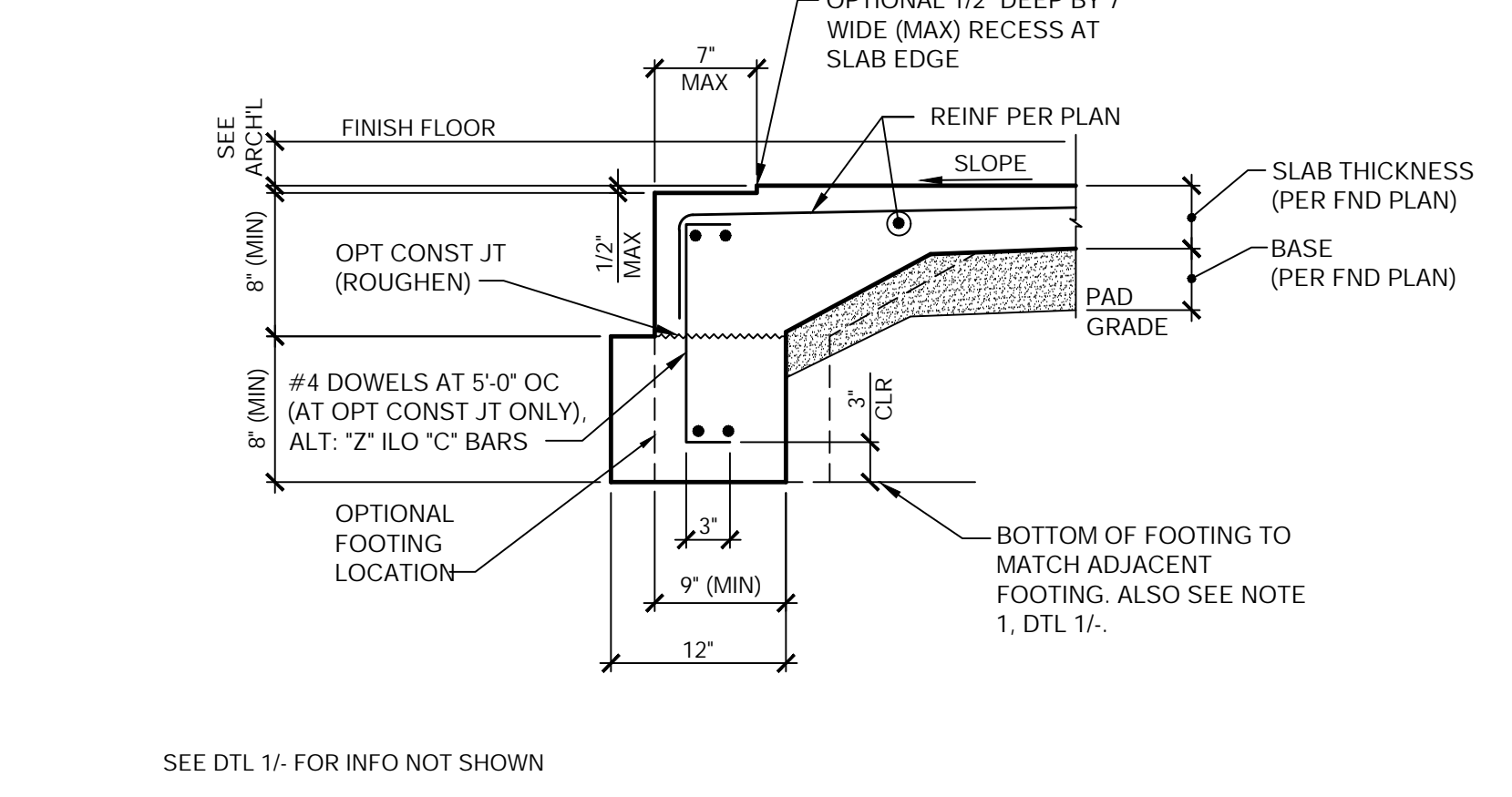
SEE DTL 3/- FOR INFORMATION NOT SHOWN
7 CURB BETWEEN GARAGE DOORS NTS



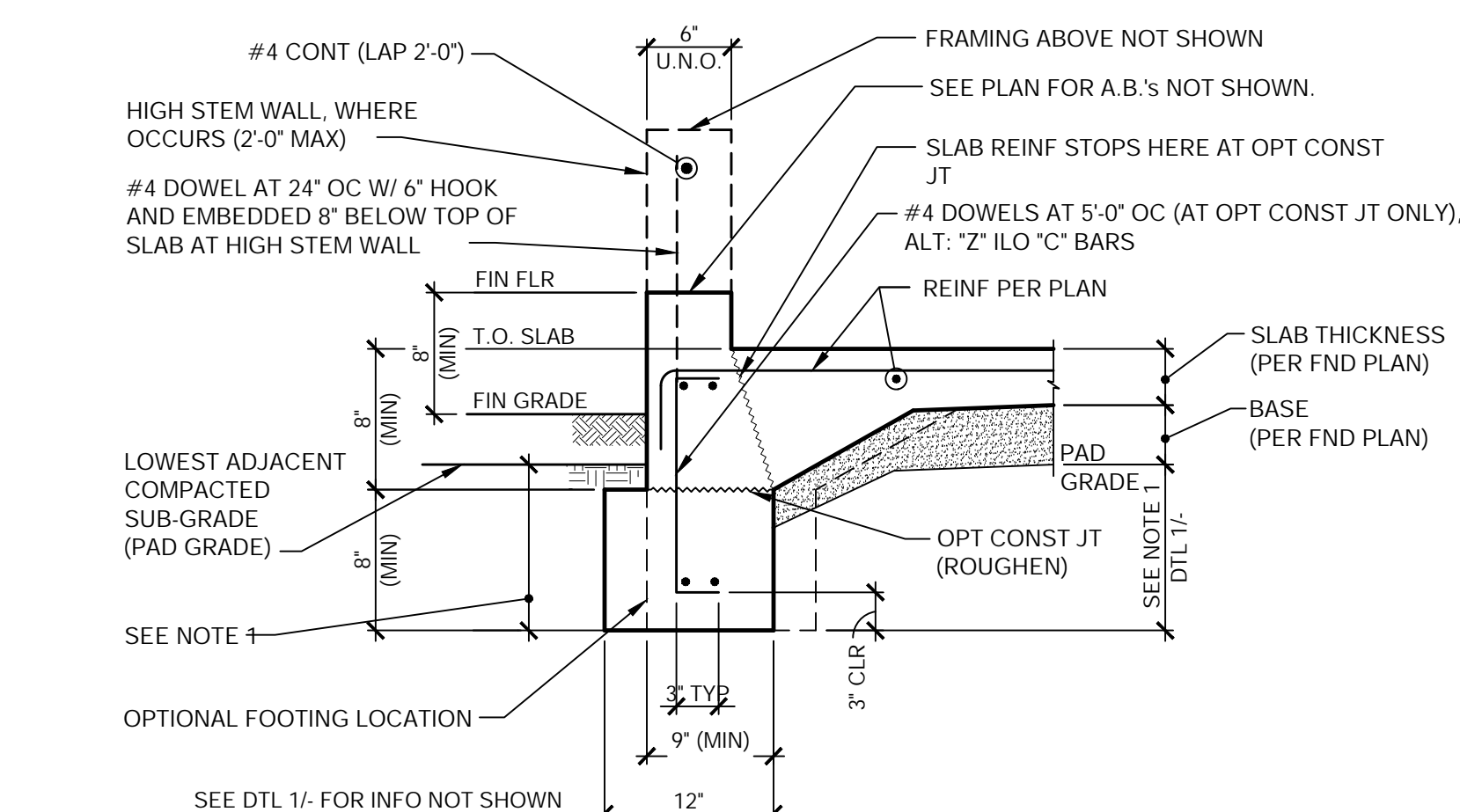
SEE DTL 1/- FOR INFORMATION NOT SHOWN
8 STRAP HOLDOWN NTS



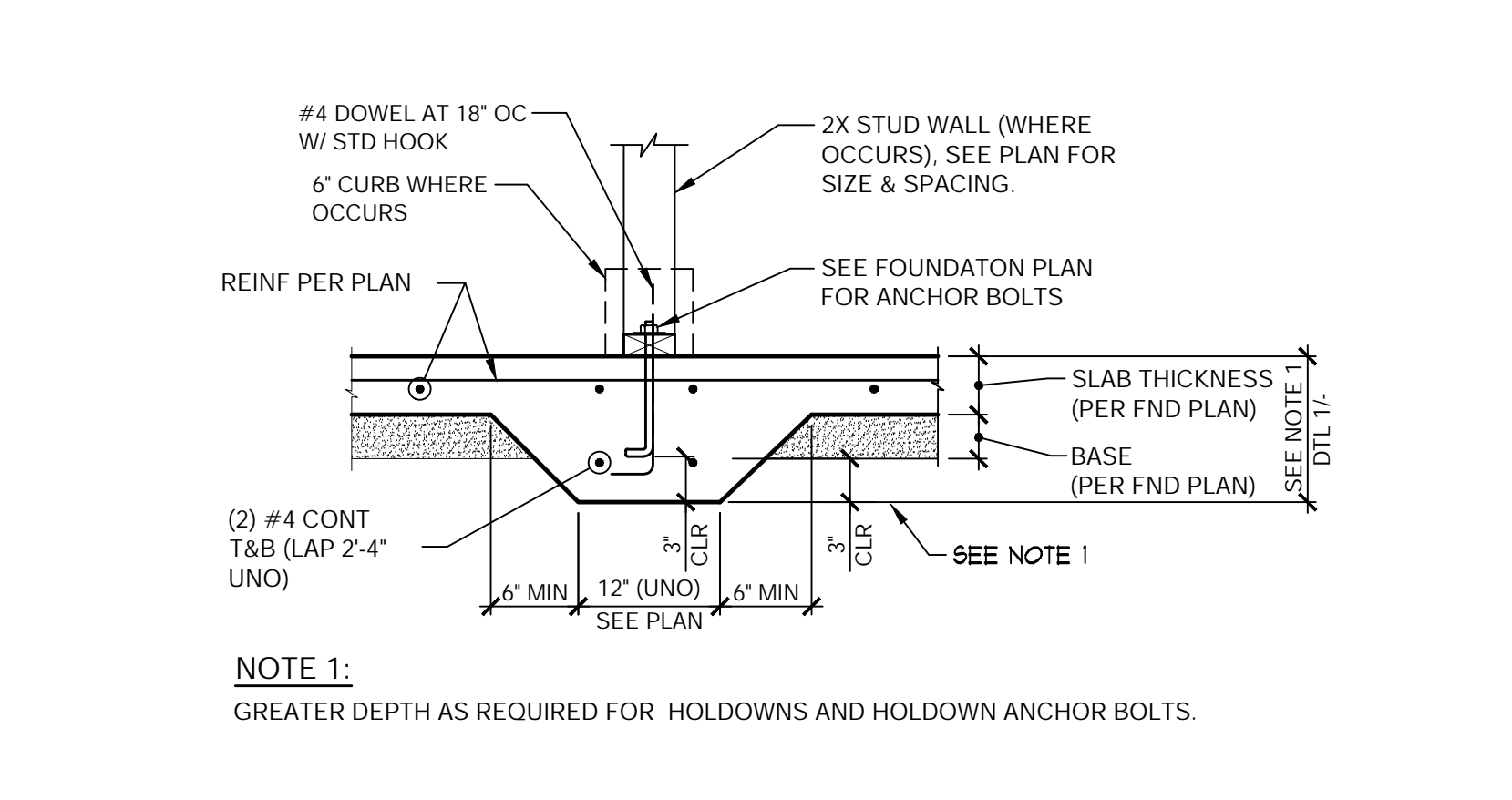
NOTE 1:
 BOTTOM OF FTG SHALL BE 12\"/>
1 EDGE OF FOUNDATION NTS



SEE DTL 1/- FOR INFO NOT SHOWN
2 EDGE OF FOUNDATION (GARAGE) NTS



SEE DTL 1/- FOR INFO NOT SHOWN
3 EDGE OF FOUNDATION (CURB) NTS



NOTE 1:
 GREATER DEPTH AS REQUIRED FOR HOLDOWNS AND HOLDOWN ANCHOR BOLTS.
4 INTERIOR CONTINUOUS FTG NTS

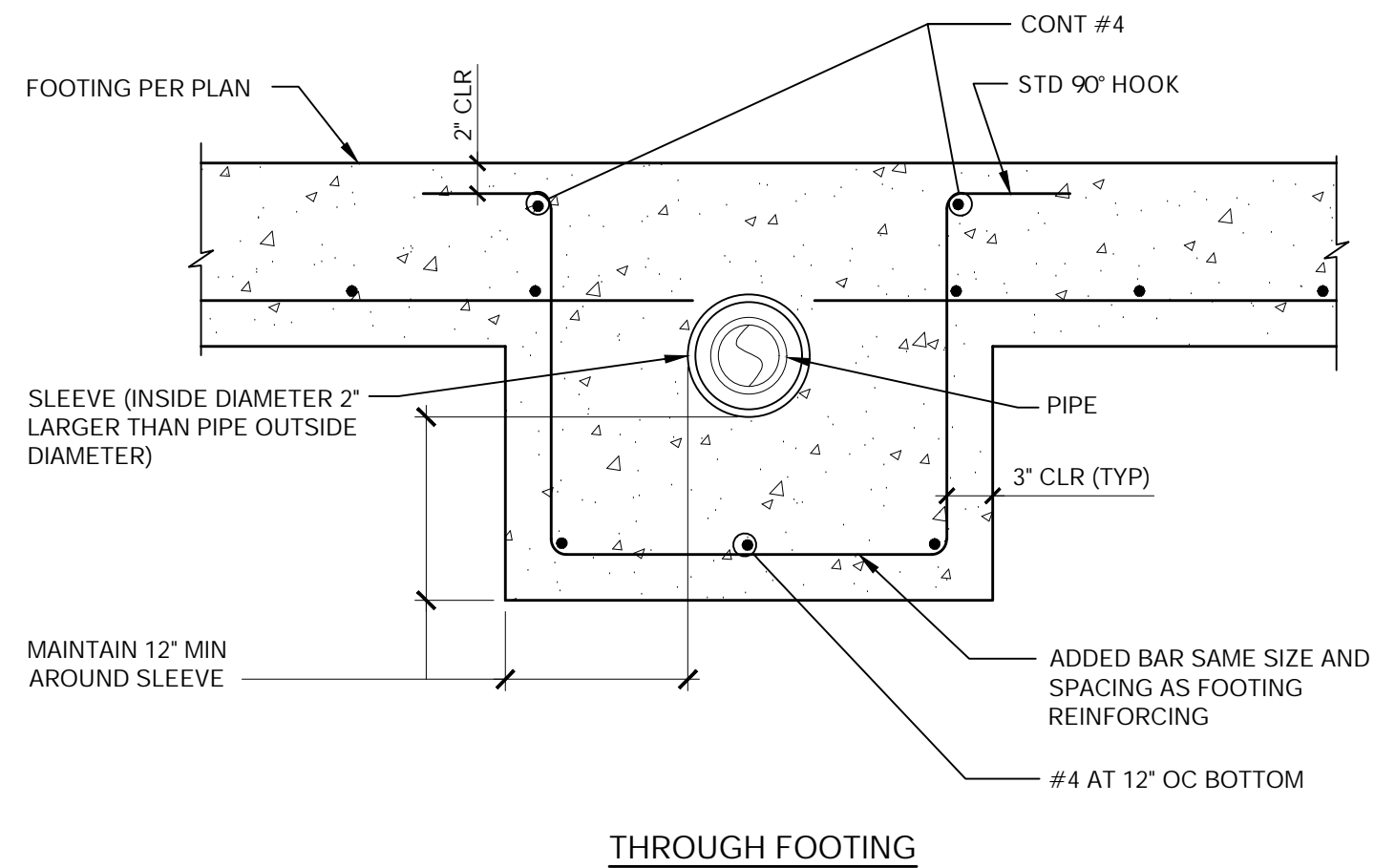
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 REVISIONS:

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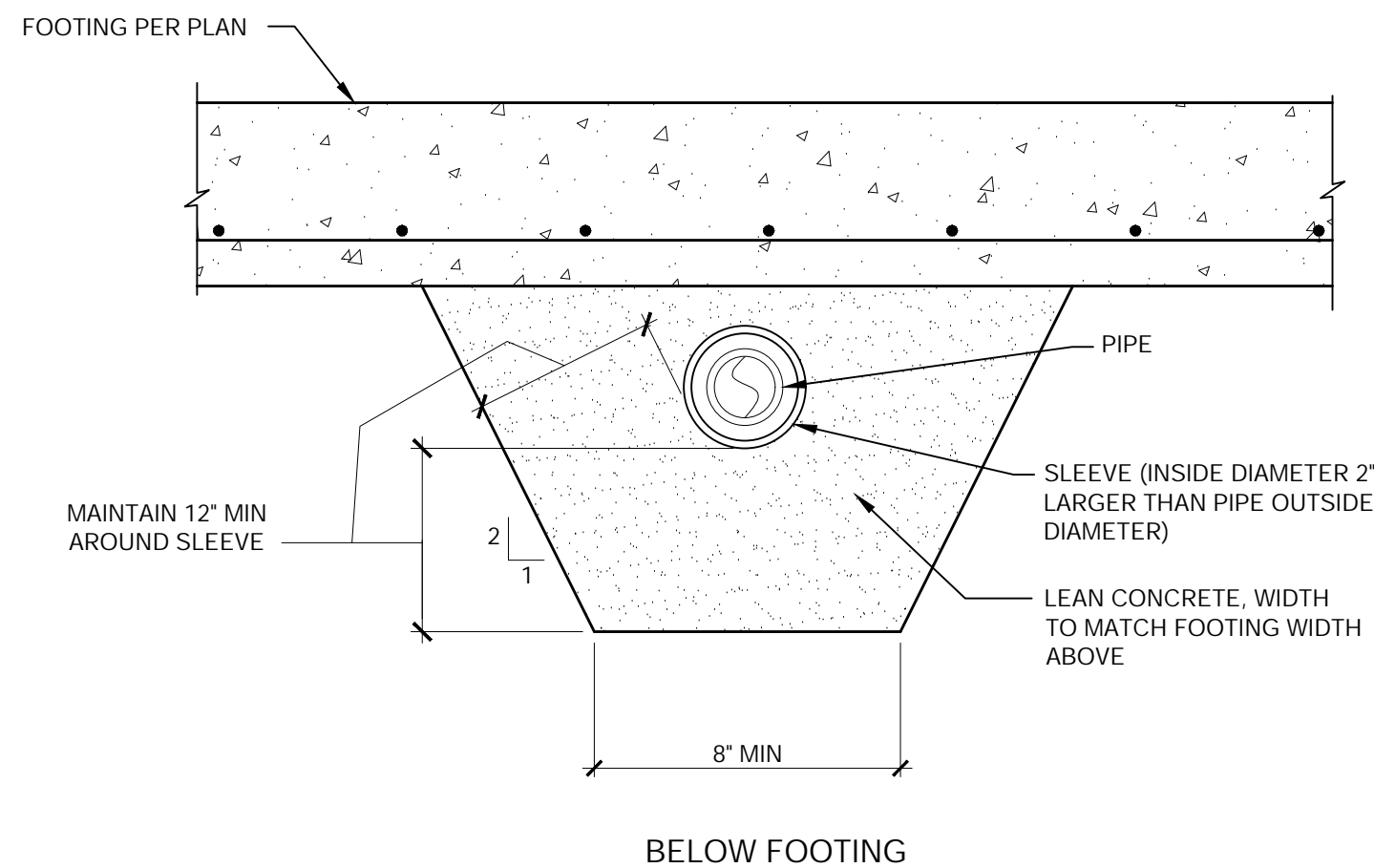
ASSURED DEVELOPMENT
 ATHENS LOT 2
 HENDERSON, NV



SD-1



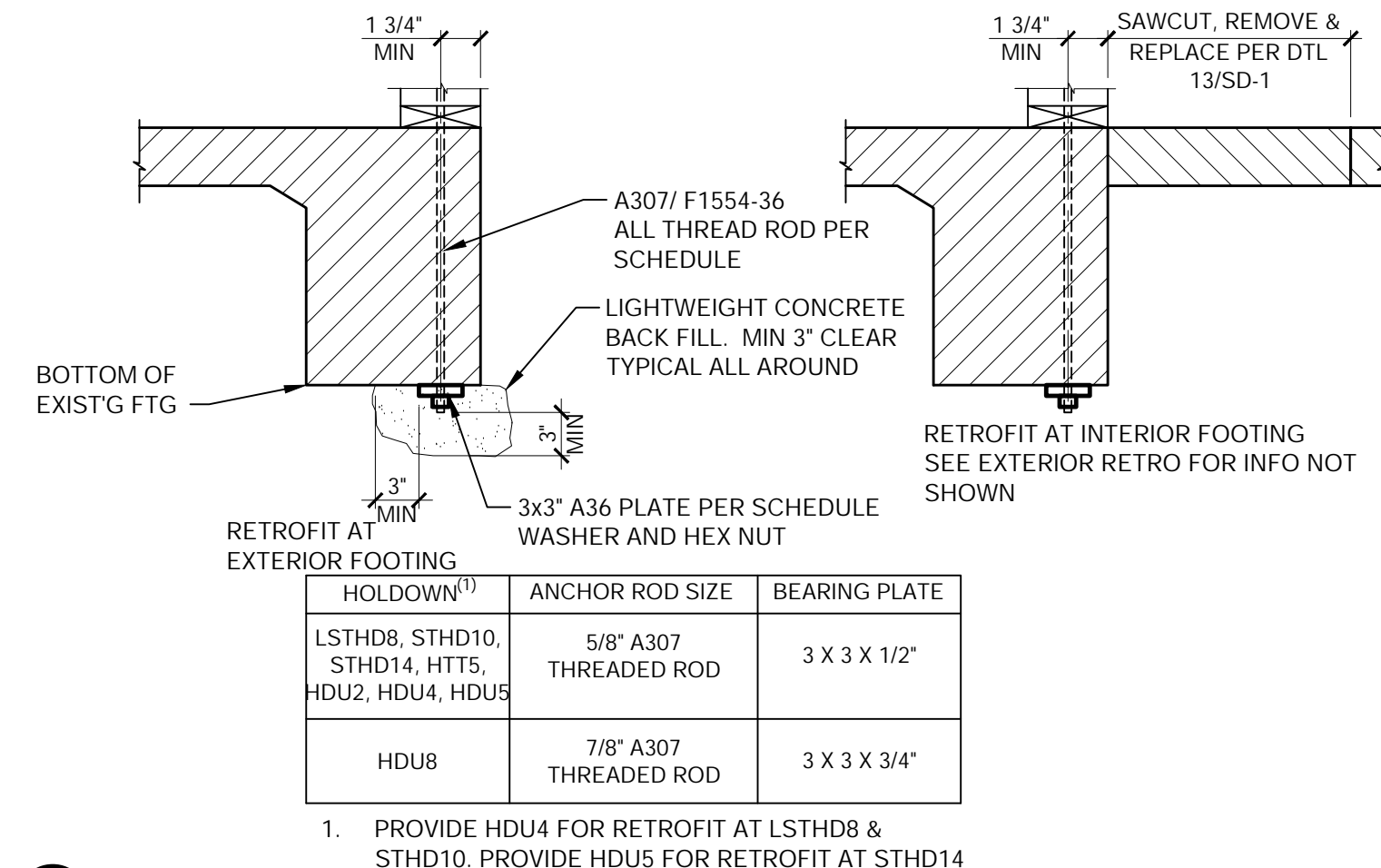
THROUGH FOOTING



BELOW FOOTING

9 PIPE SLEEVE DETAIL

NTS

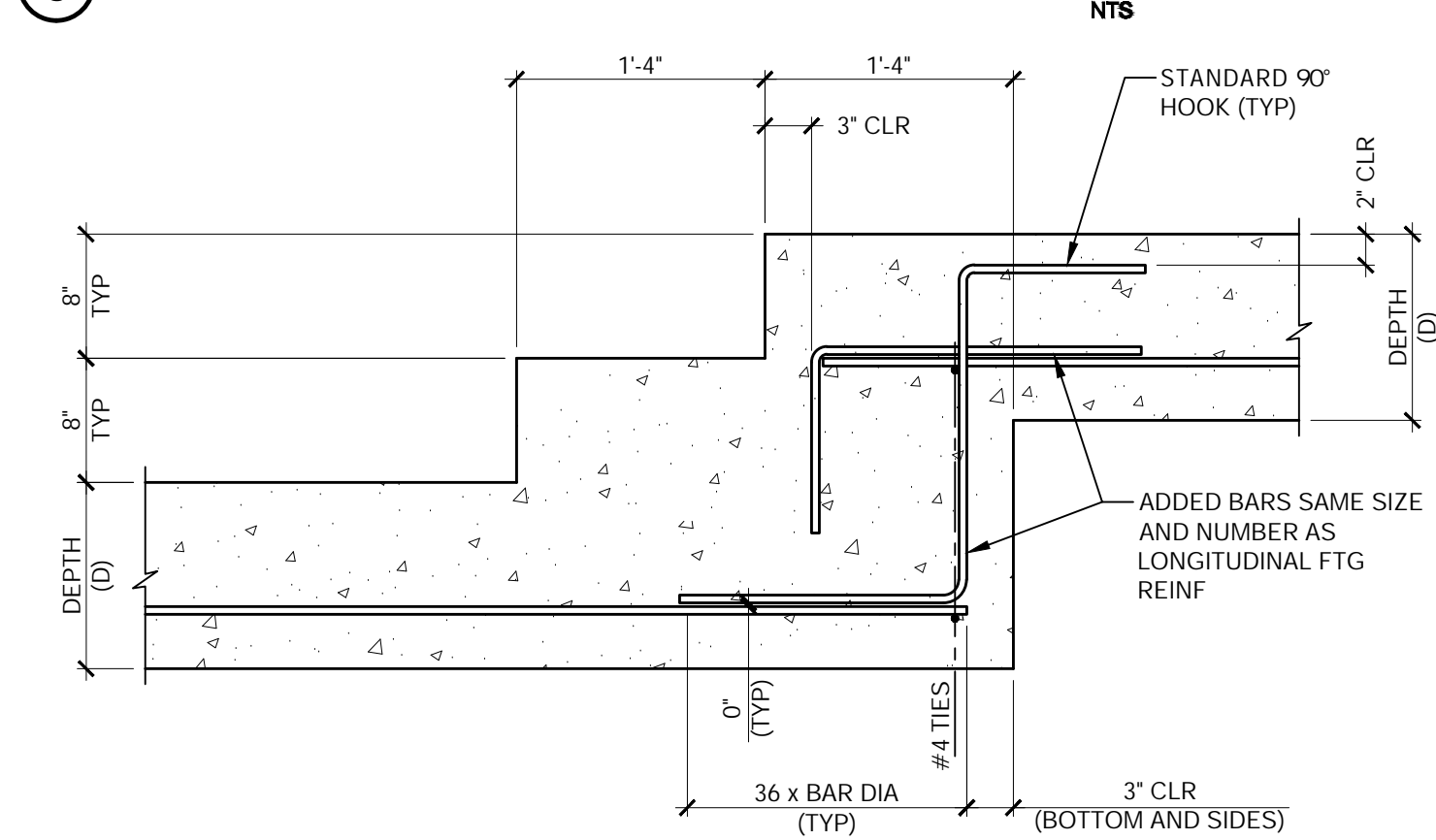


HOLDOWN ⁽¹⁾	ANCHOR ROD SIZE	BEARING PLATE
LSTHD8, STHD10, STHD14, HTTS, HDU2, HDU4, HDU5	5/8" A307 THREADED ROD	3 X 3 X 1/2"
HDU8	7/8" A307 THREADED ROD	3 X 3 X 3/4"

1. PROVIDE HDU4 FOR RETROFIT AT LSTHD8 & STHD10. PROVIDE HDU5 FOR RETROFIT AT STHD14

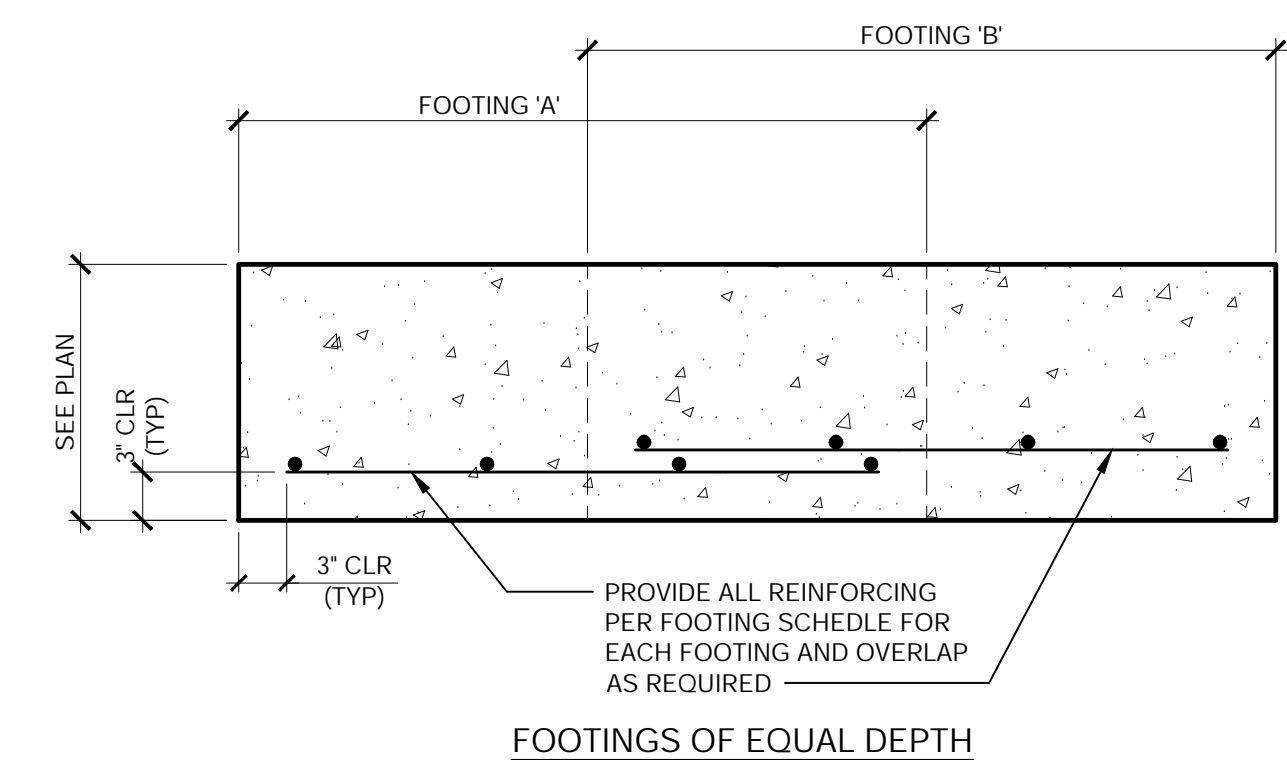
6

NTS



7 TYPICAL STEPPED FOOTING DETAIL

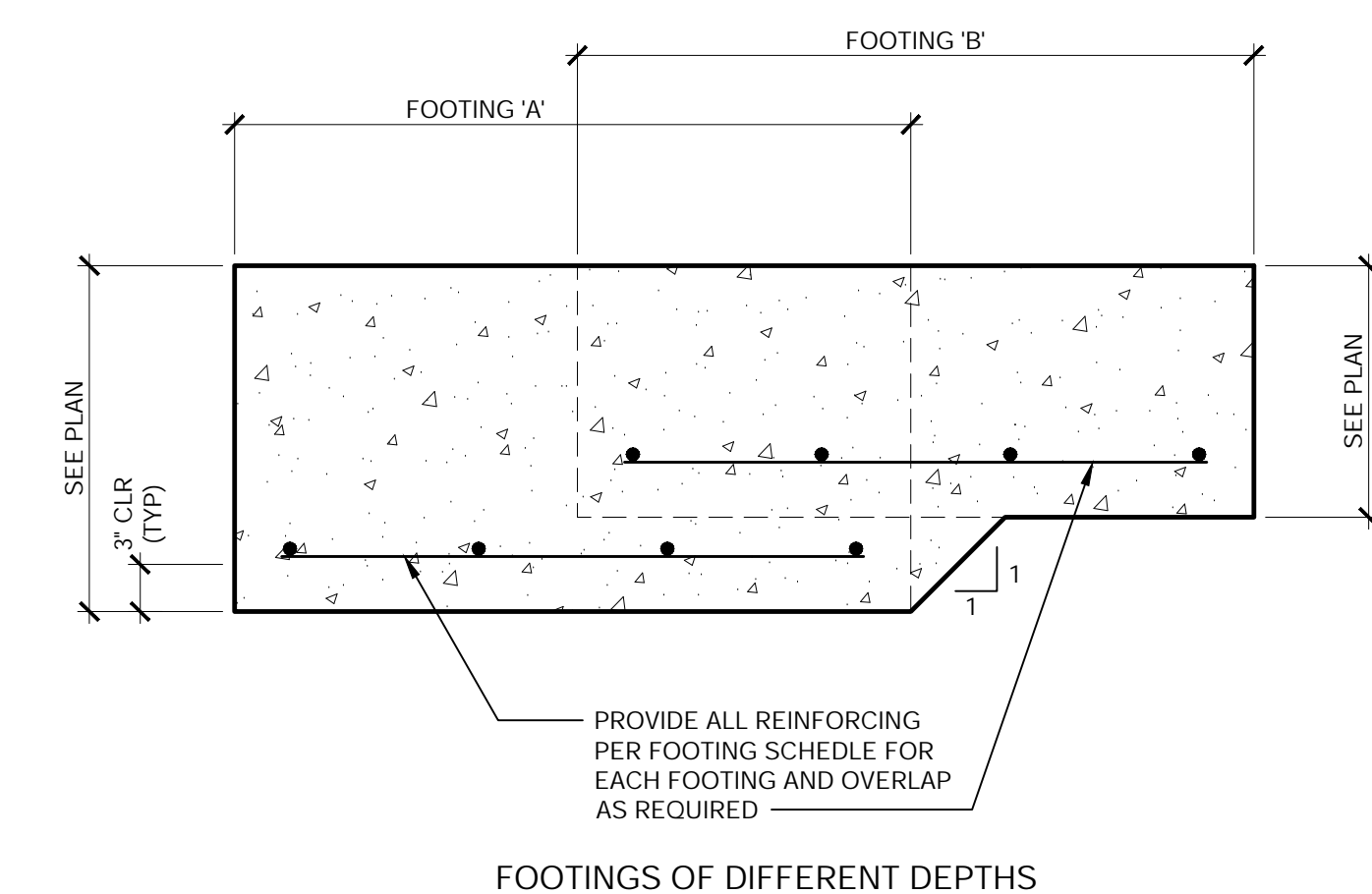
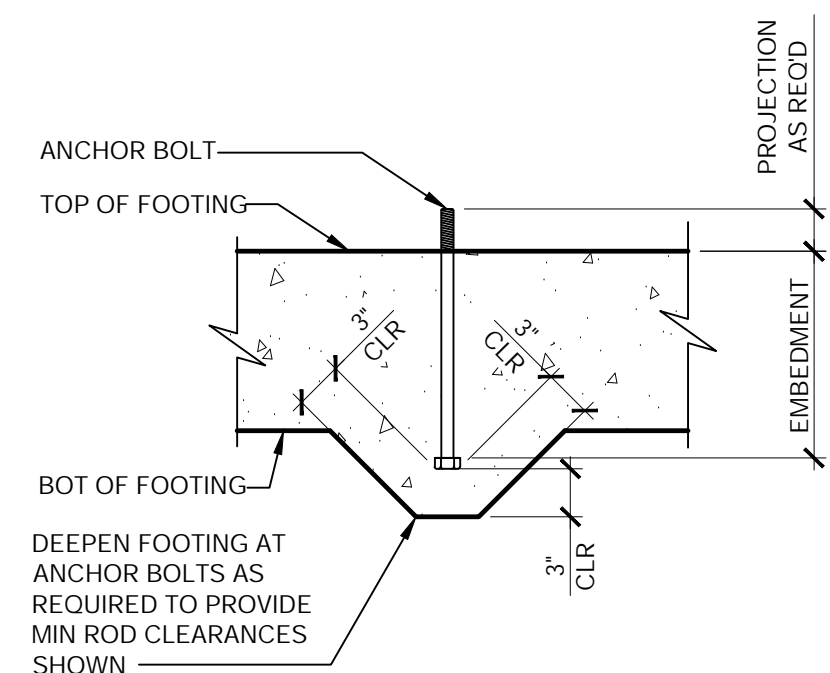
NTS



FOOTINGS OF EQUAL DEPTH

8 ANCHOR BOLT EMBEDMENT DETAIL

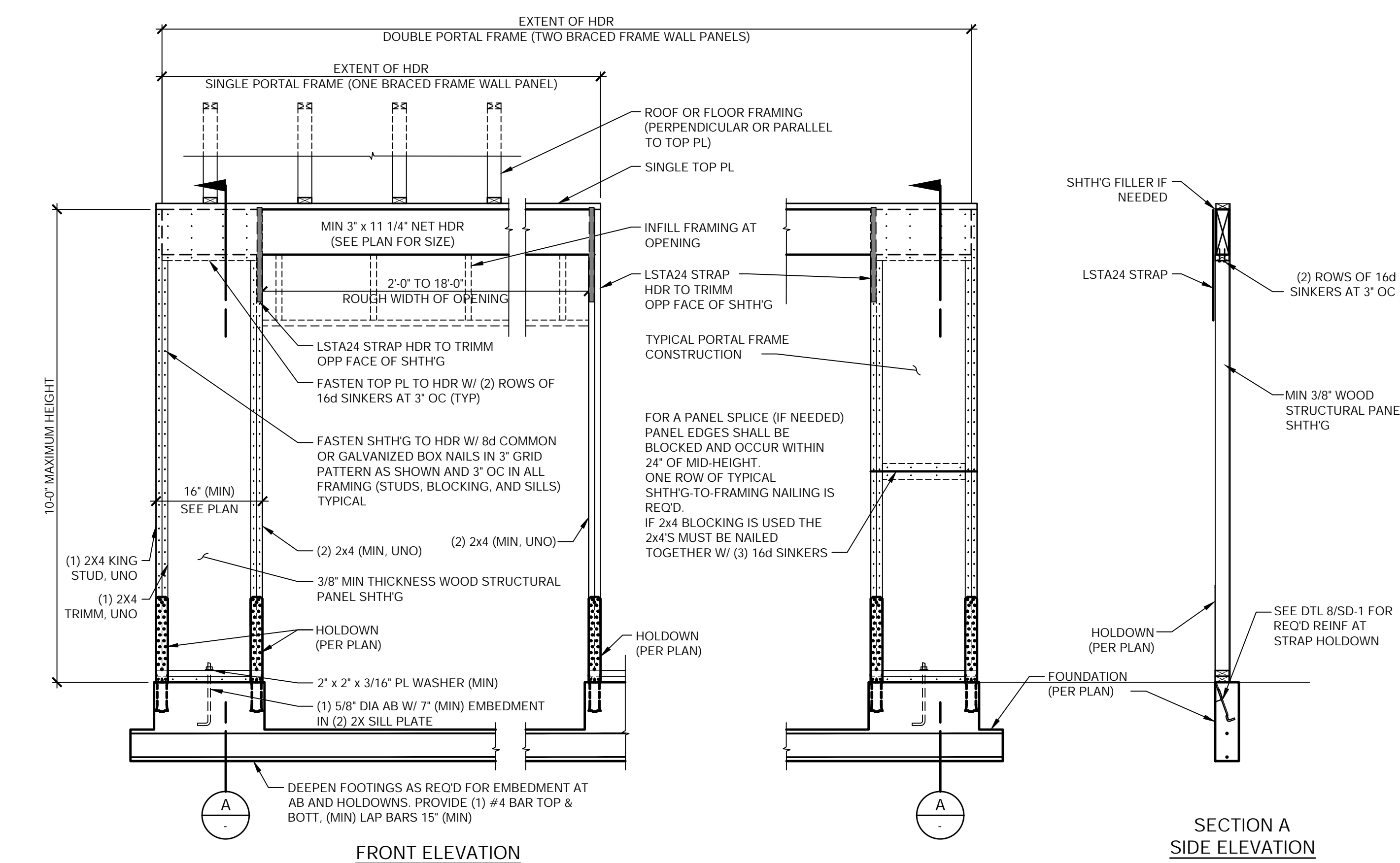
NTS



FOOTINGS OF DIFFERENT DEPTHS

10 OVERLAPPING SPREAD FOOTING DETAIL

NTS

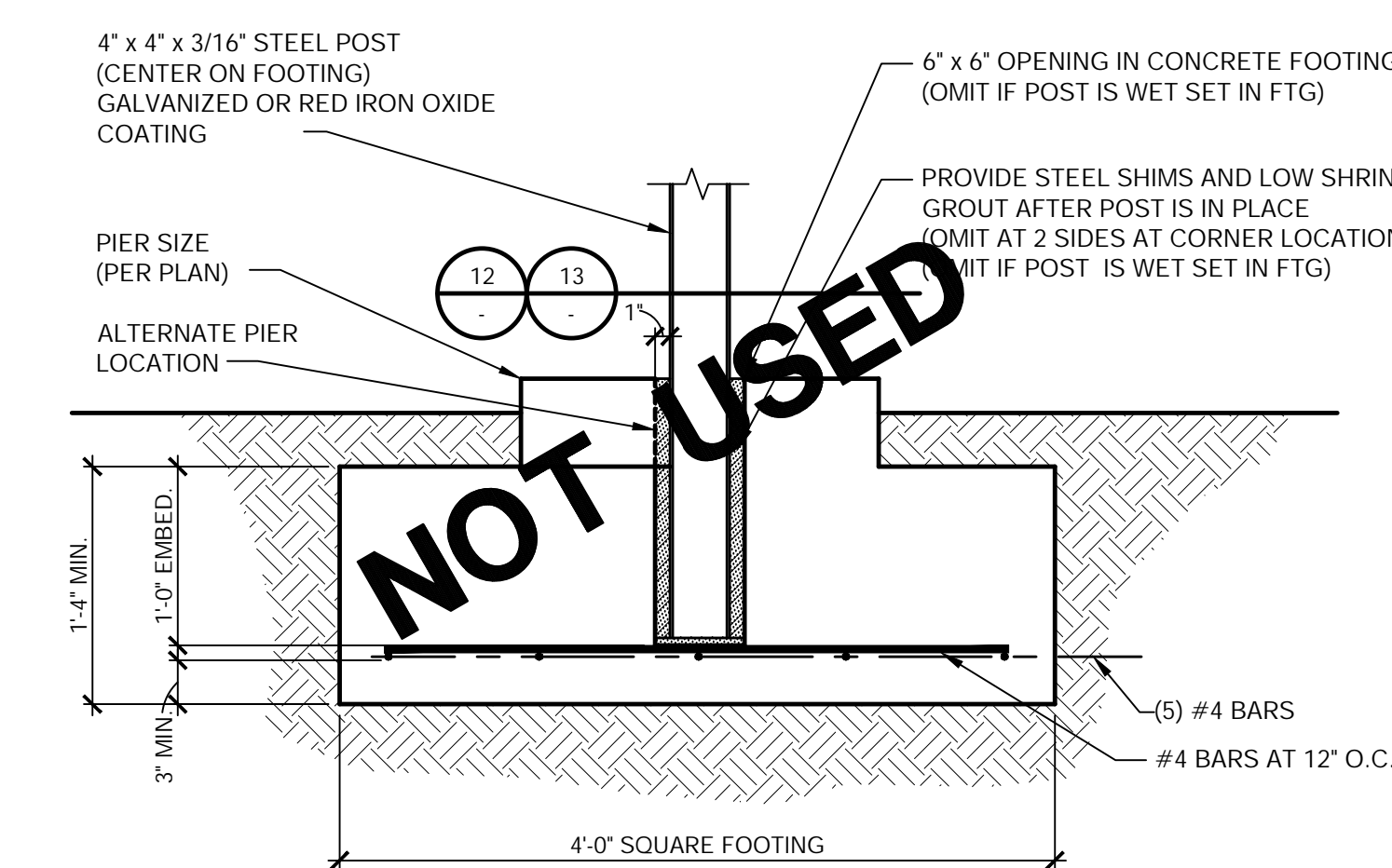


FRONT ELEVATION

SECTION A SIDE ELEVATION

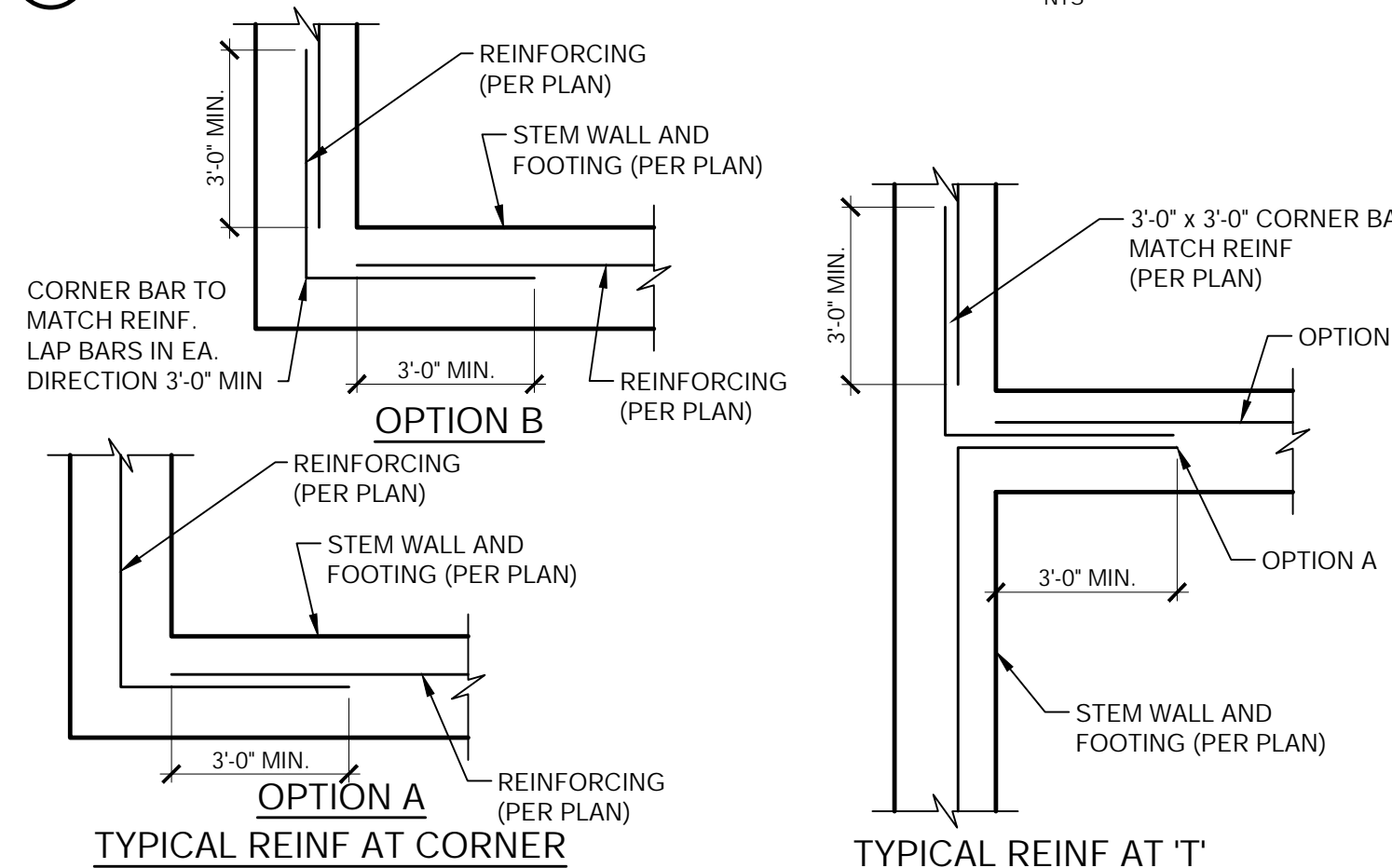
1 APA PORTAL FRAME

NTS



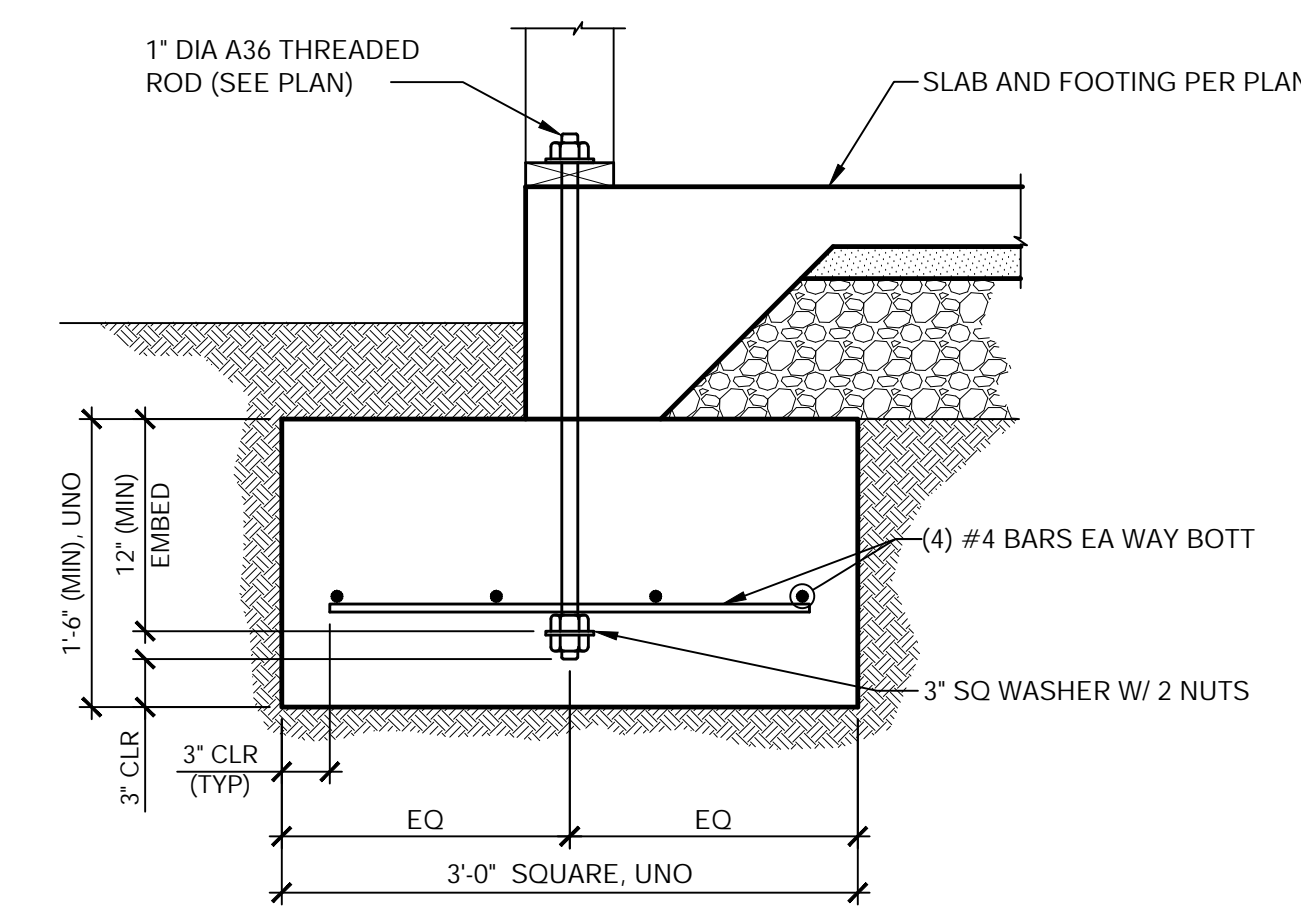
4 EMBEDDED STEEL POST DETAIL

NTS



5 CONCRETE WALL/FOOTING REINFORCING

NTS



2 HOLDOWN FOOTING

SCALE:

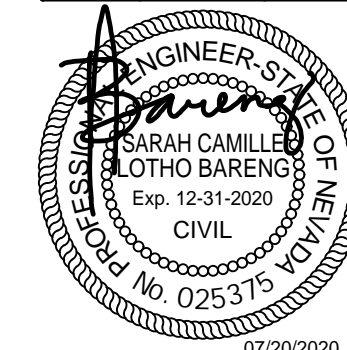
3

SCALE:

REVISIONS:

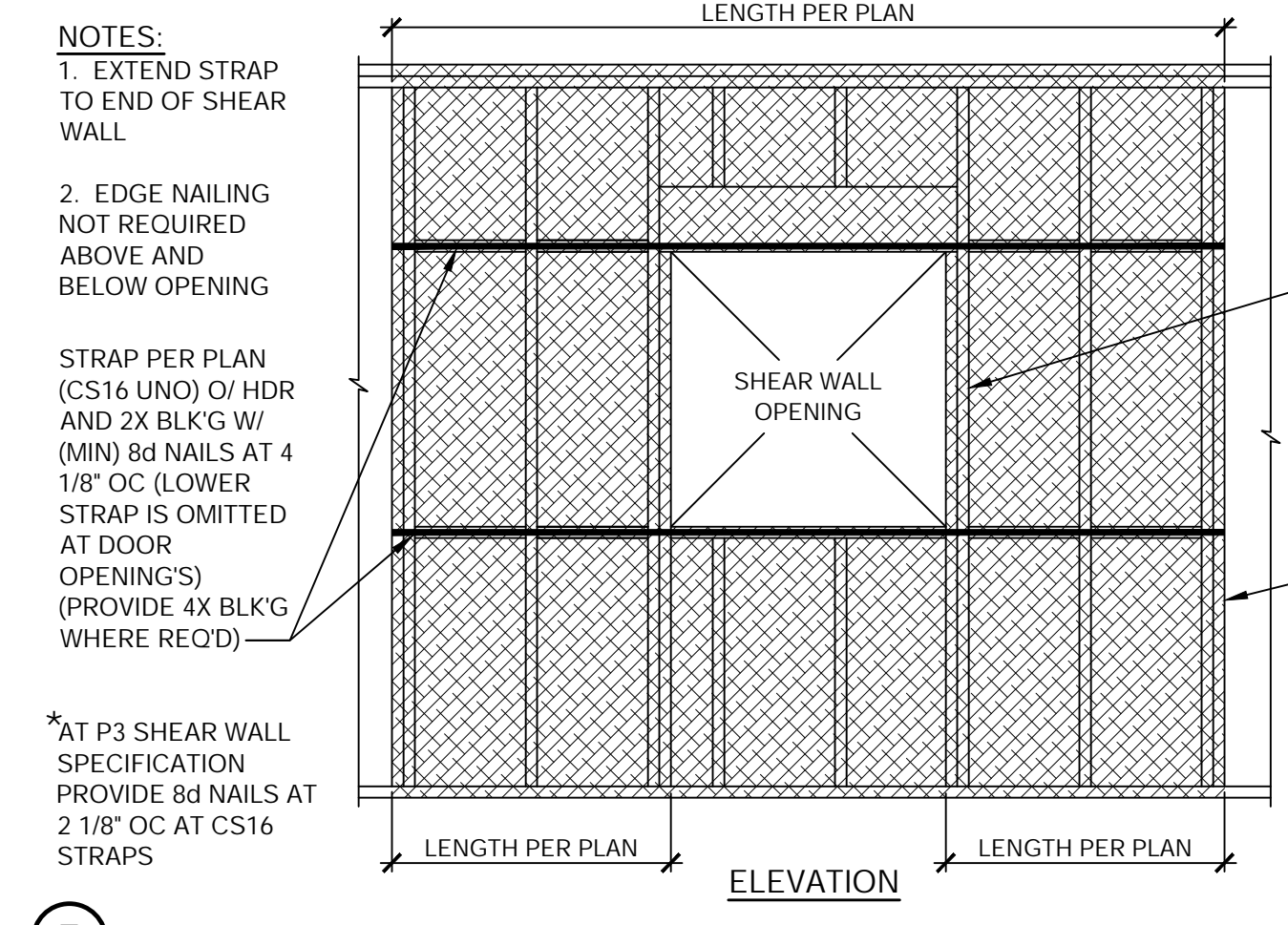
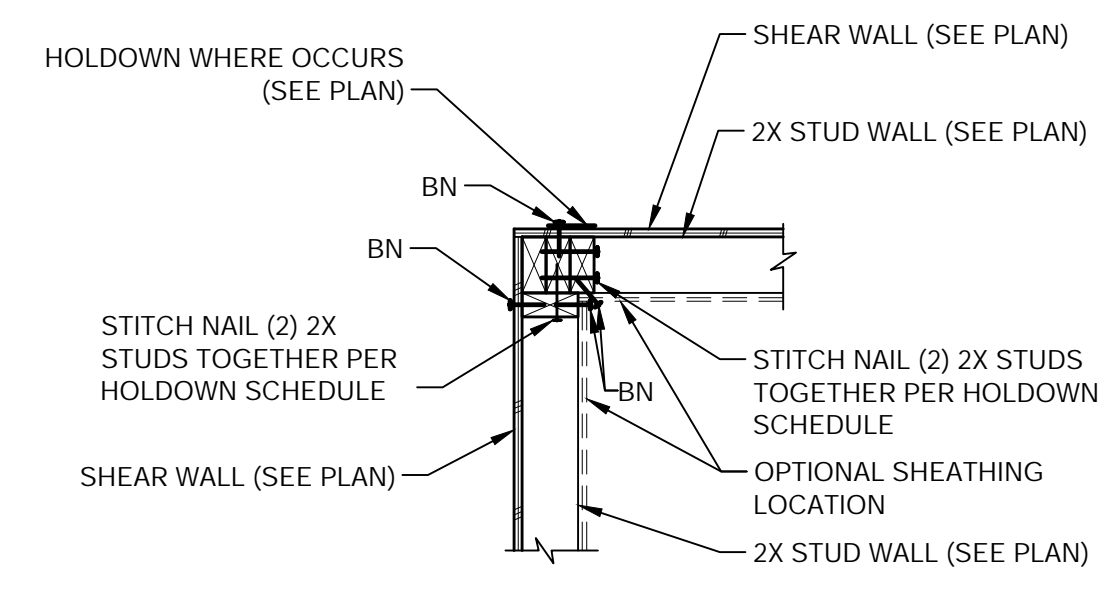
L. R. NELSON CONSULTING ENGINEERS, LLC.
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ASSURED DEVELOPMENT
 ATHENS LOT 2
 HENDERSON, NV

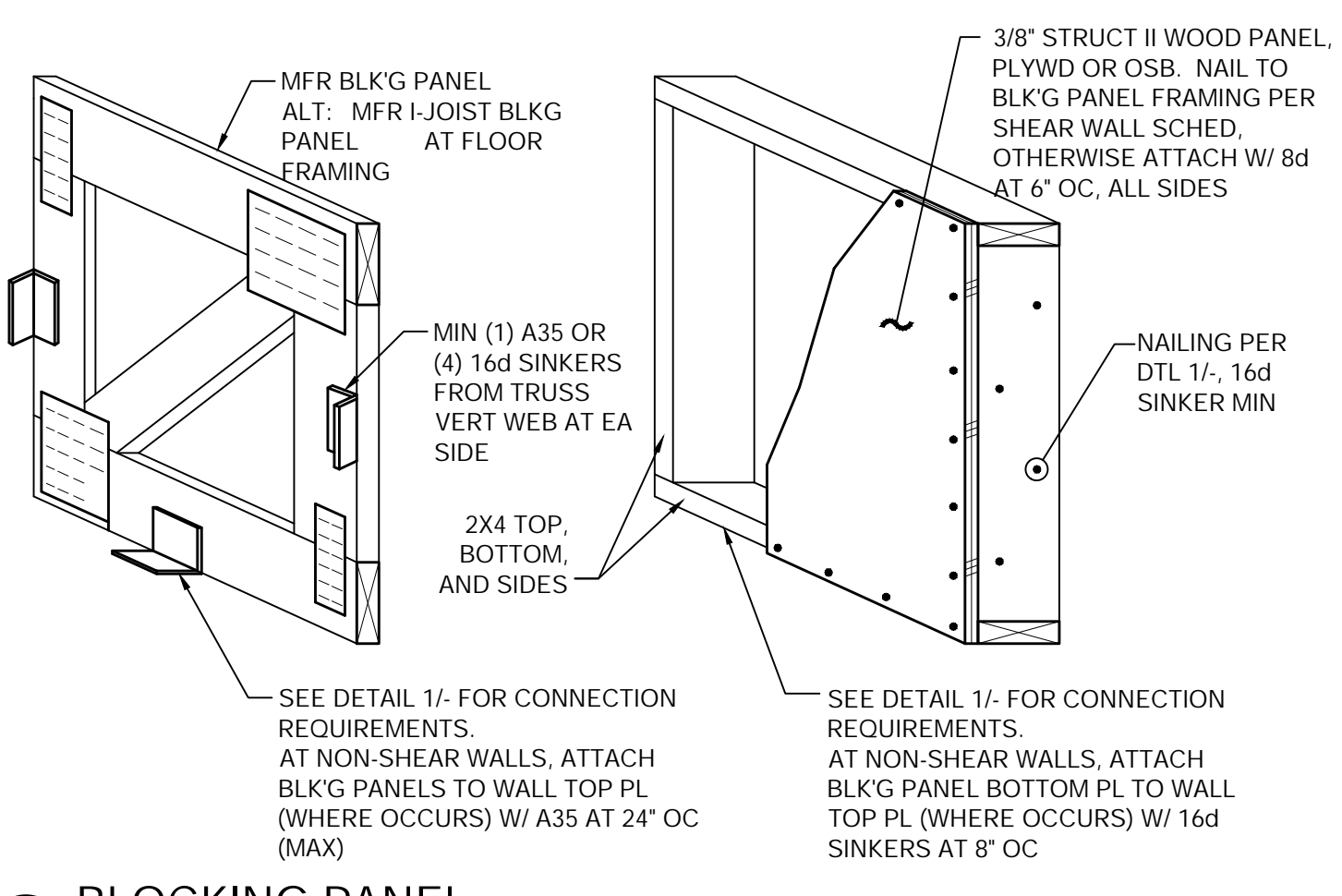


SD-1.1

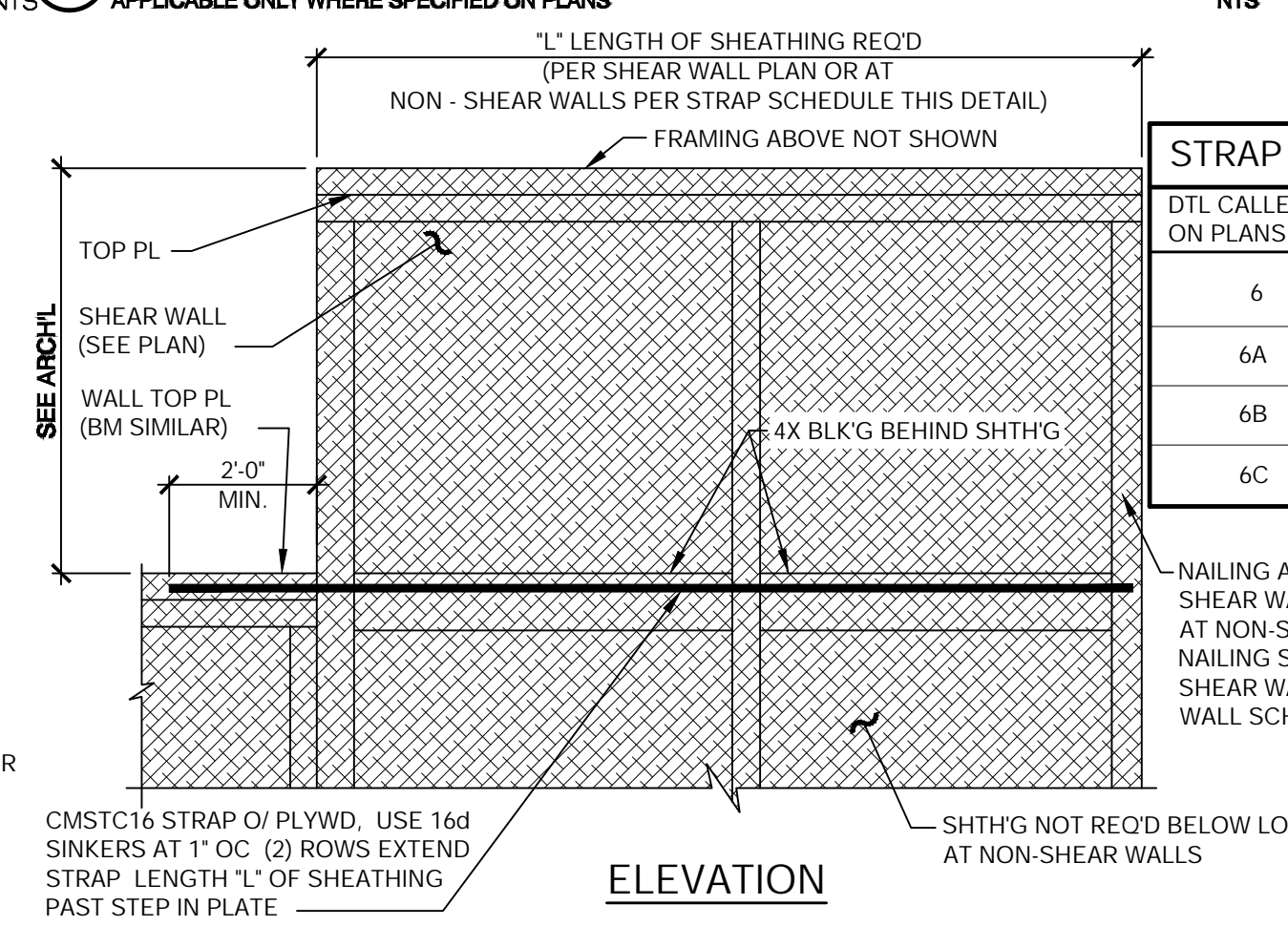
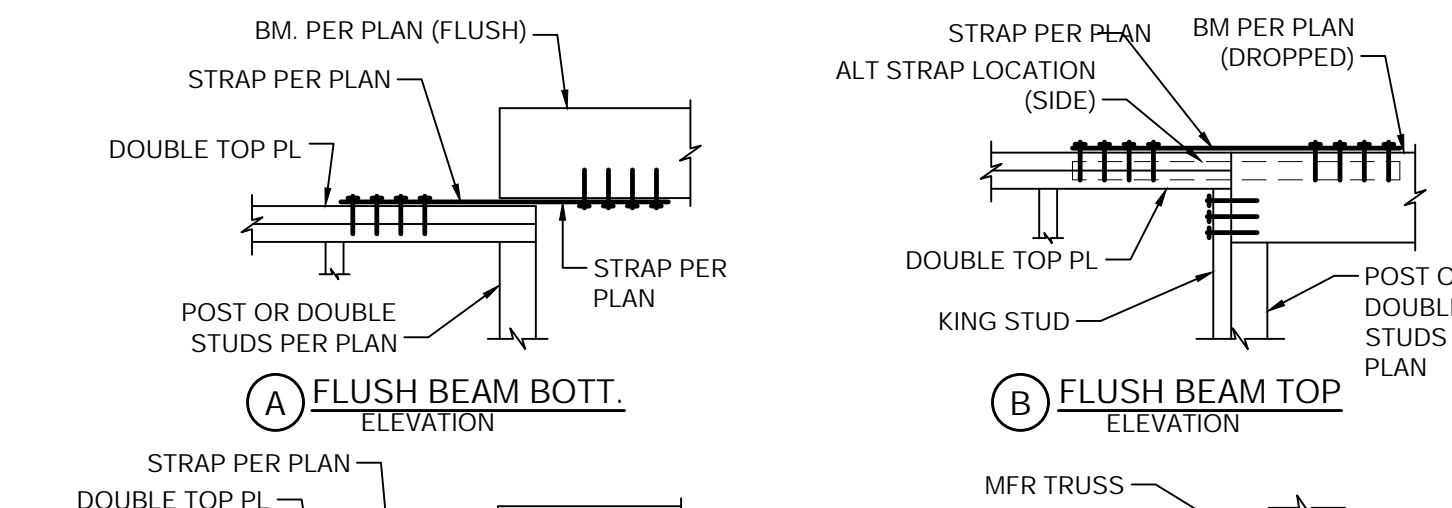
9 TWO SHEAR WALLS AT CORNER W/ SHARED HOLDOWN



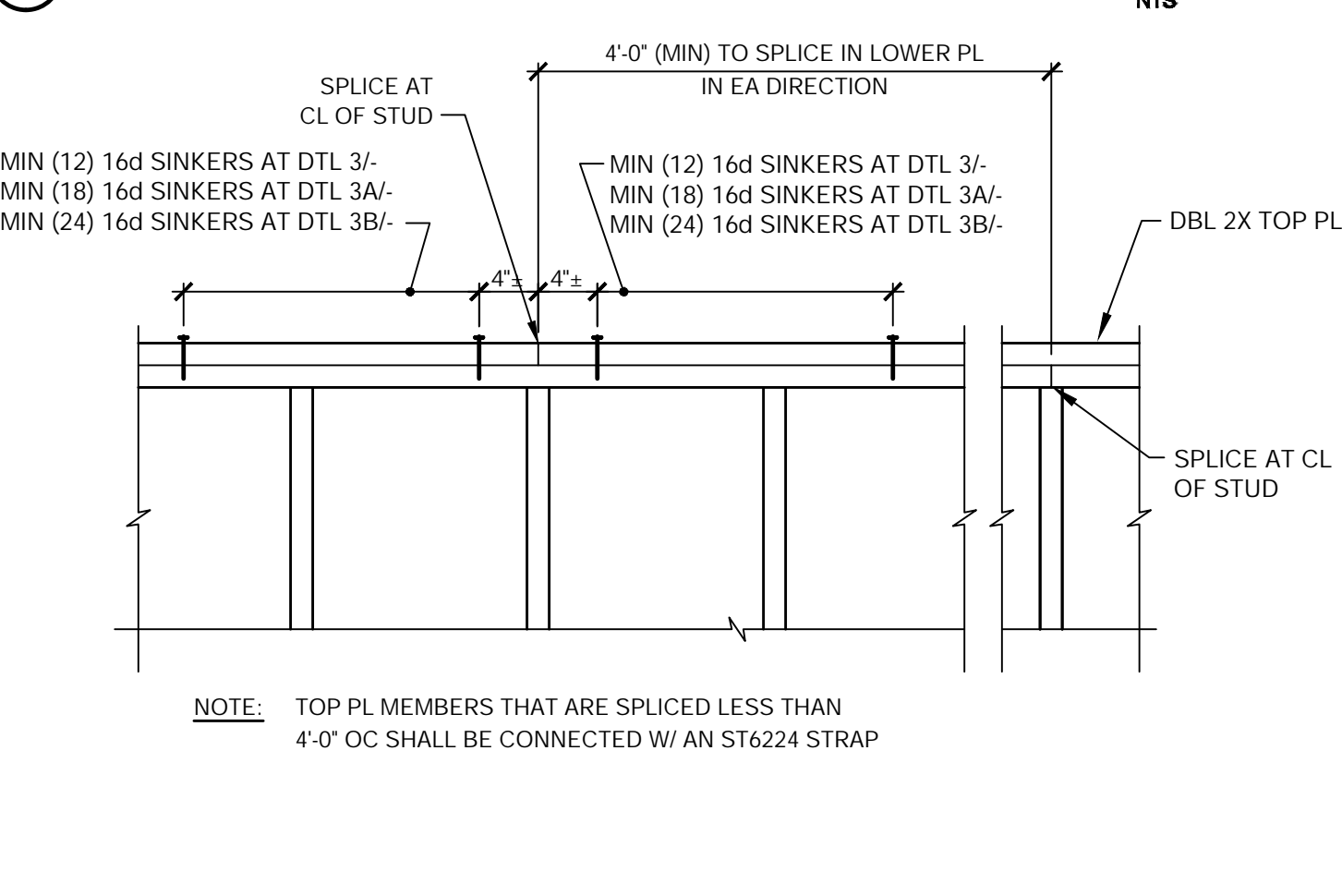
2 BLOCKING PANEL



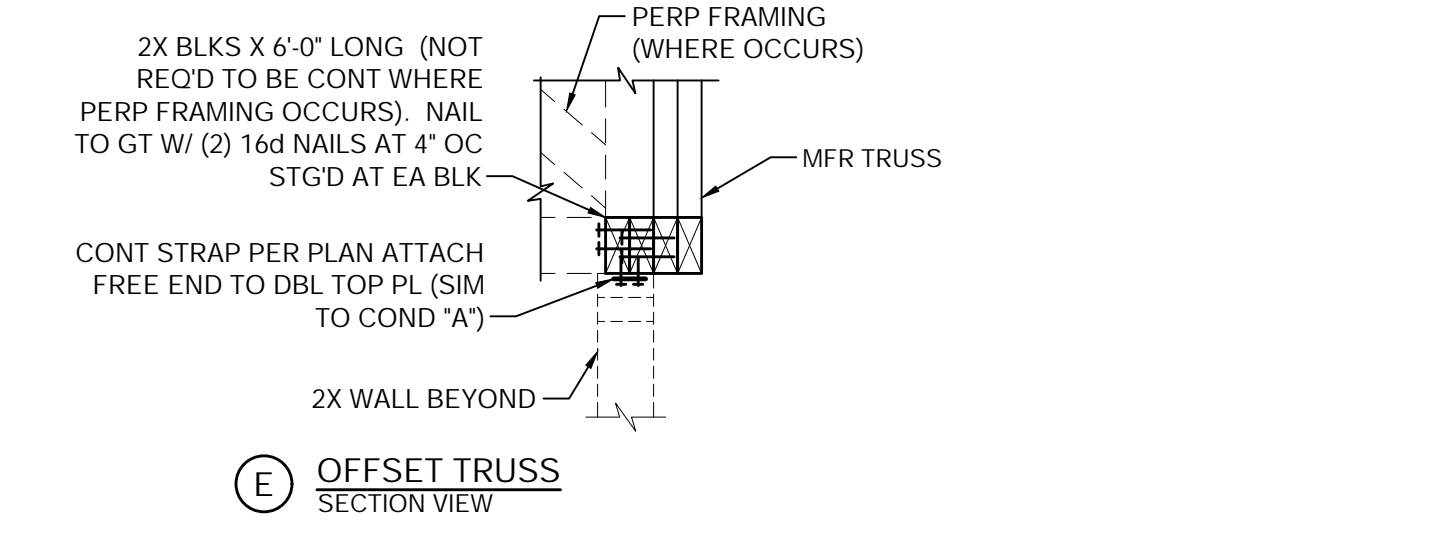
6A 6B 6C PLATE STEP



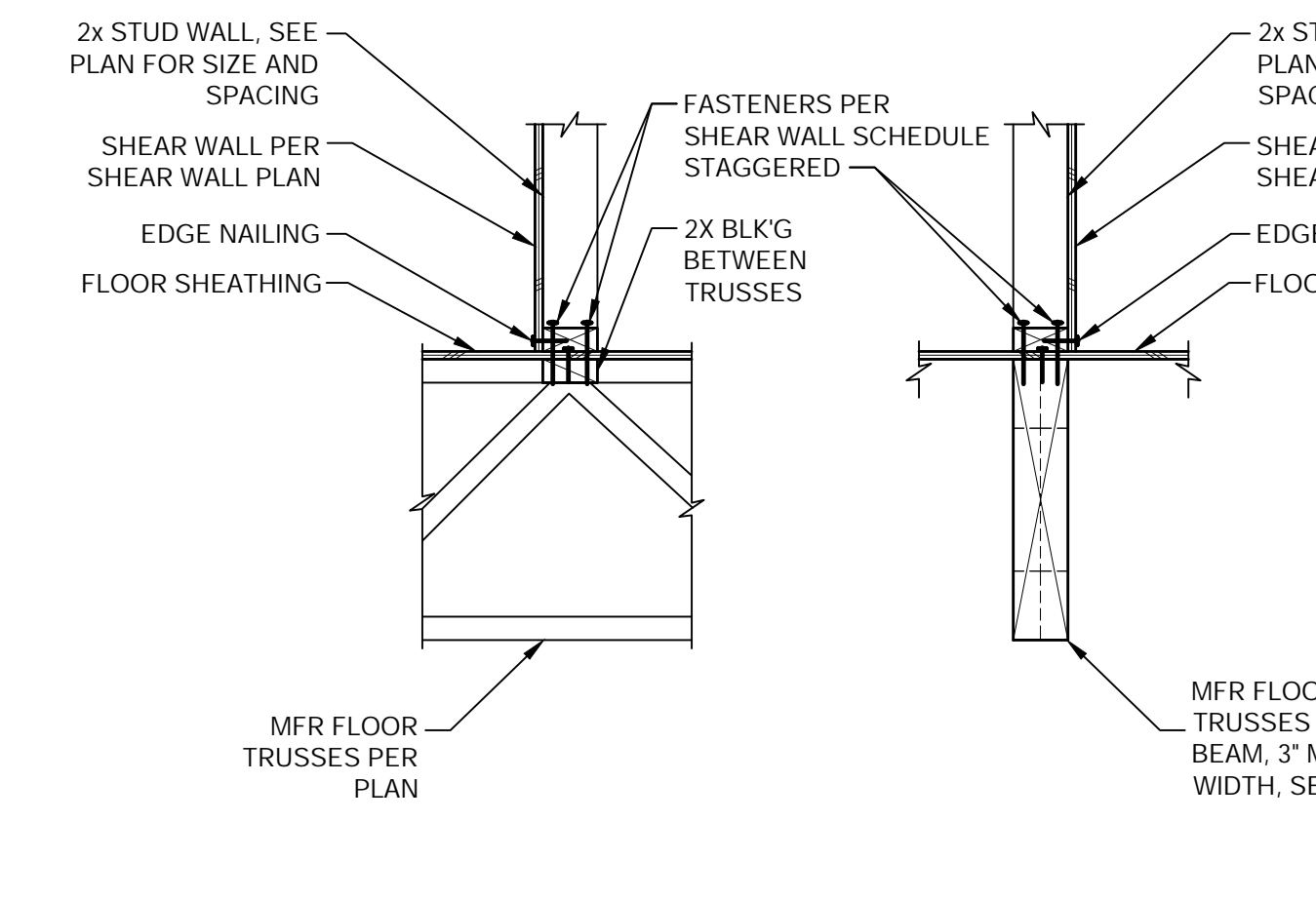
3 3A 3B TOP PLATE SPLICE



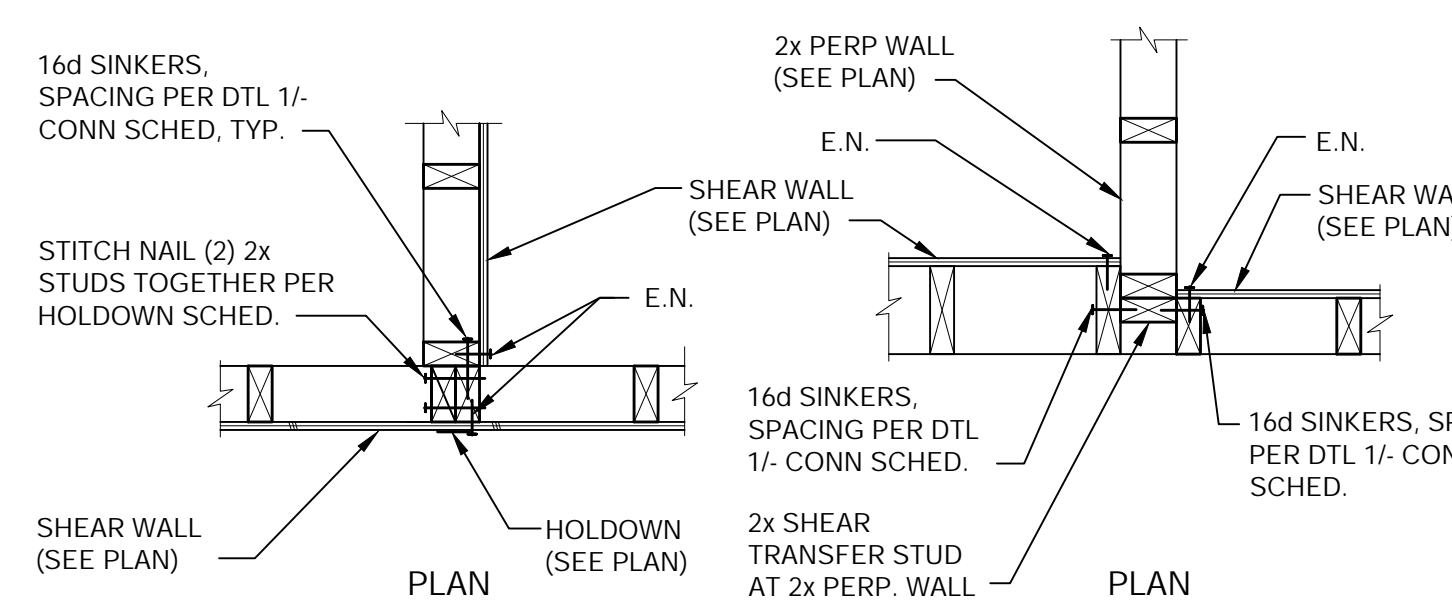
10 STRAP PLACEMENT



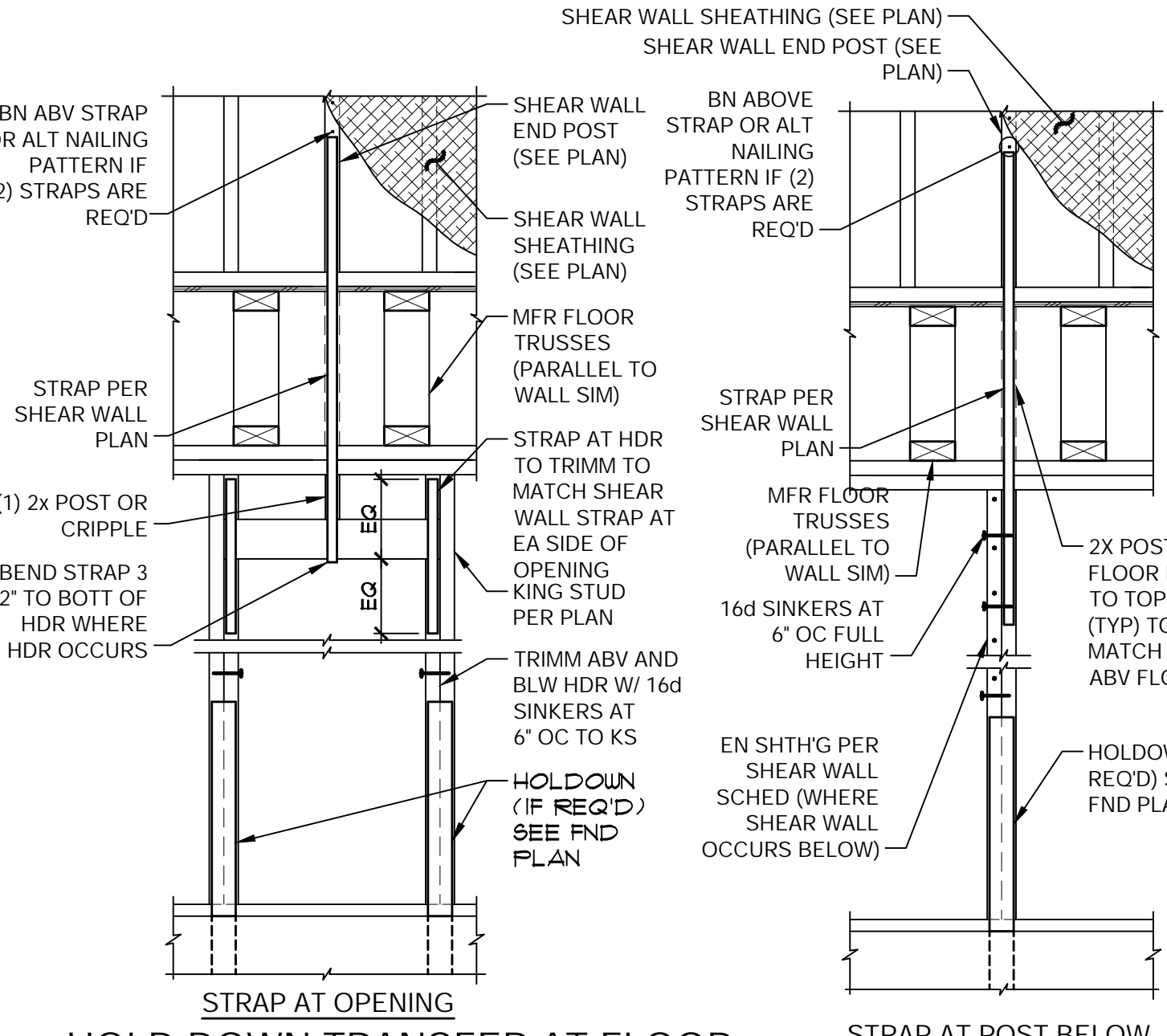
7 INT. SW TO MFR FLOOR TRUSSES OR BEAM



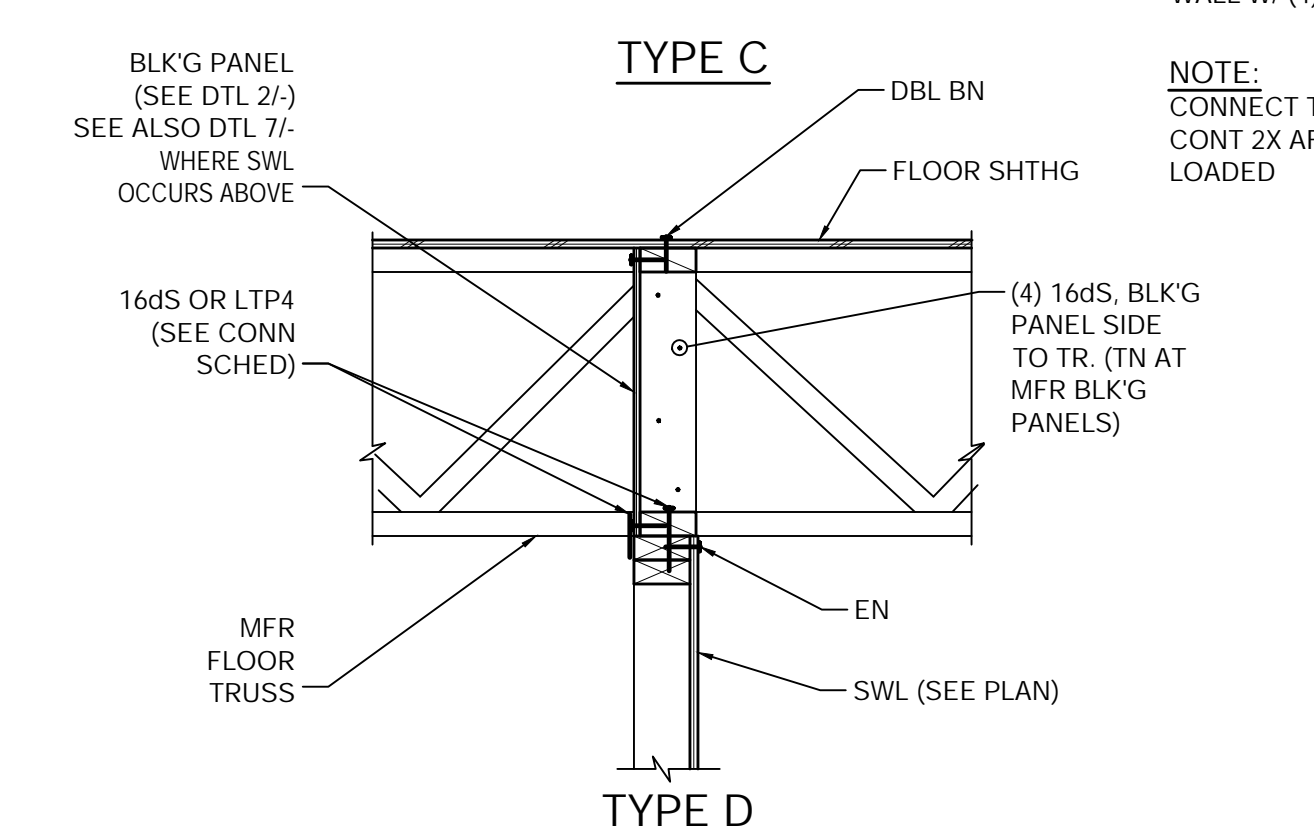
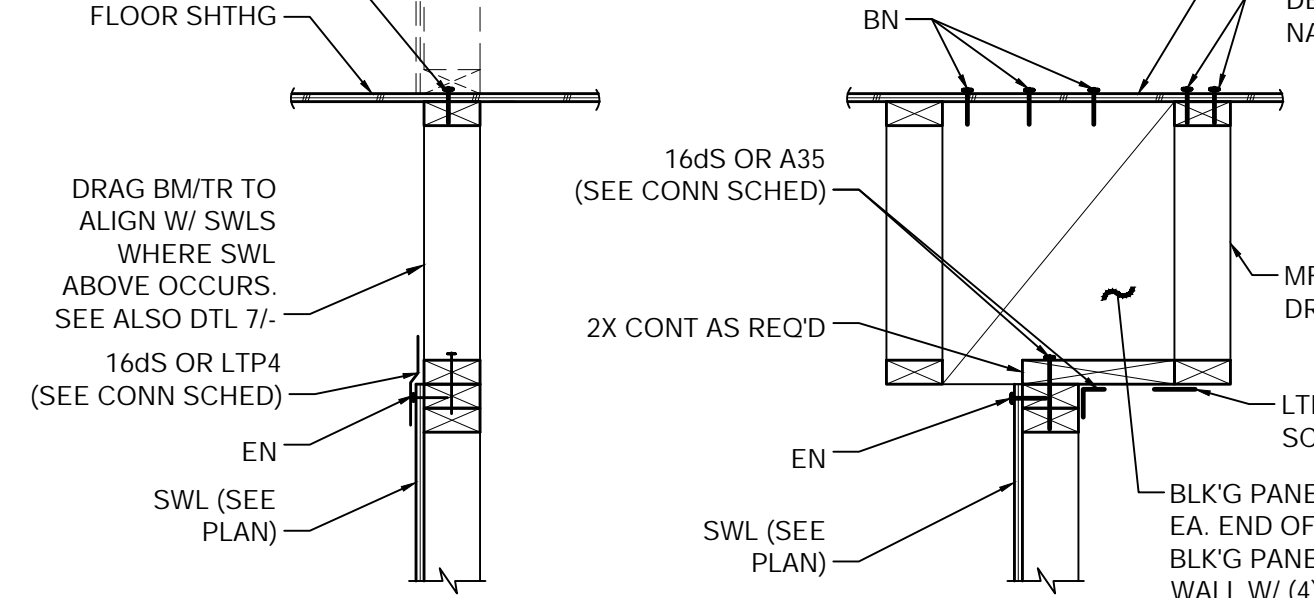
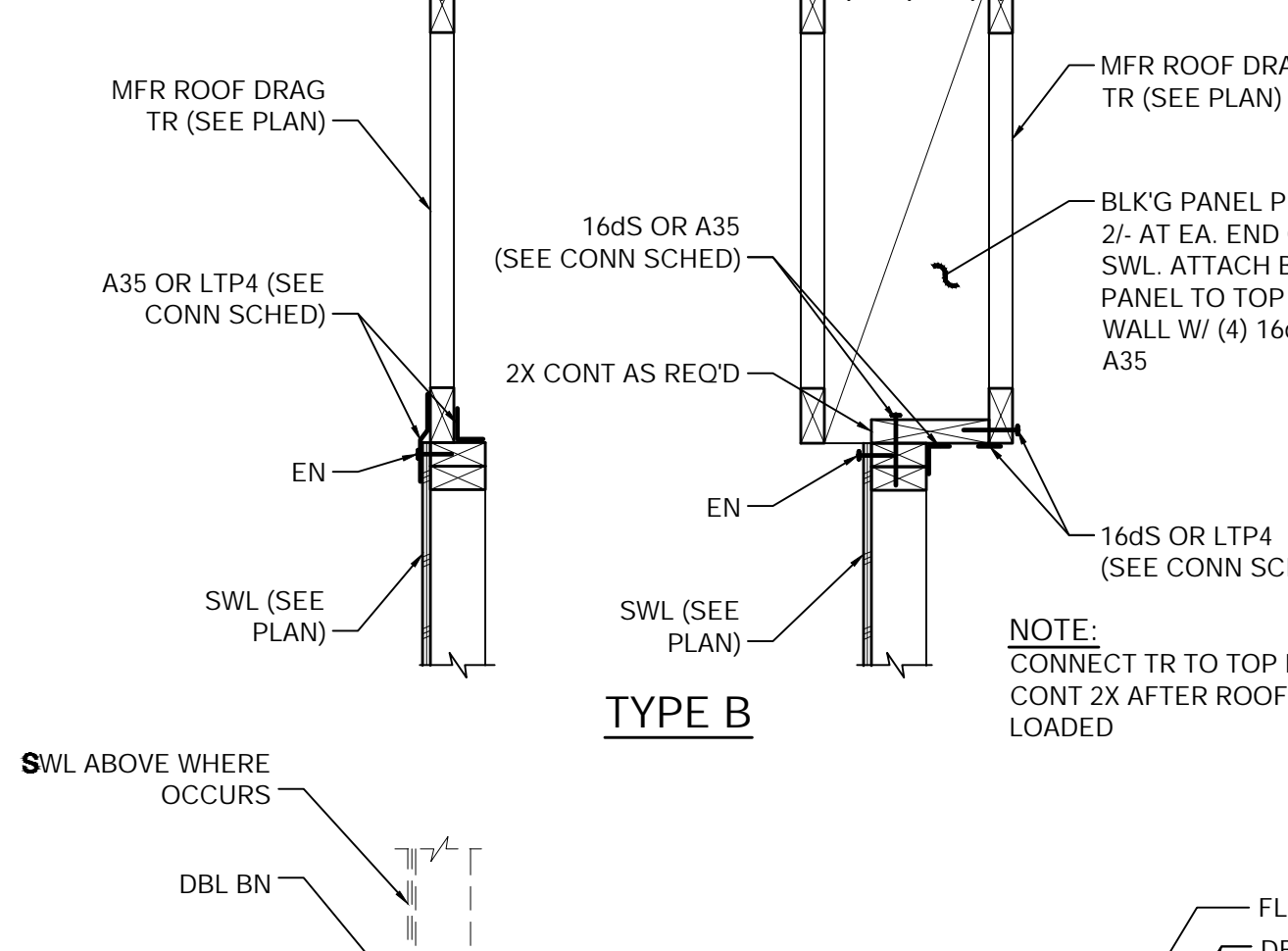
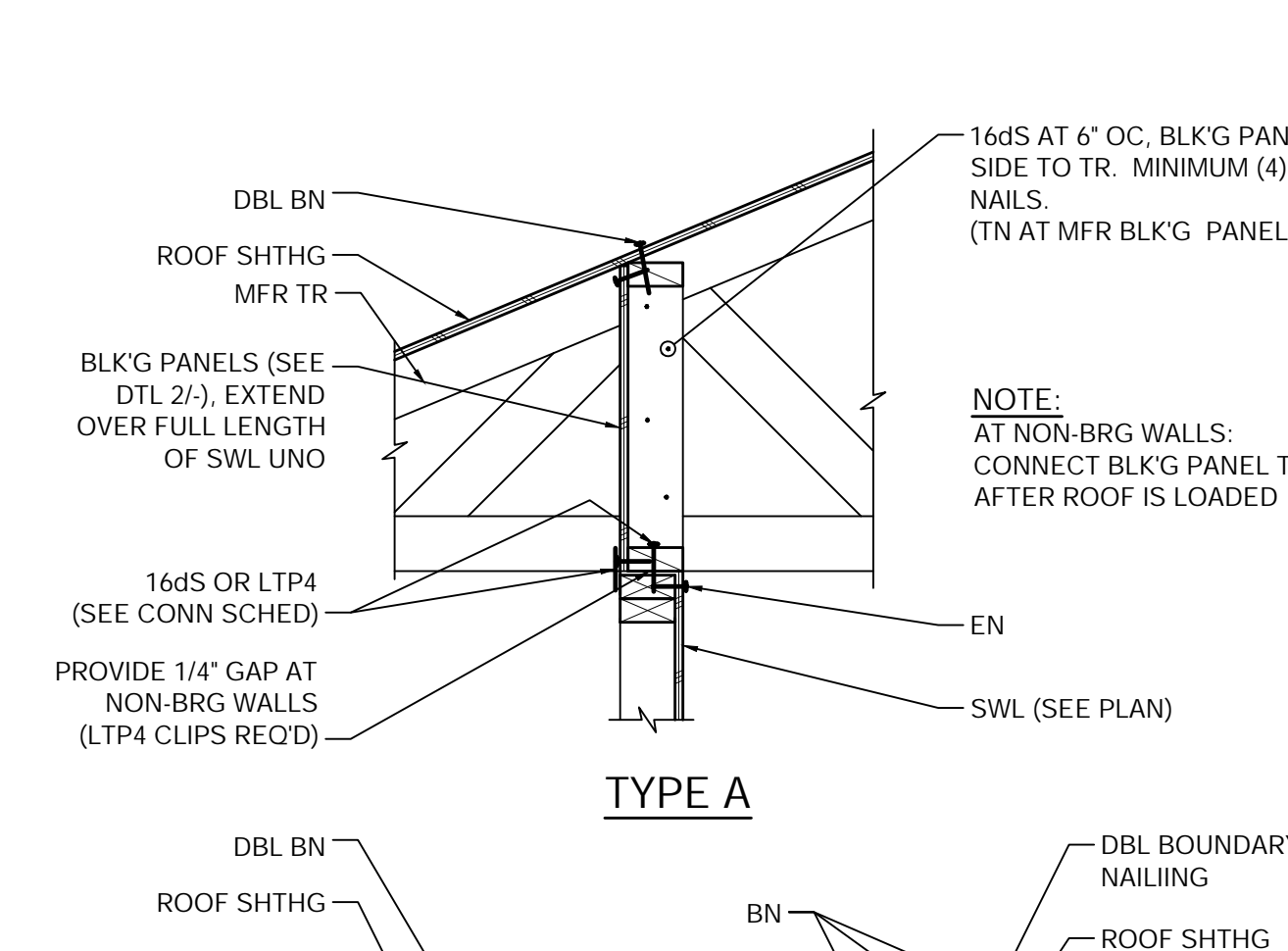
11 SHEAR WALL HOLDDOWN



4 HOLD DOWN TRANSFER AT FLOOR



1 SHEAR TRANSFER DTLS

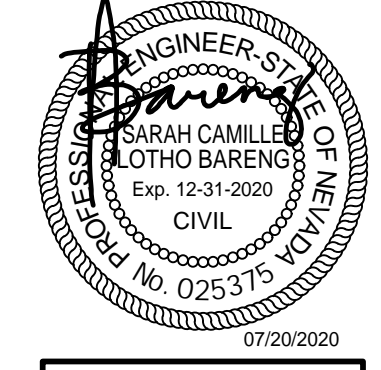


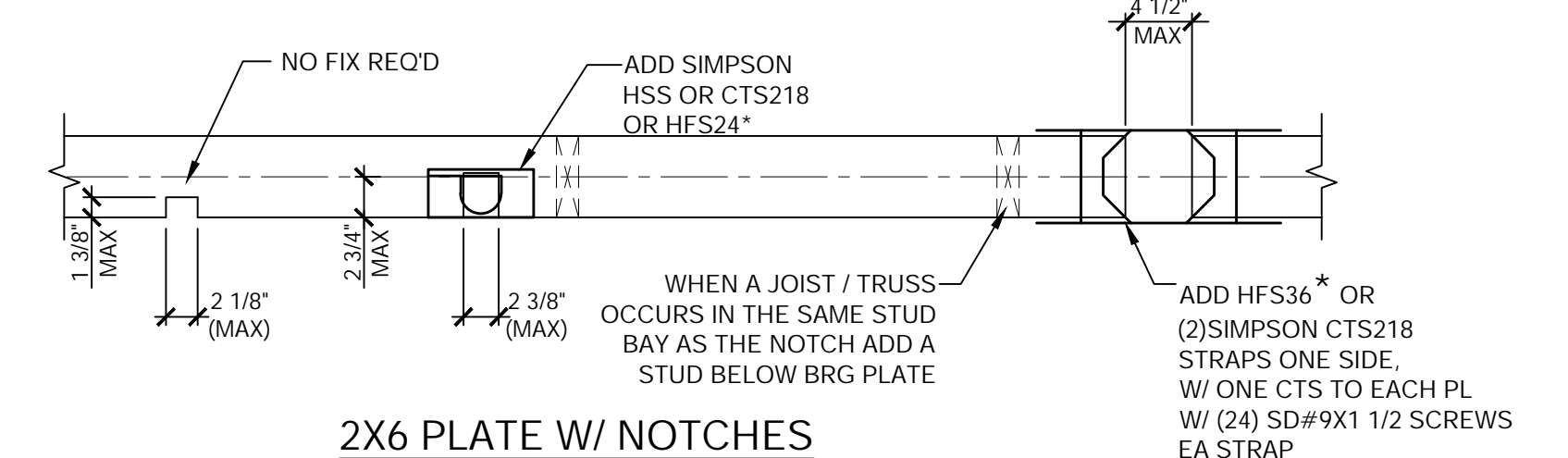
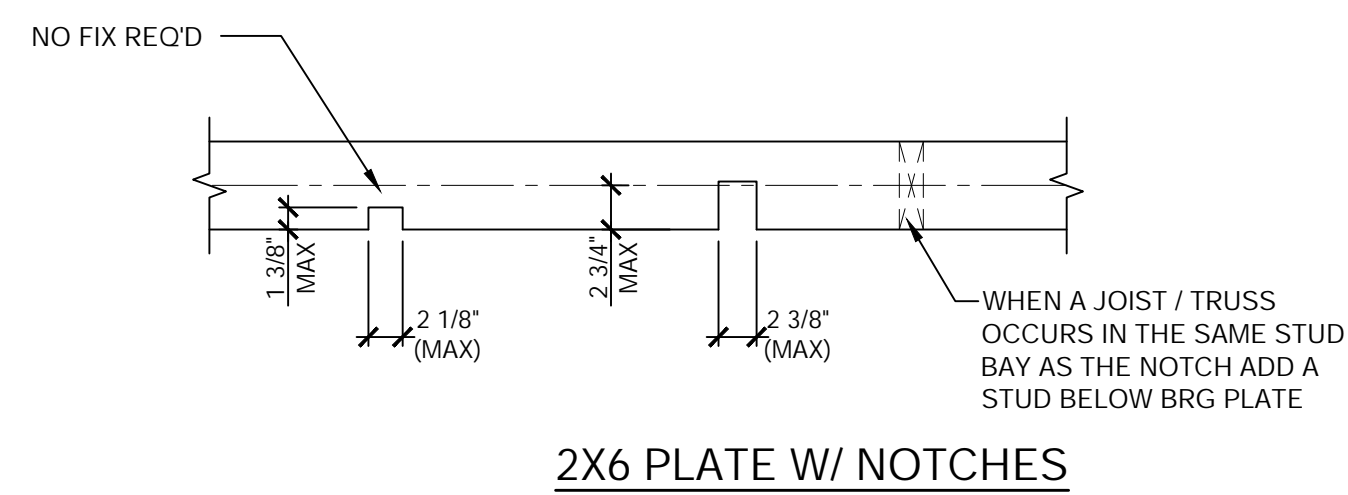
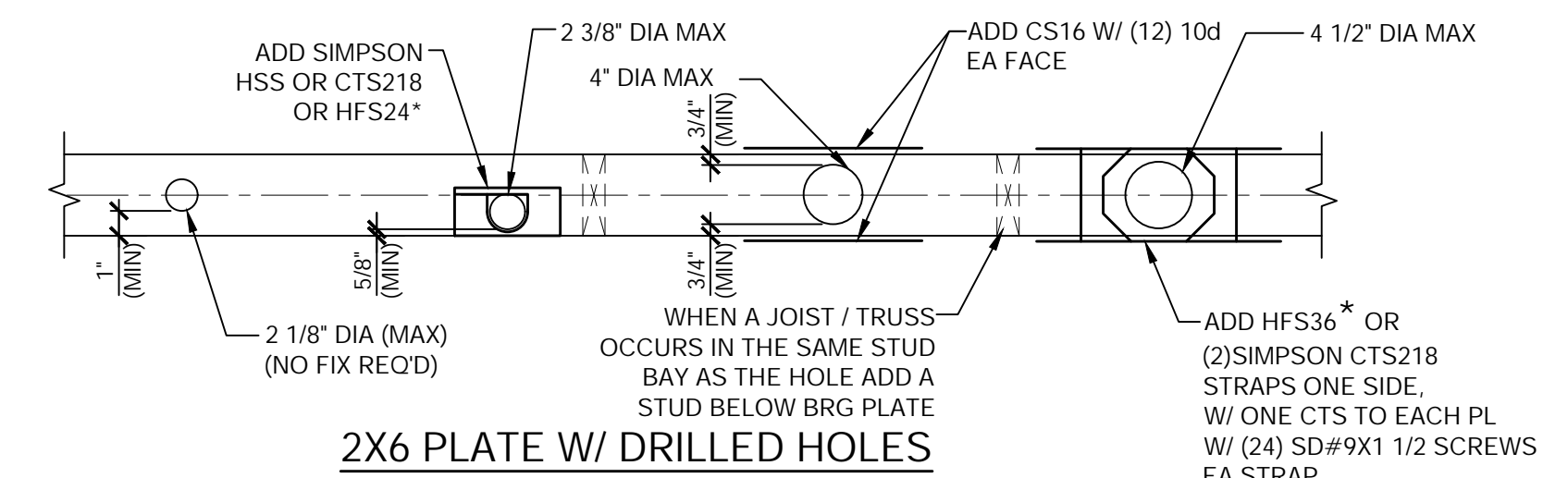
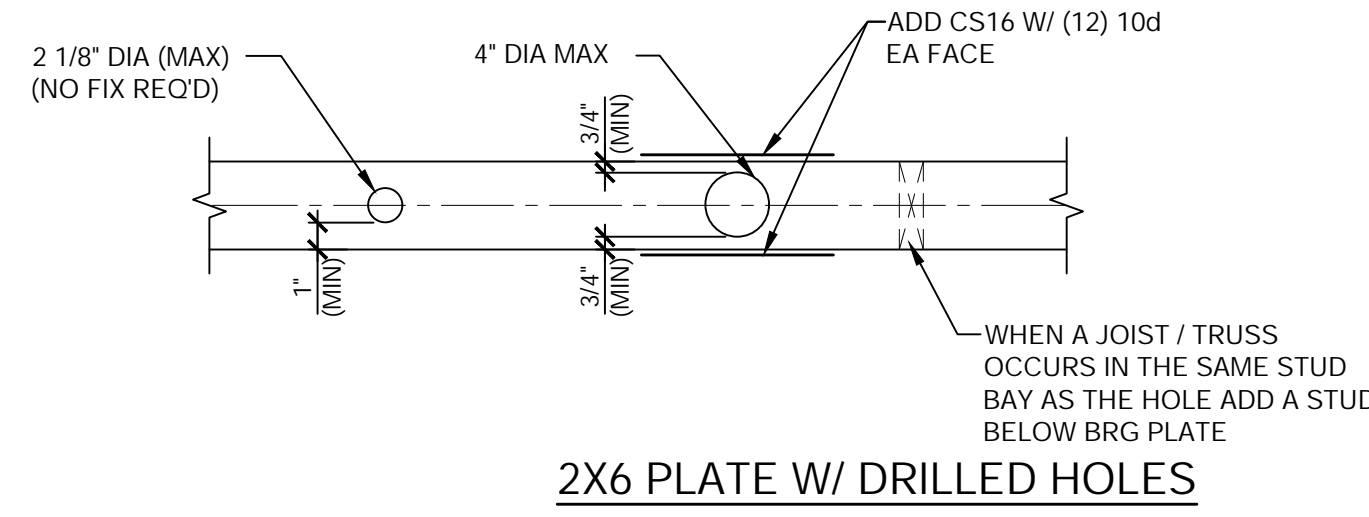
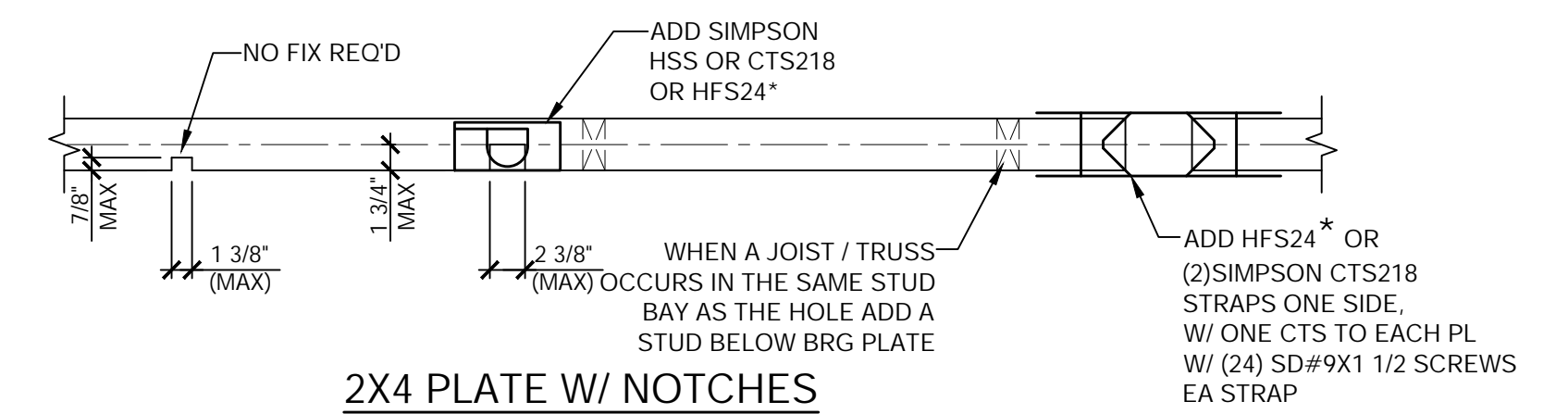
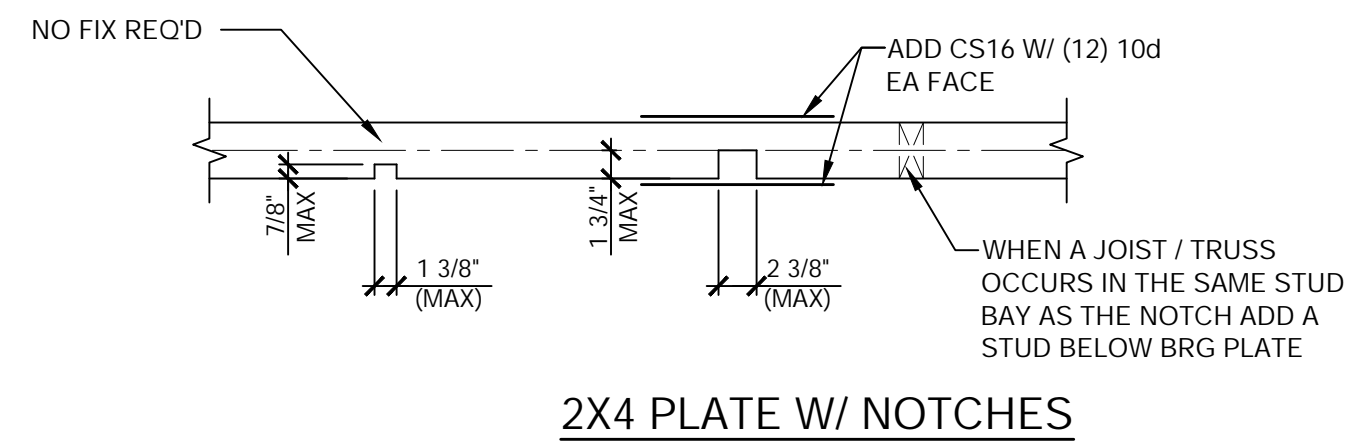
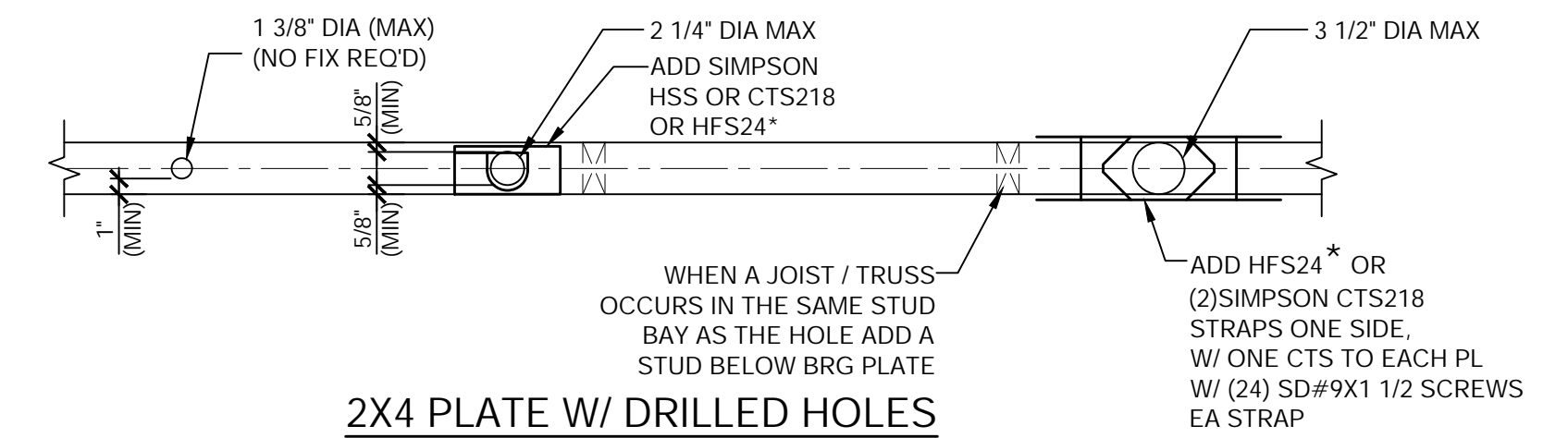
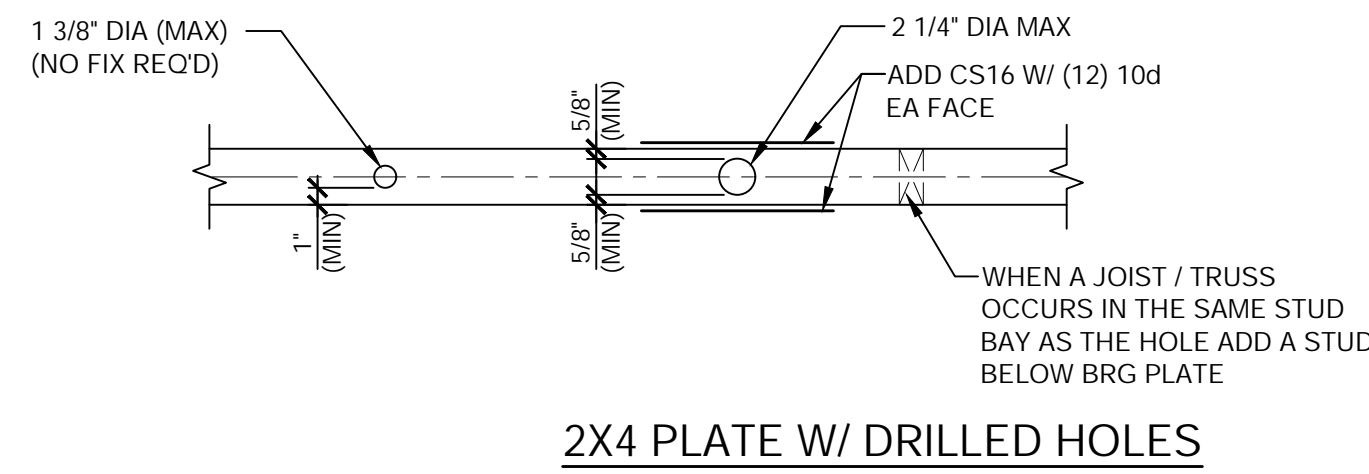
SWL	DFL / SPF	
	A35 OR LTP4	16dS
NON-SWL	32" OC	8" OC
G1, G2, WSP ABW, PFG, PFH	32" OC	8" OC
P1	18" OC	5" OC
P2/P2a	12" OC	3" OC
P3	10" OC	2" OC (STG'D)
P4	8" OC	2" OC (STG'D)
P5	12" OC EA SIDE OF PL	SDS 1/4"x4 1/2" AT 4" OC

NOTE 1: DBL-SIDED SWLS, USE 1/2 OF THE SPACING SHOWN ABOVE

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ASSURED DEVELOPMENT
 ATHENS LOT 2
 HENDERSON, NV

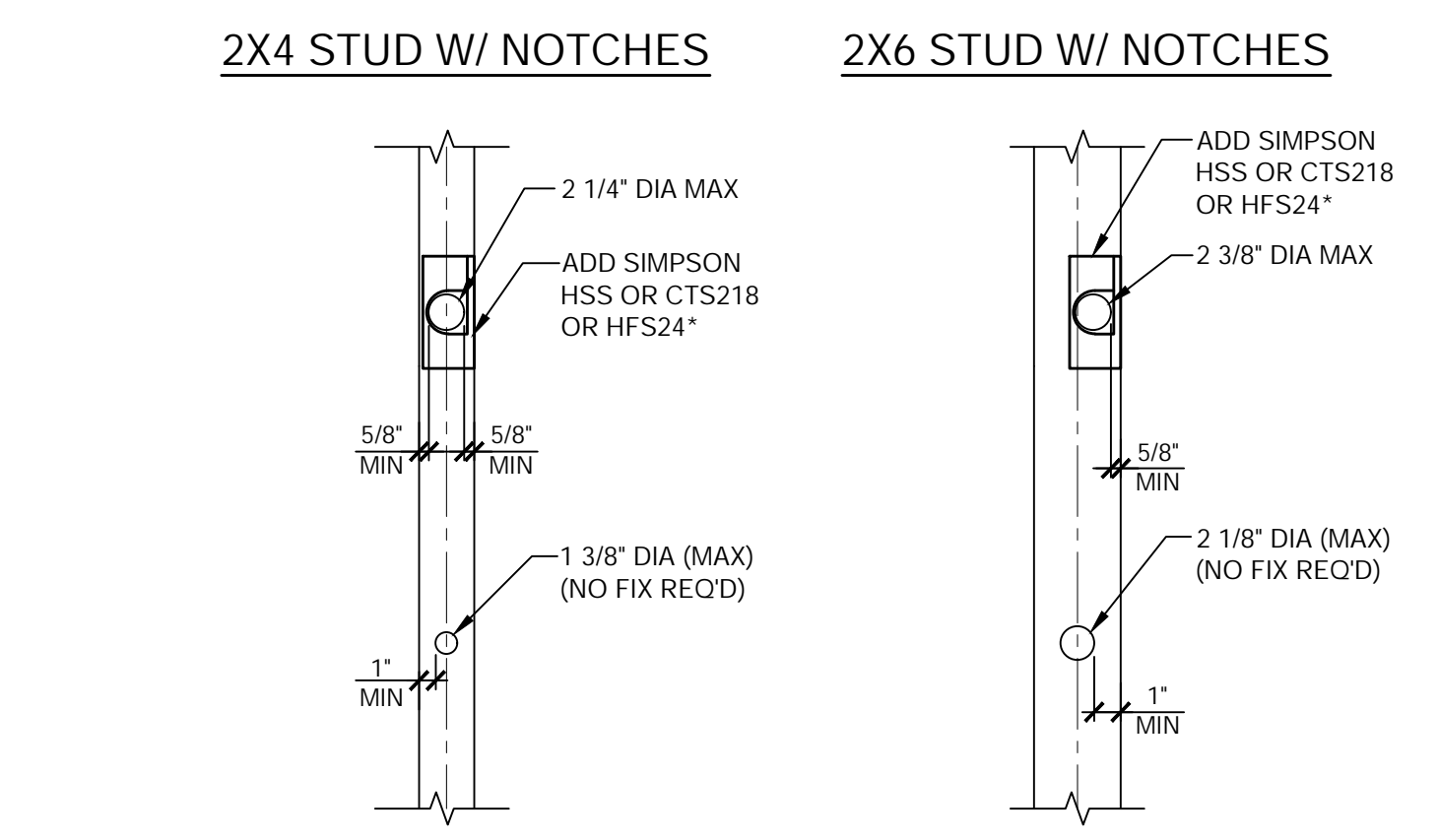
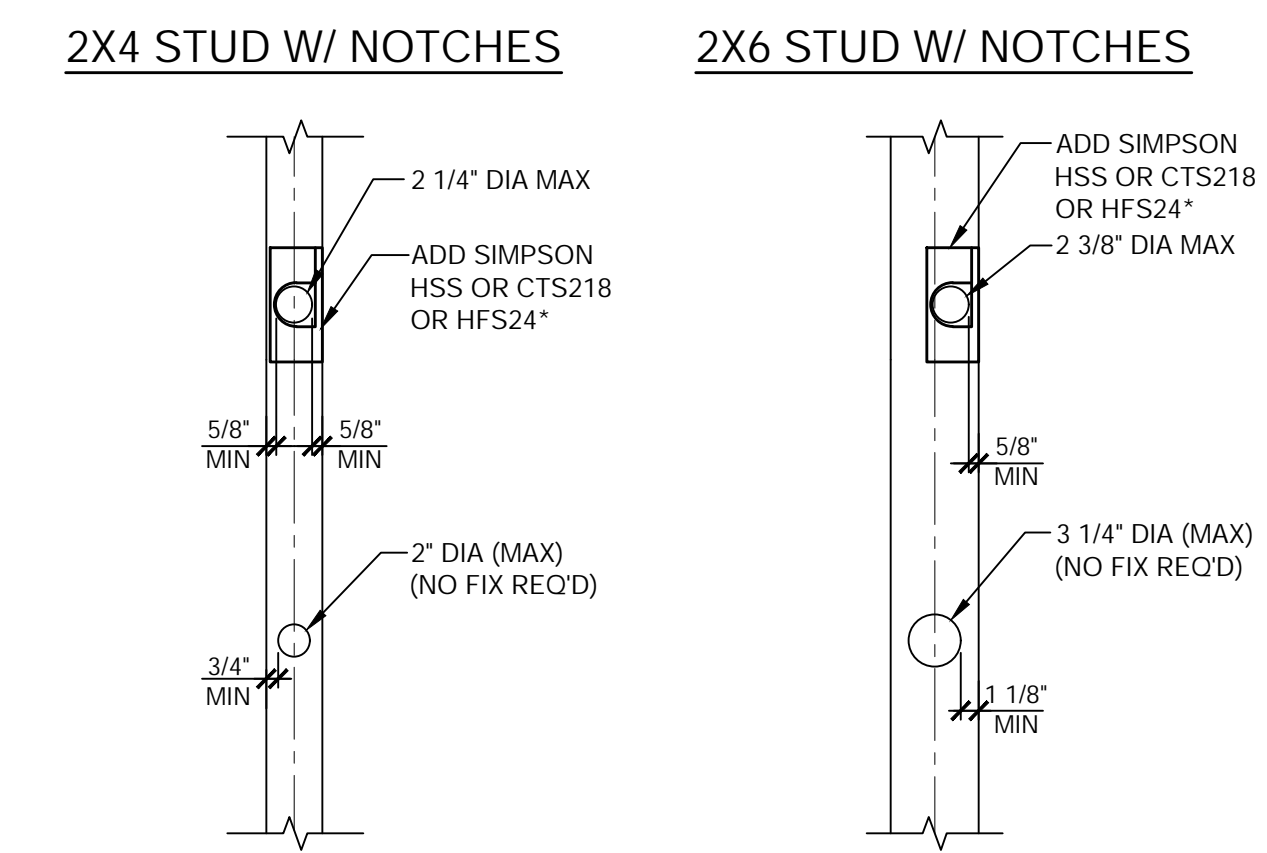
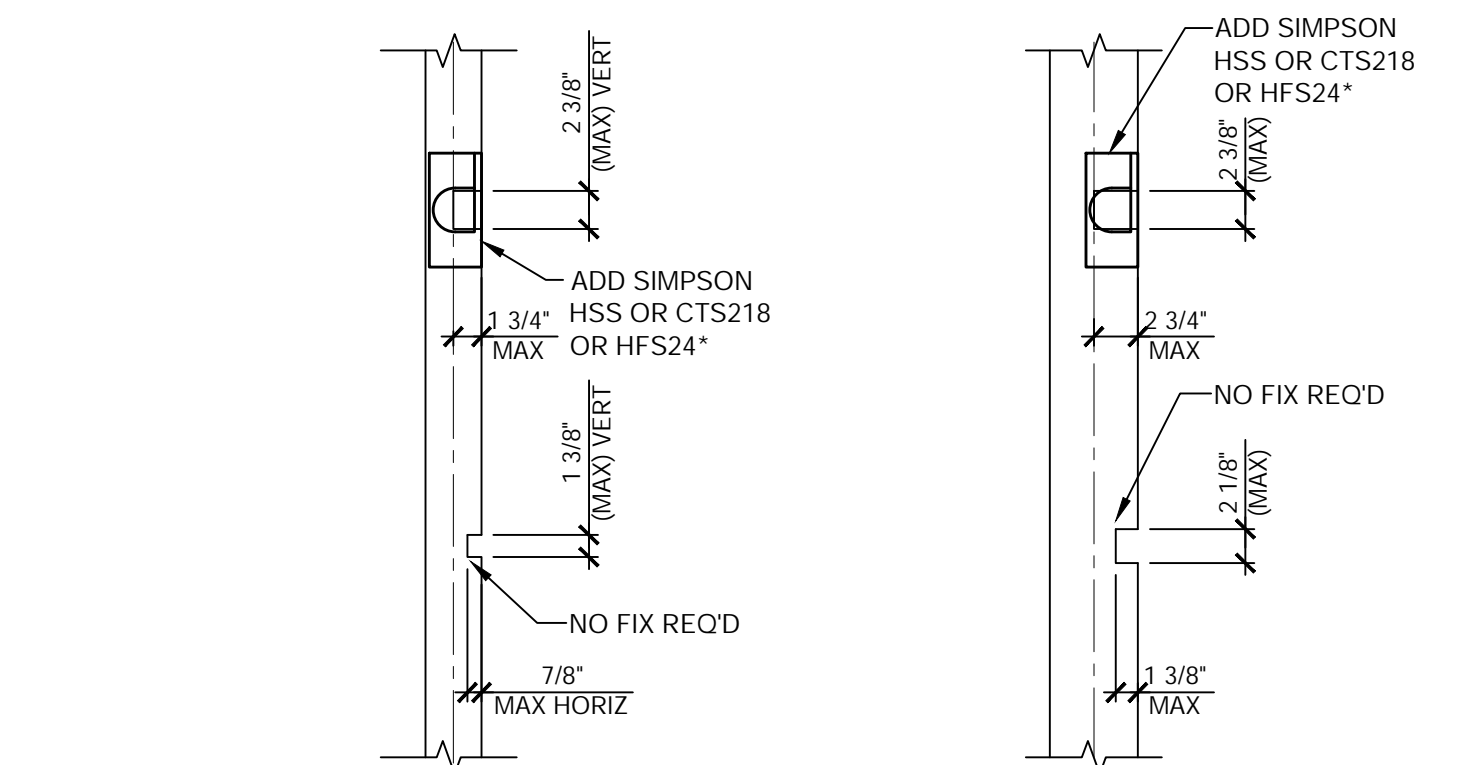
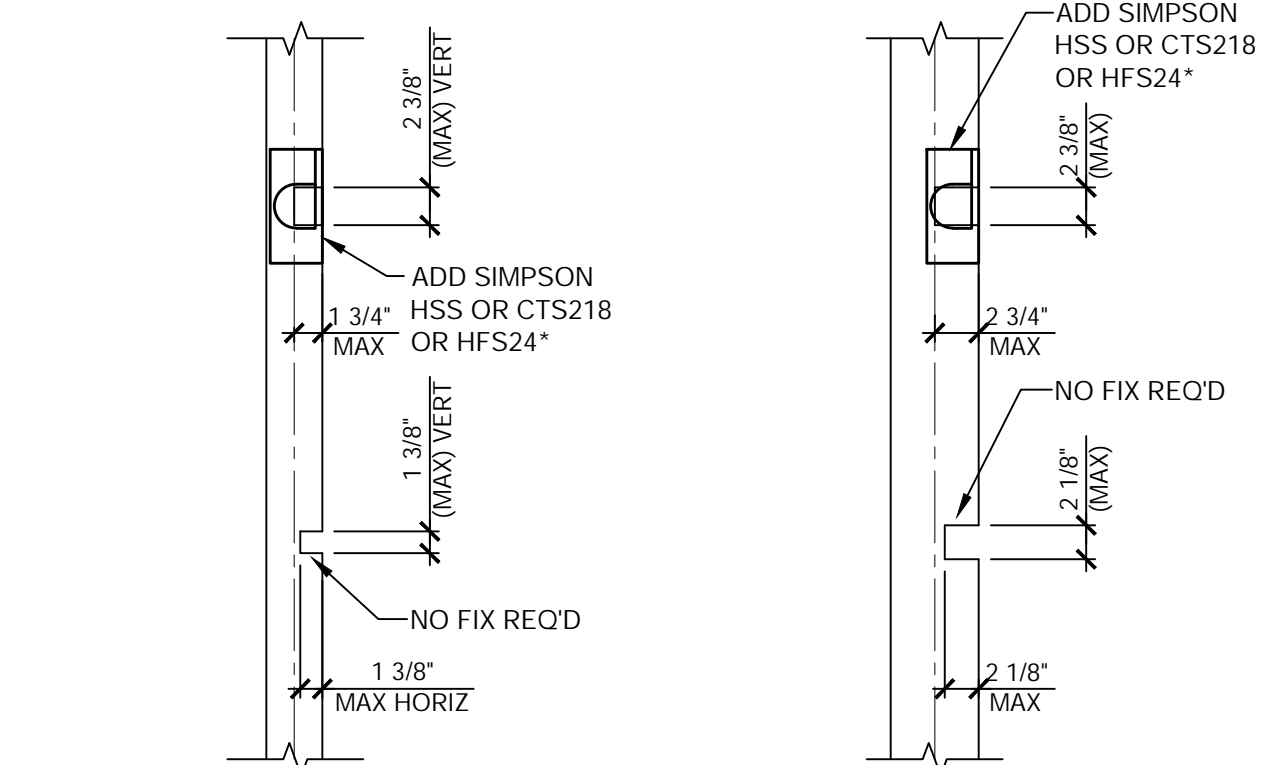




* HARDWARE MANUFACTURED BY HARDY FRAMES SEE IMFR PRODUCT CATALOG

3 ALLOWABLE HOLES AND NOTCHES IN PLATES
INTERIOR WALLS (BEARING AND NON-BEARING) NTS

1 ALLOWABLE HOLES AND NOTCHES IN PLATES
EXTERIOR WALLS AND INTERIOR SHEAR WALLS NTS



2X4 STUD W/ DRILLED HOLES 2X6 STUD W/ DRILLED HOLES

2X4 STUD W/ DRILLED HOLES 2X6 STUD W/ DRILLED HOLES

4 ALLOWABLE HOLES AND NOTCHES IN STUDS
INTERIOR NON-BEARING WALLS NTS

2 ALLOWABLE HOLES AND NOTCHES IN STUDS
EXTERIOR WALLS, INTERIOR SHEAR WALLS AND INTERIOR BEARING WALLS NTS

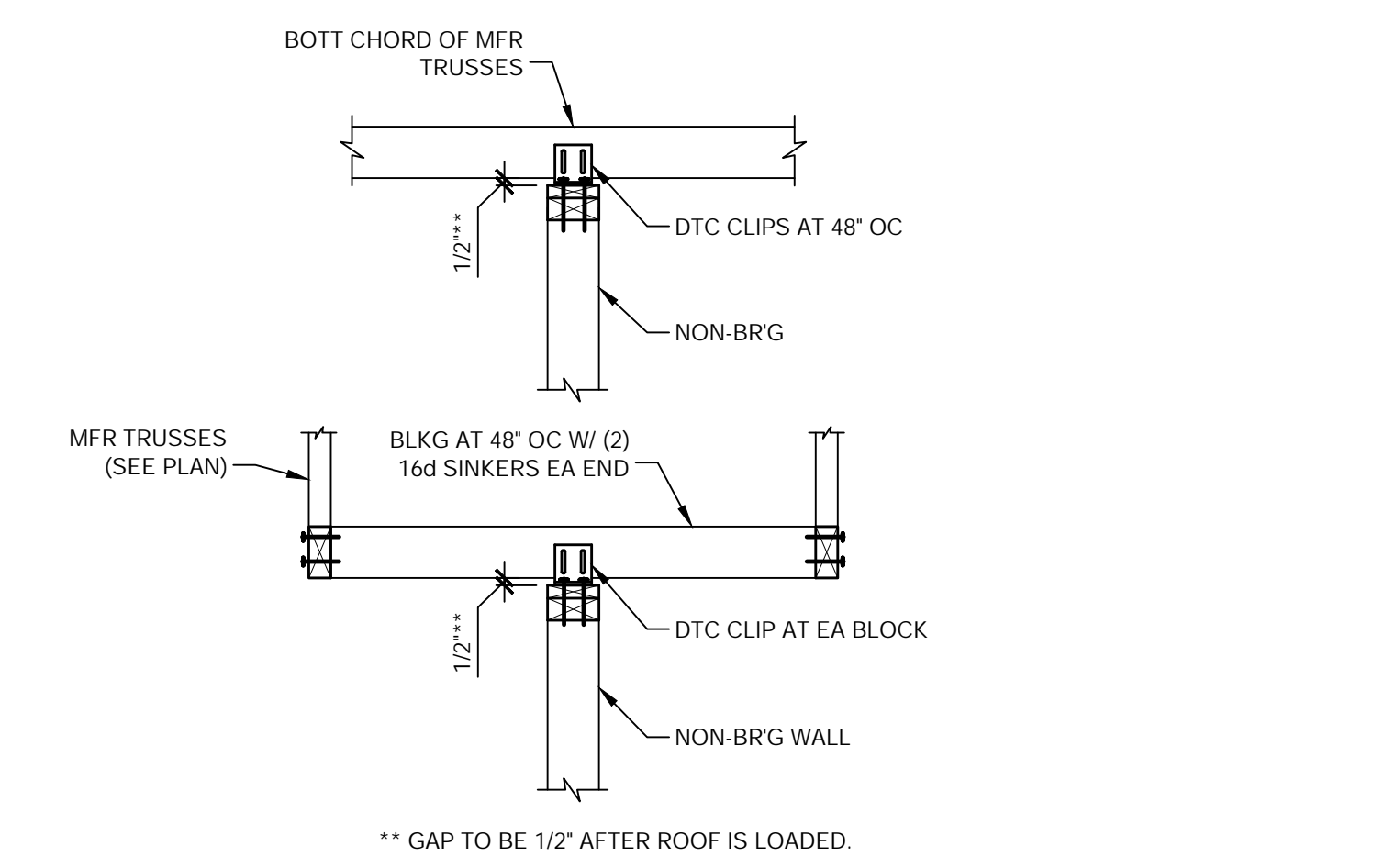
JOB NO: 1939-006-201
DESIGNED BY: SLB
DRAWN BY: ACM
ISSUED FOR:
CONSTRUCTION DOCUMENTS
DATE: 07-09-20
SHEET TITLE:
STRUCTURAL DETAILS
REVISIONS:

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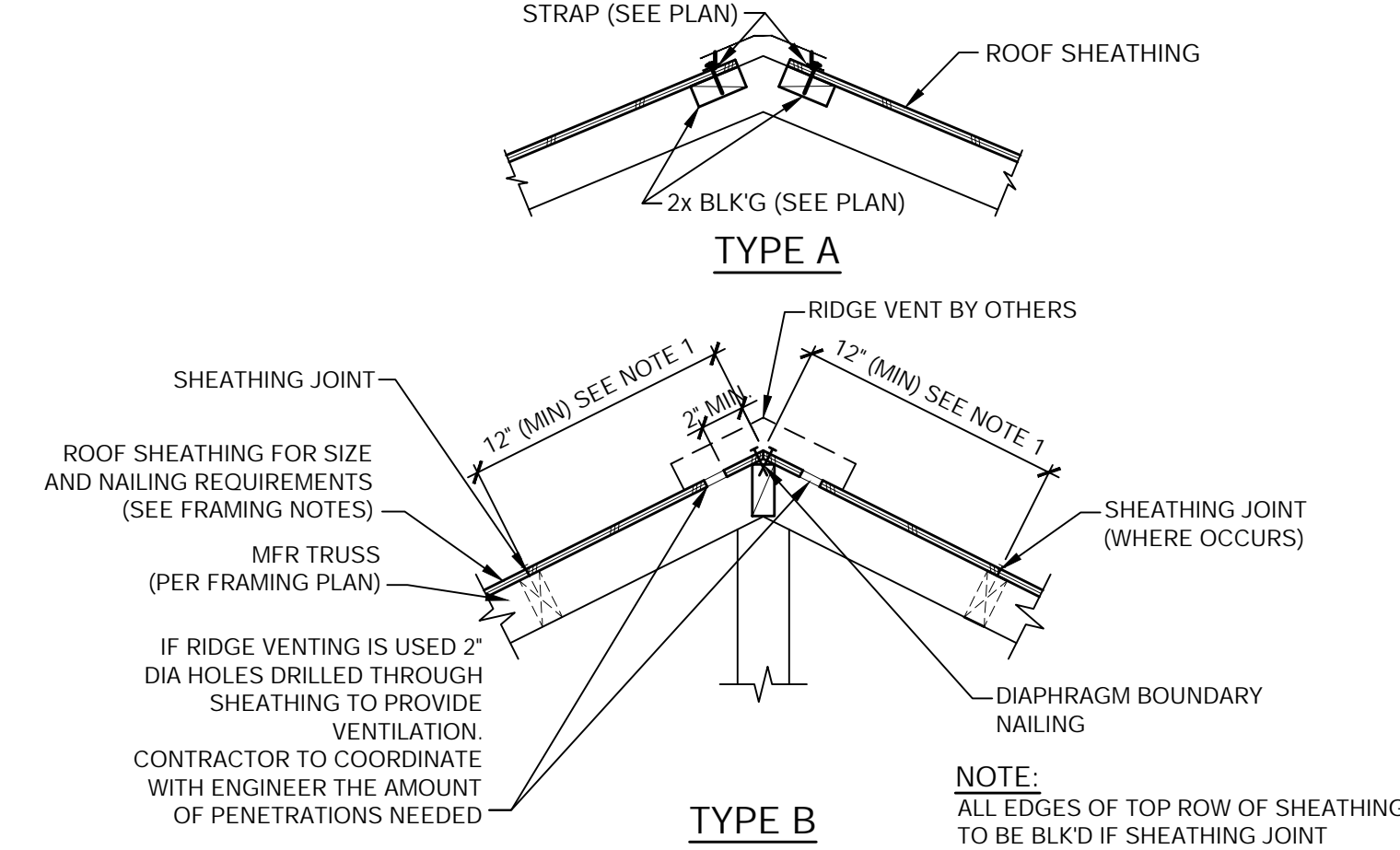
ASSURED DEVELOPMENT
ATHENS LOT 2
HENDERSON, NV



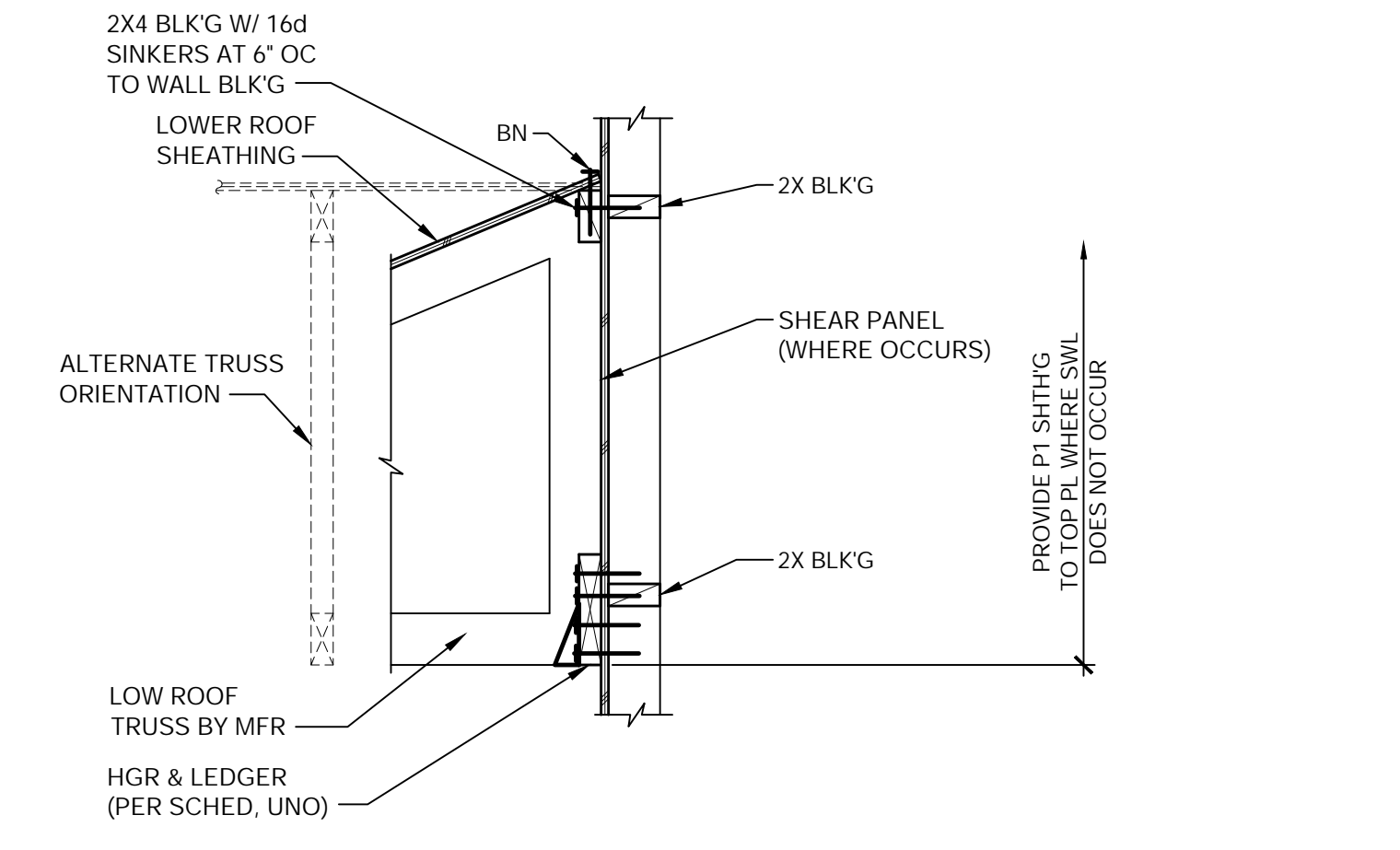
SD-2.1



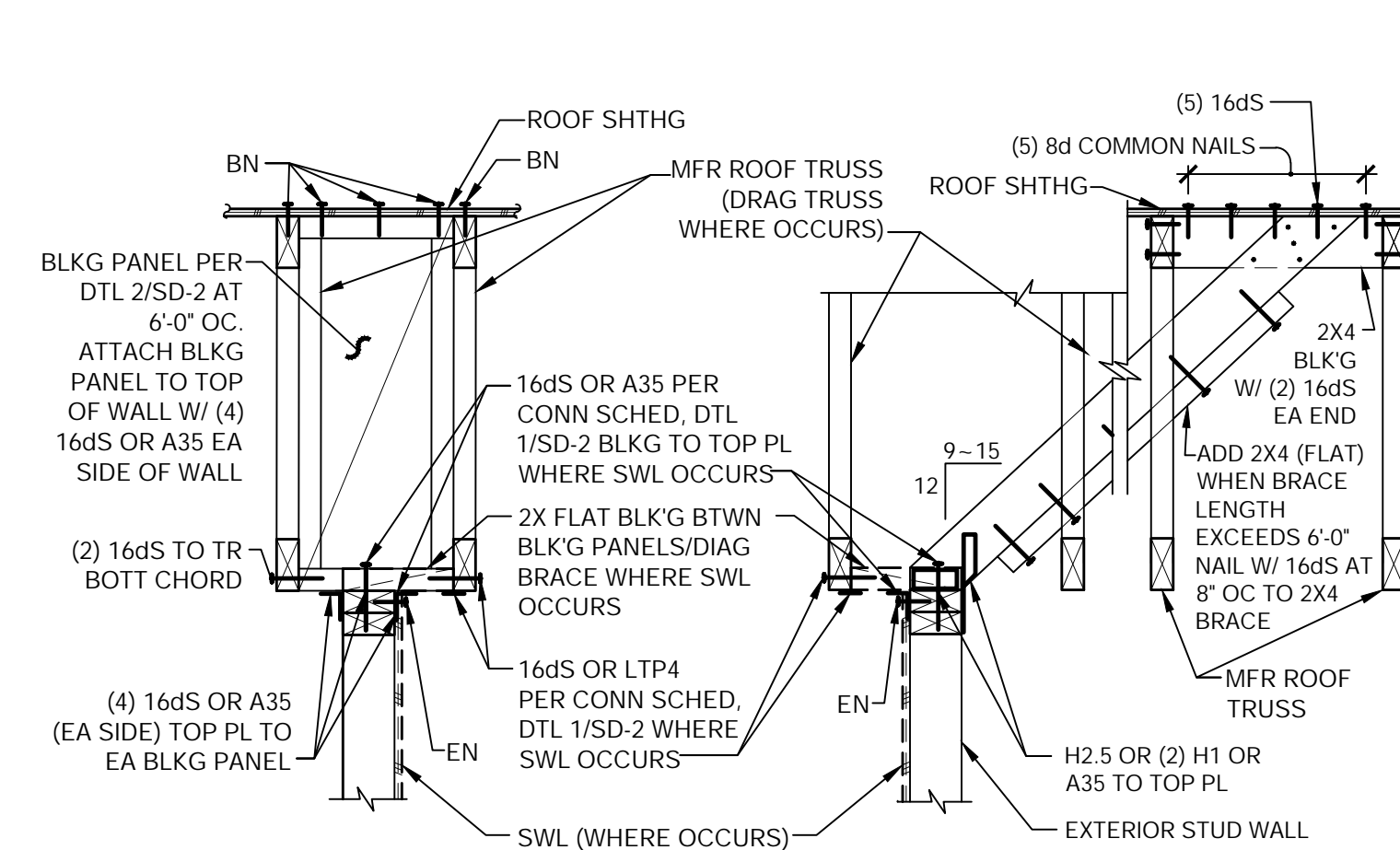
11 BRACING AT NON-BRG WALLS NTS



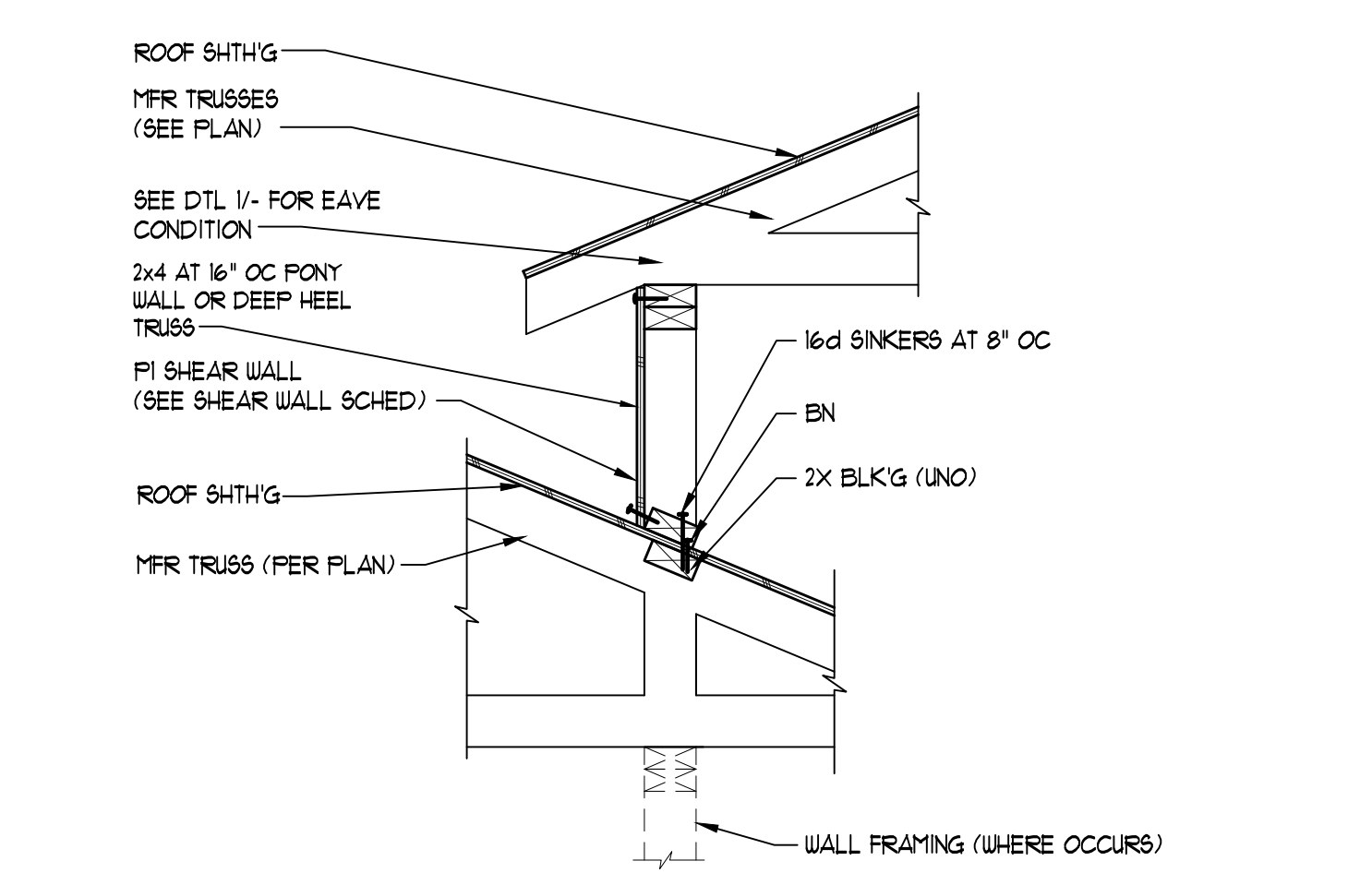
12 RIDGE BLOCKING NTS



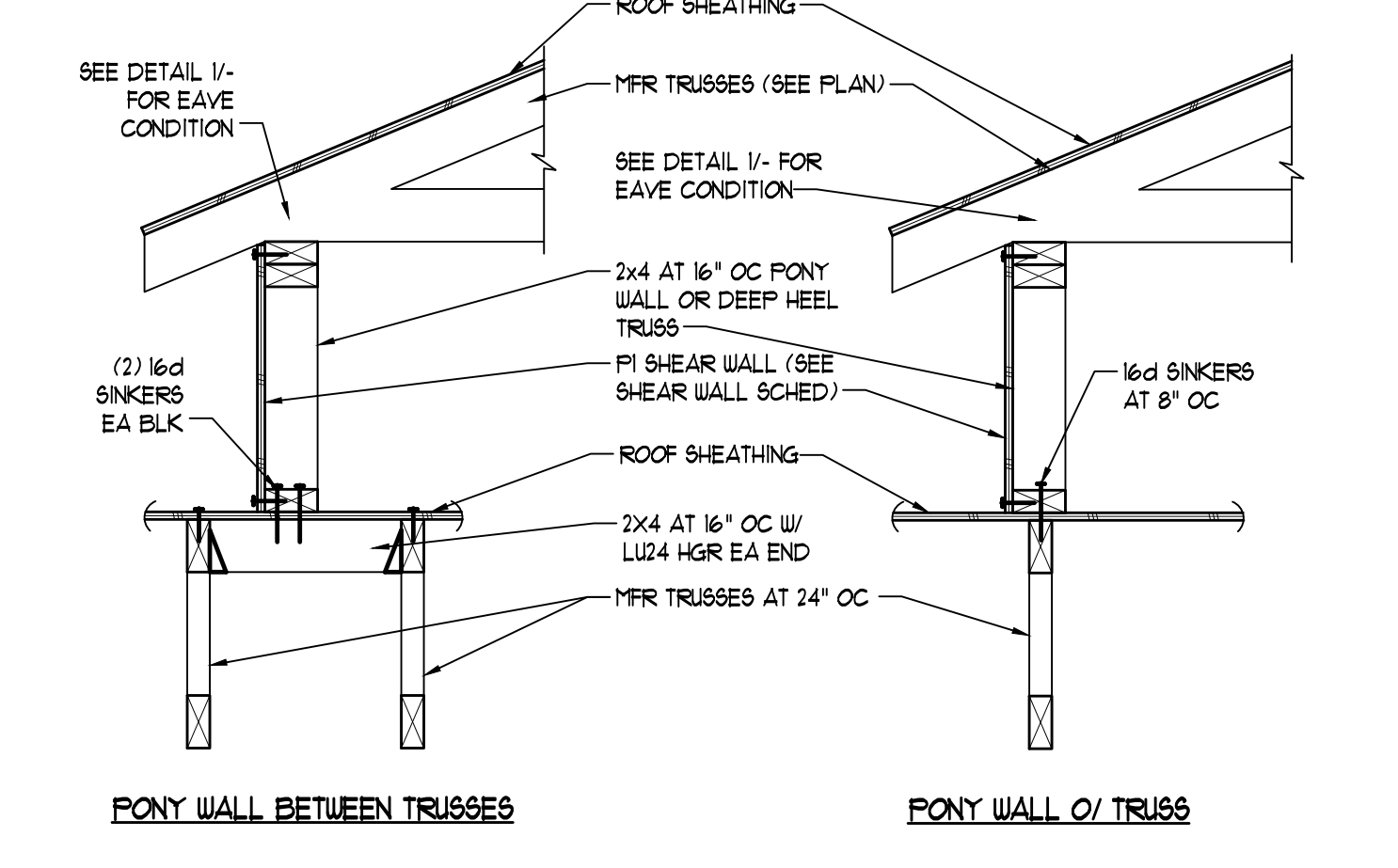
13 LOW ROOF TO WALL NTS



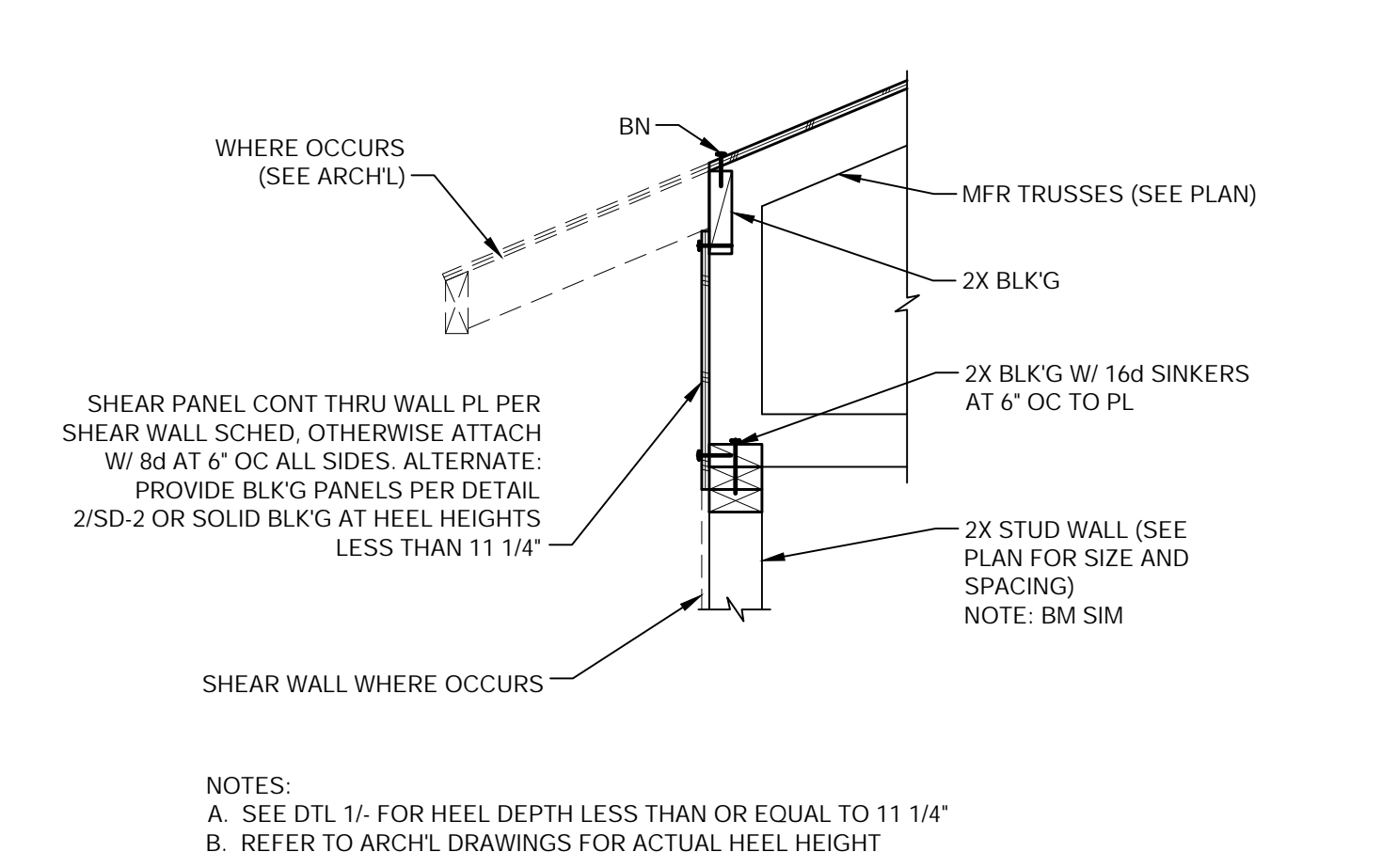
14 EXTERIOR WALL BRACING NTS



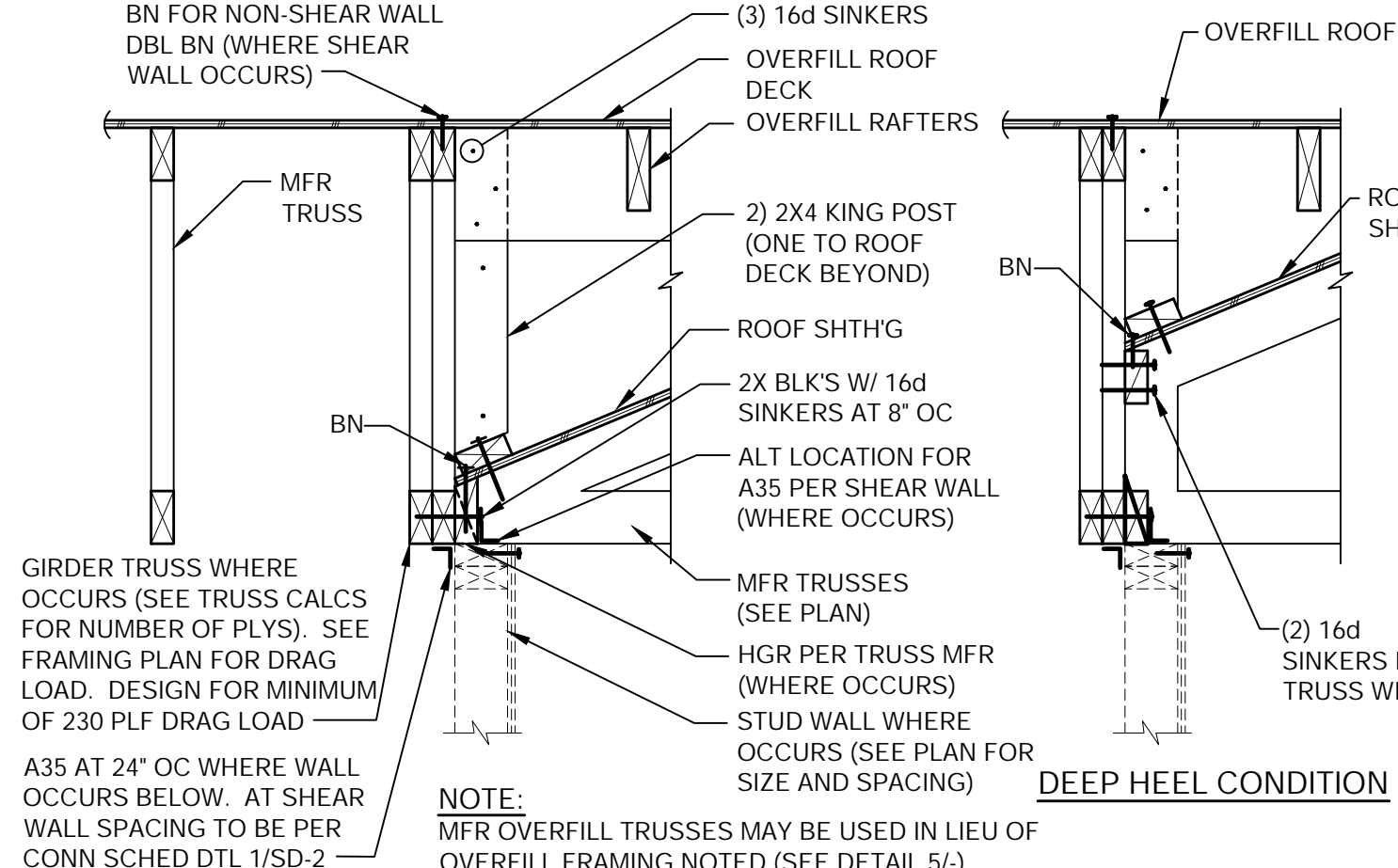
7 PONY WALL NTS



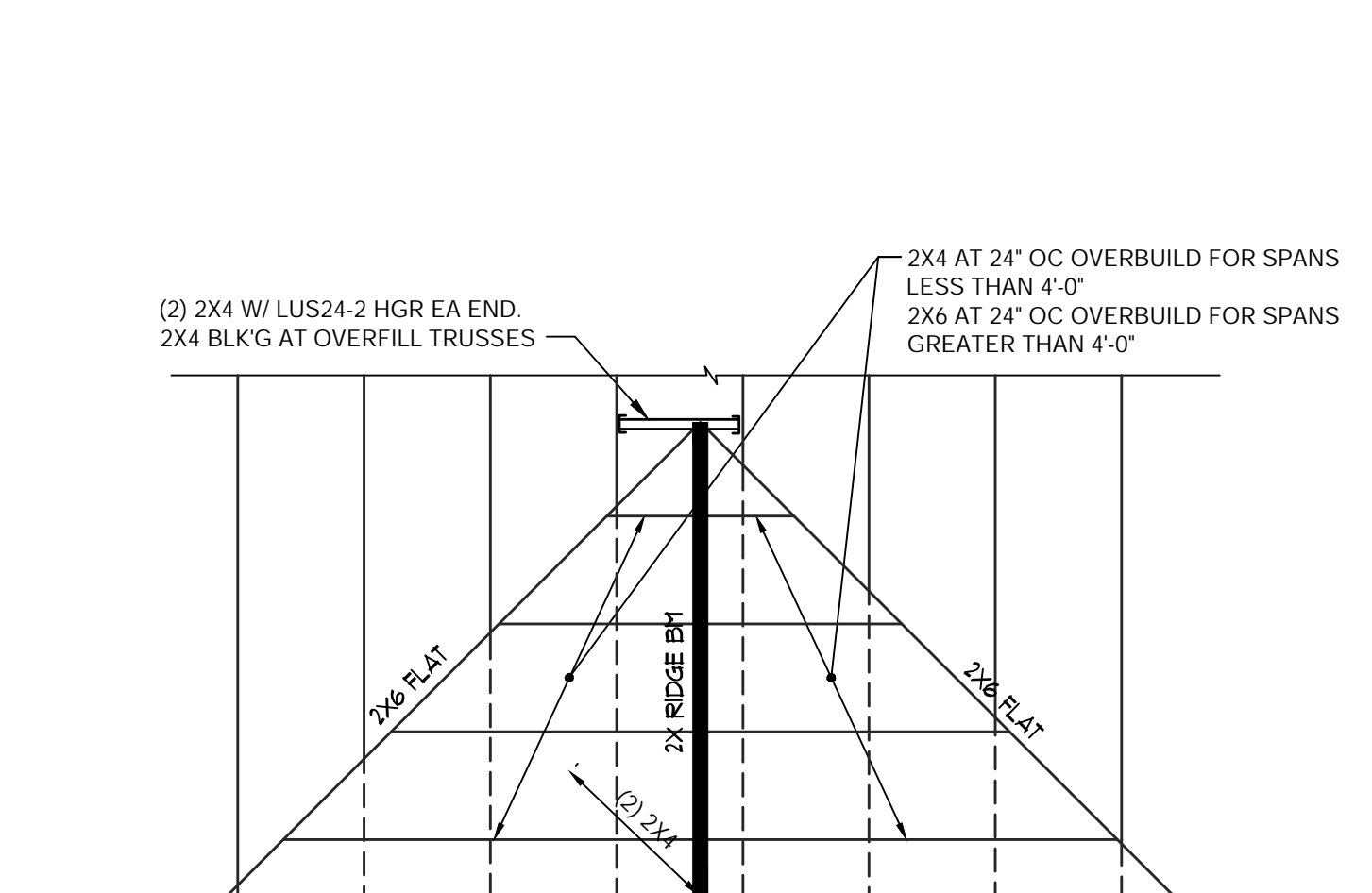
8 PONY WALL NTS



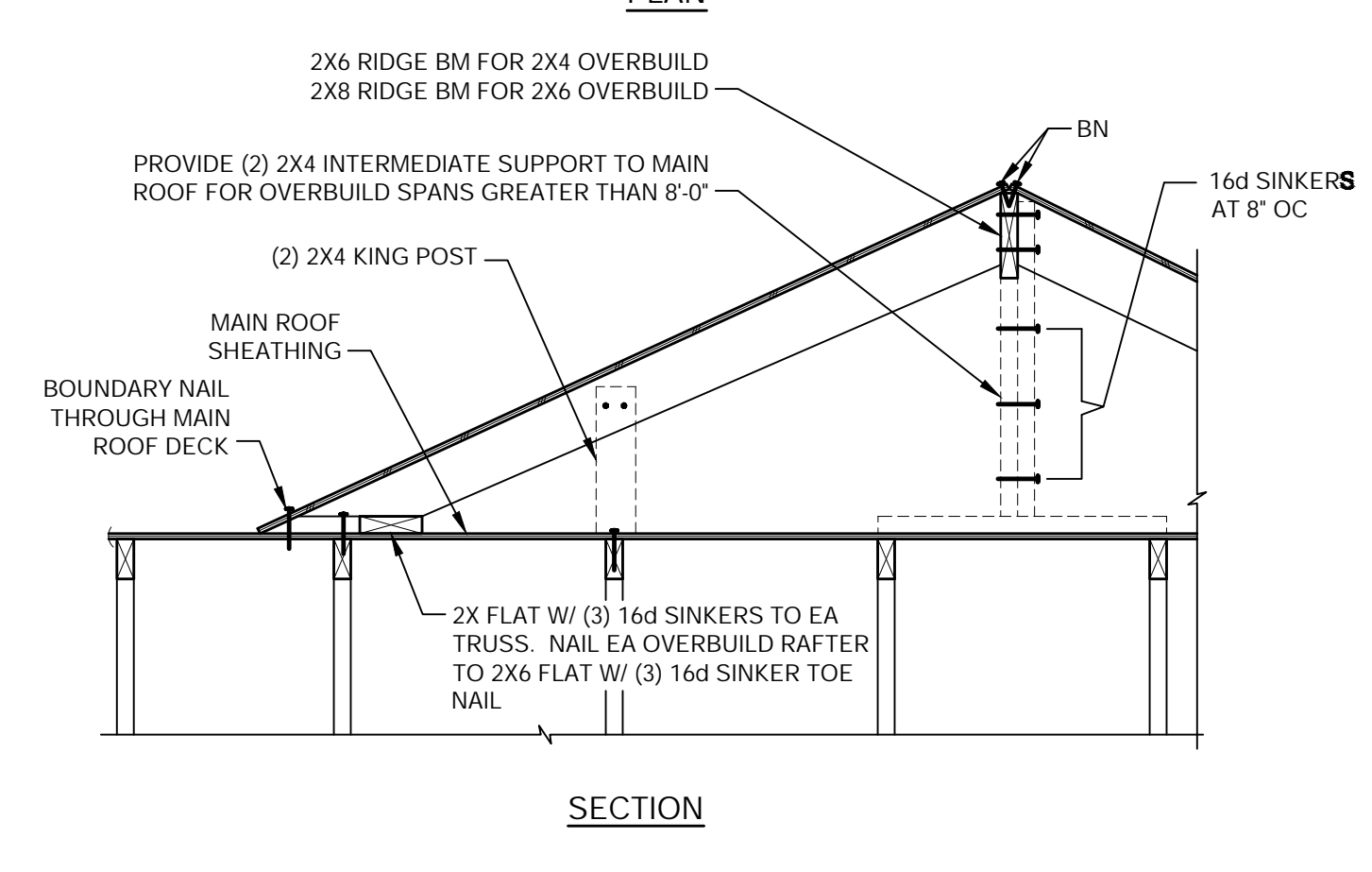
9 TRUSS W/ DEEP HEEL AT BEARING NTS



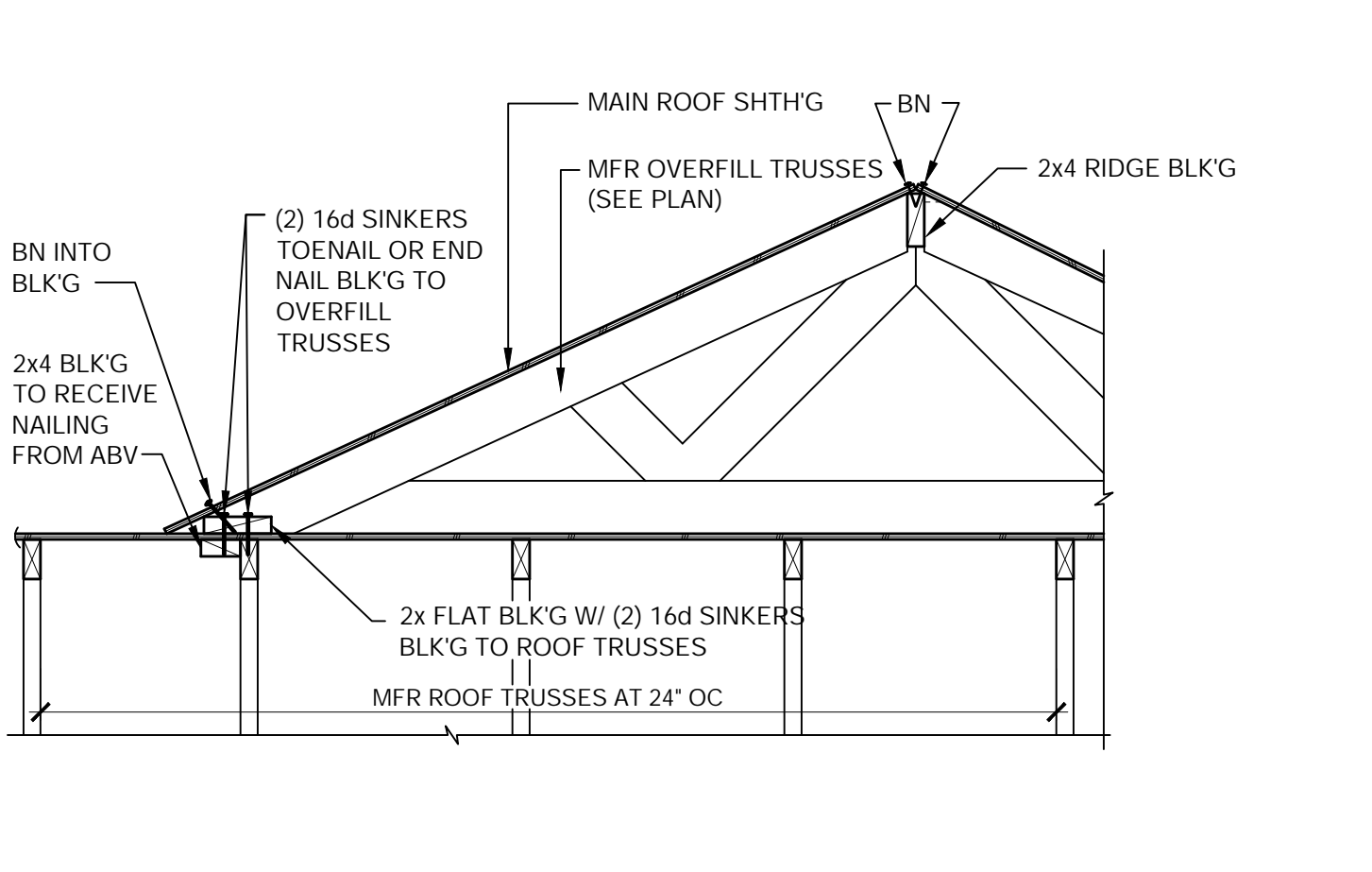
10 TRUSS TO GIRDER NTS



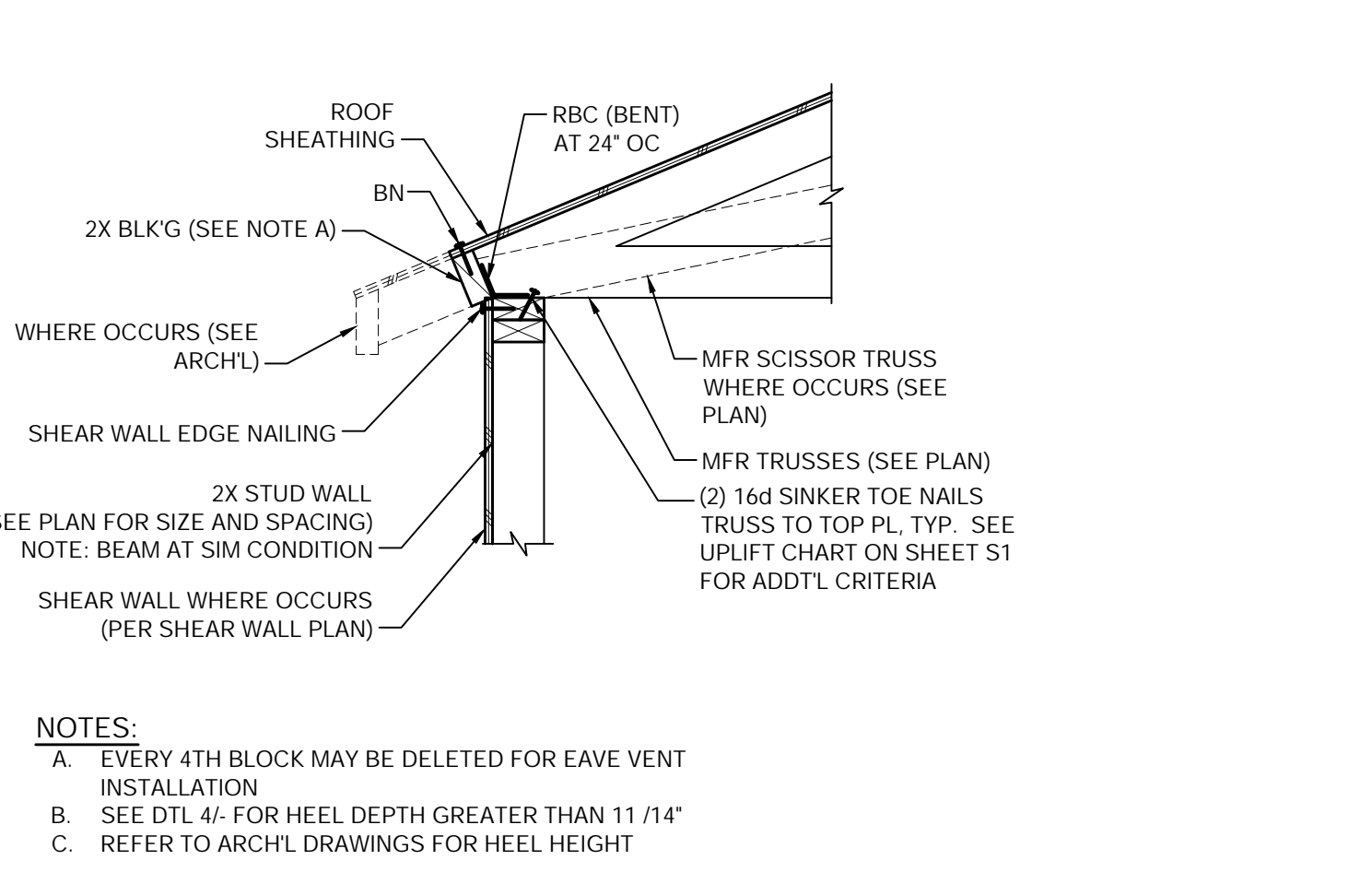
4 OVERFILL FRAMING NTS



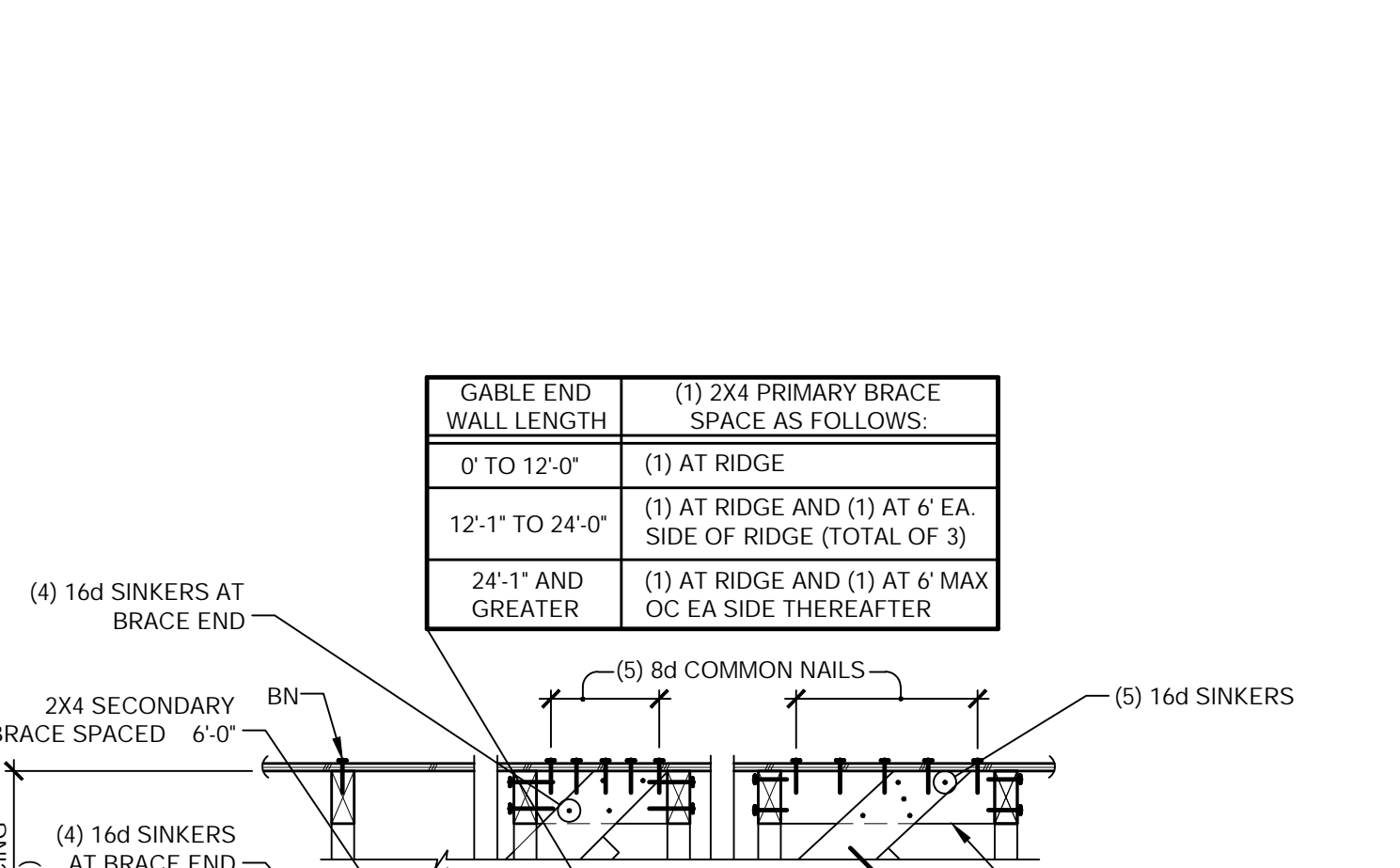
5 OVERFILL FRAMING NTS



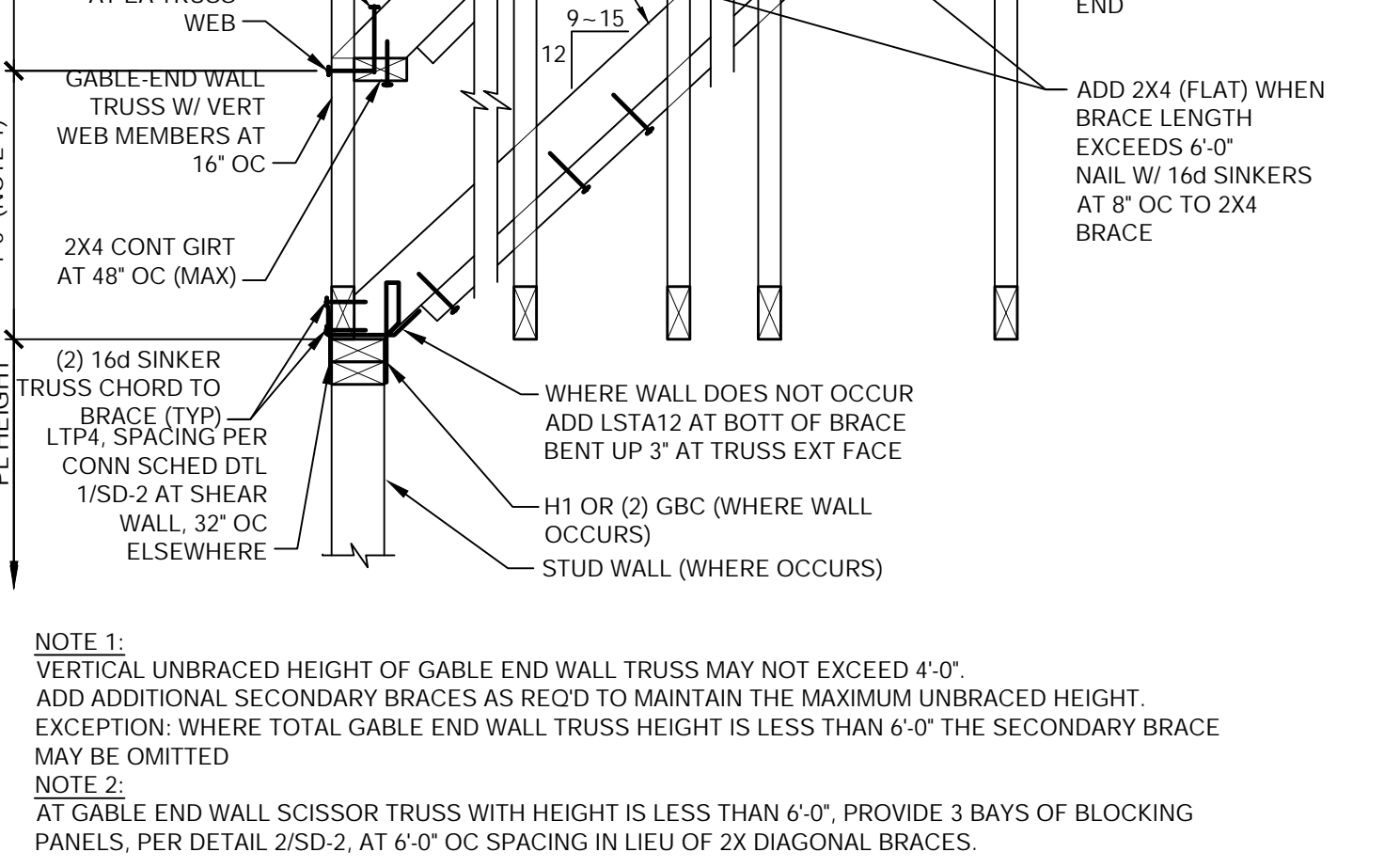
6 STEP IN ROOF NTS



1 EAVE CONDITION NTS



2 GABLE END WALL TRUSS NTS

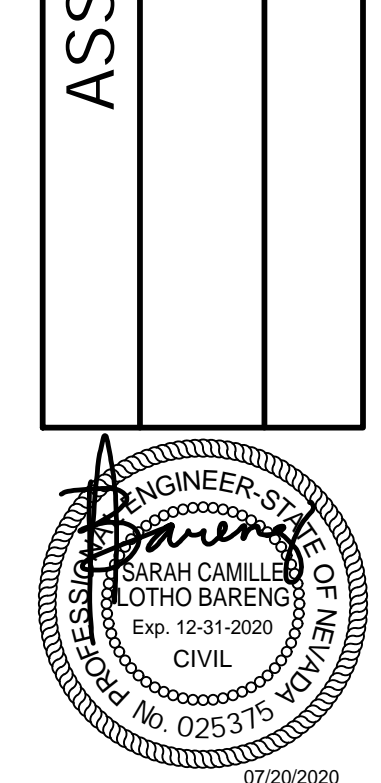


3 2X FLAT OUTRIGGER NTS

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CIVIL FORENSICS PLANNING STRUCTURAL SURVEY

ASSURED DEVELOPMENT
 ATHENS LOT 2
 HENDERSON, NV



SD-3

REVISIONS:

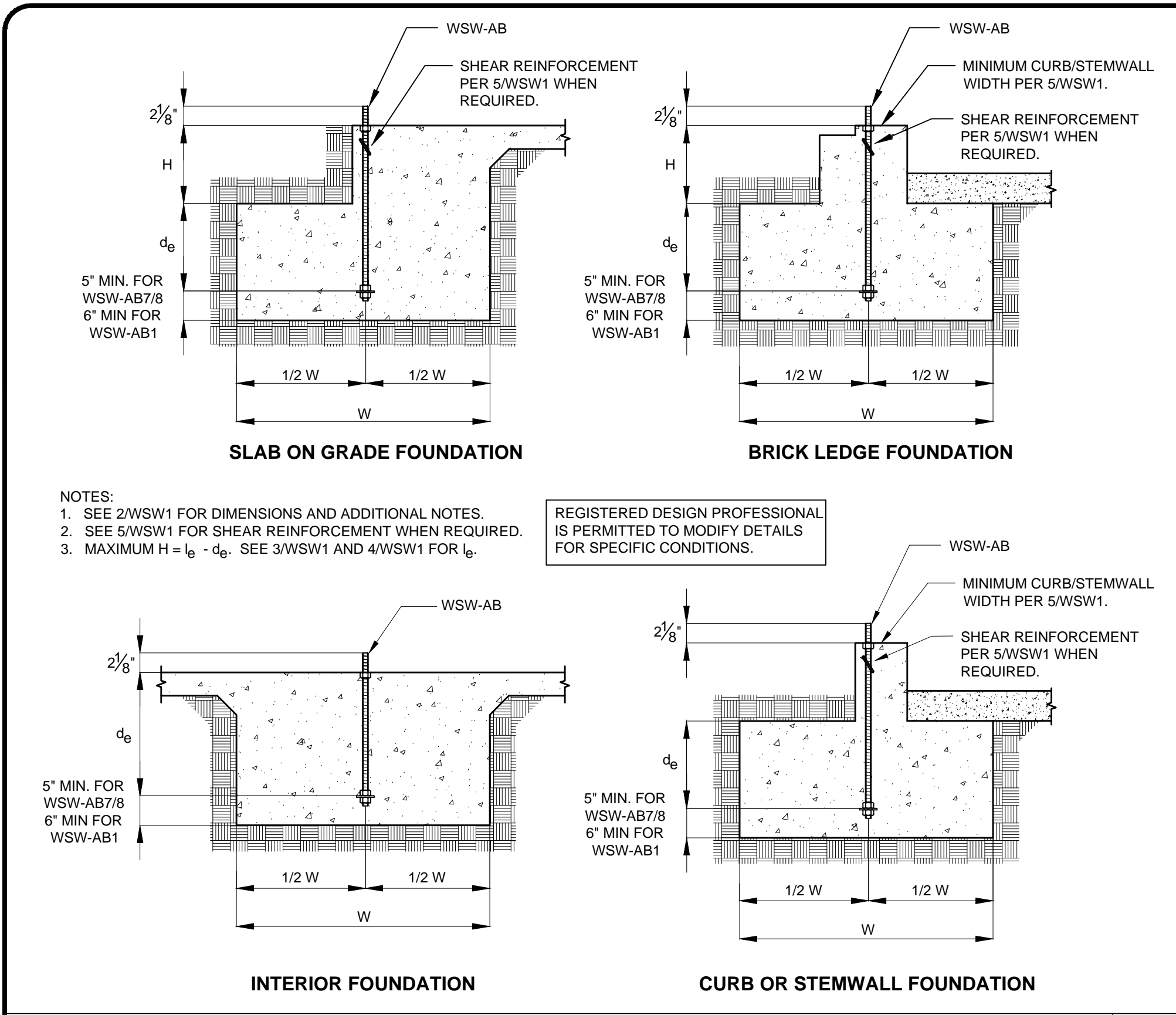
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CIVIL FORENSICS PLANNING STRUCTURAL SURVEY

ASSURED DEVELOPMENT
 ATHENS LOT 2
 HENDERSON, NV

REGISTERED PROFESSIONAL ENGINEER
 SARAH CAMILLE BARENGO
 No. 12-31-2020
 CIVIL
 No. 025375
 07/20/2020

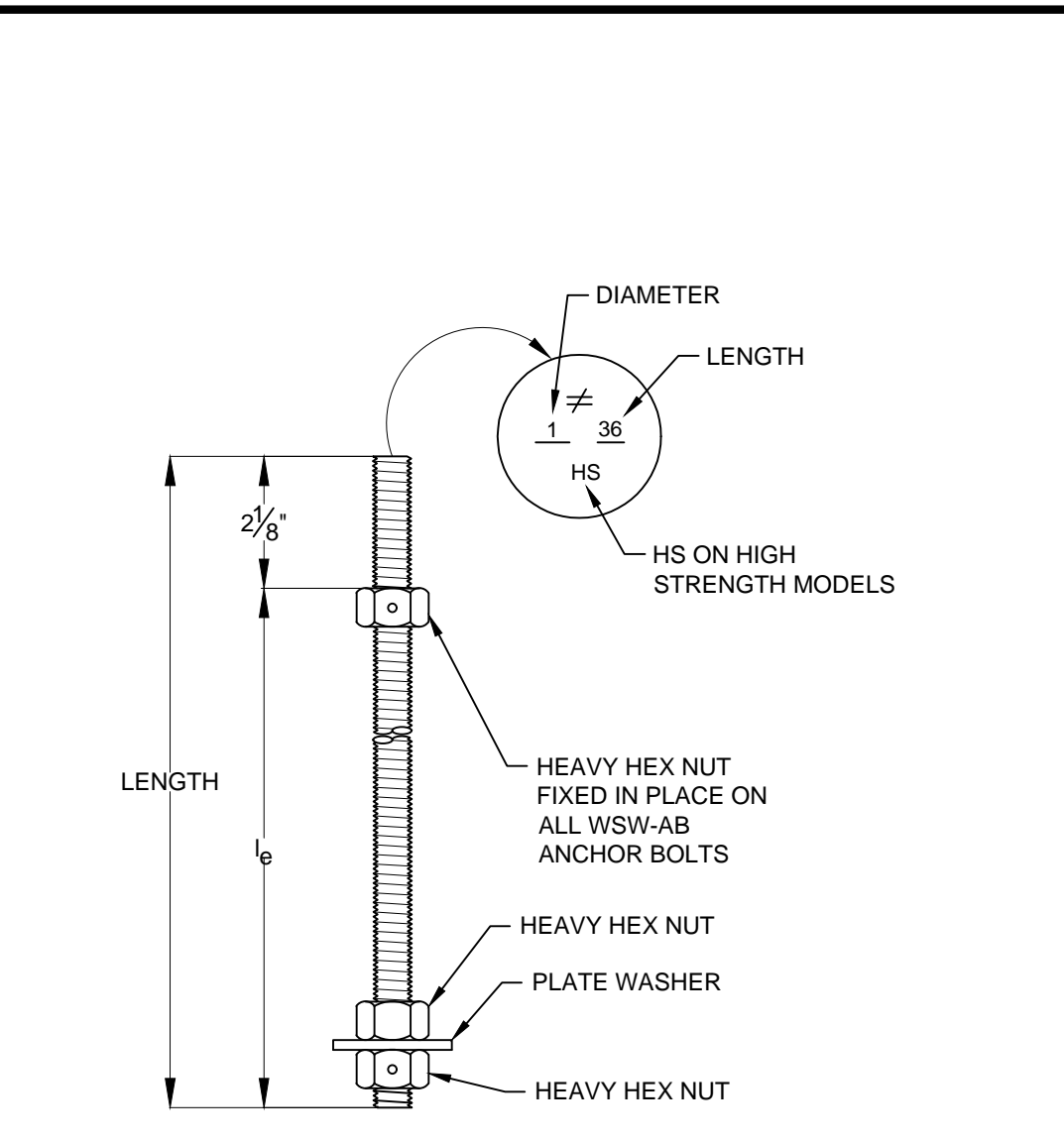
WSW1



NOTES:
 1. SEE 2WSW1 FOR DIMENSIONS AND ADDITIONAL NOTES.
 2. SEE 5WSW1 FOR SHEAR REINFORCEMENT WHEN REQUIRED.
 3. MAXIMUM H = l_e - d_b. SEE 3WSW1 AND 4WSW1 FOR l_e.

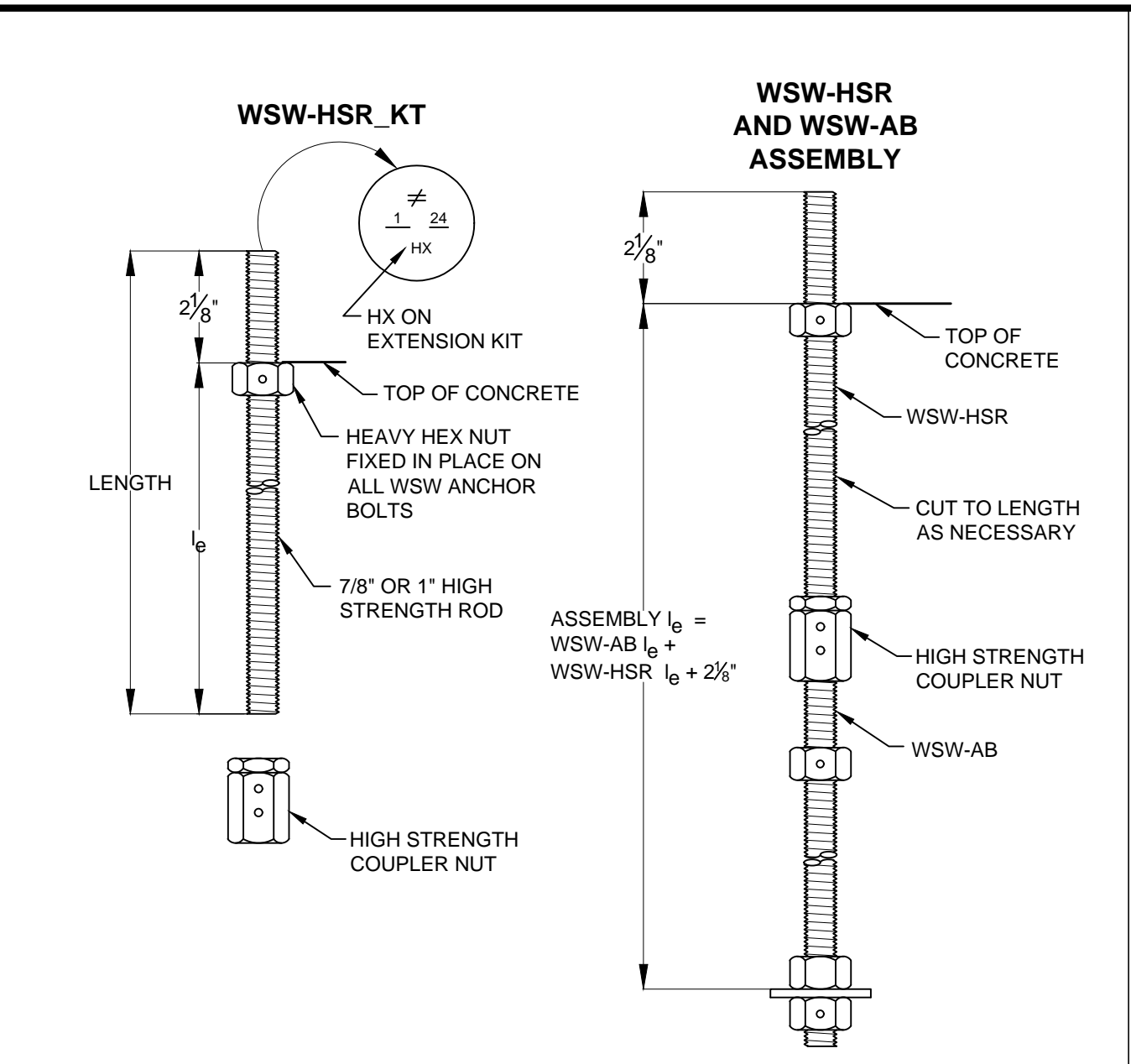
REGISTERED DESIGN PROFESSIONAL IS PERMITTED TO MODIFY DETAILS FOR SPECIFIC CONDITIONS.

STRONG-WALL® WSW ANCHORAGE - TYPICAL SECTIONS 1



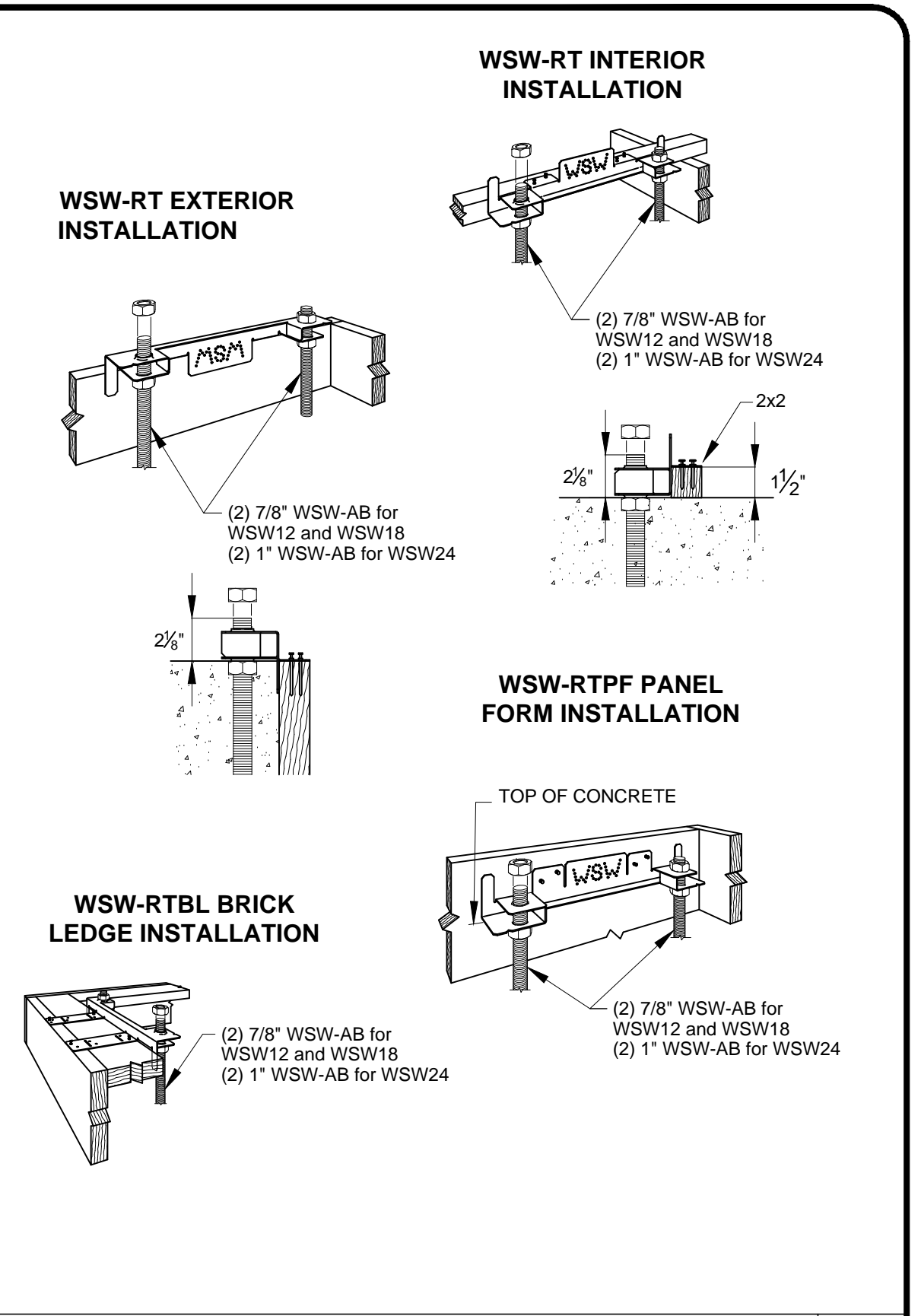
WSW PANEL MODEL	MODEL NO.	DIAMETER	LENGTH	l _e
WSW12 AND WSW18	WSW-AB7/8x24	7/8"	24"	20"
	WSW-AB7/8x24HS	7/8"	24"	20"
	WSW-AB7/8x30	7/8"	30"	26"
	WSW-AB7/8x30HS	7/8"	30"	26"
	WSW-AB7/8x36HS	7/8"	36"	32"
WSW24	WSW-AB1x24	1"	24"	20"
	WSW-AB1x24HS	1"	24"	20"
	WSW-AB1x30	1"	30"	26"
	WSW-AB1x30HS	1"	30"	26"
	WSW-AB1x36HS	1"	36"	32"

WSW ANCHOR BOLTS 3

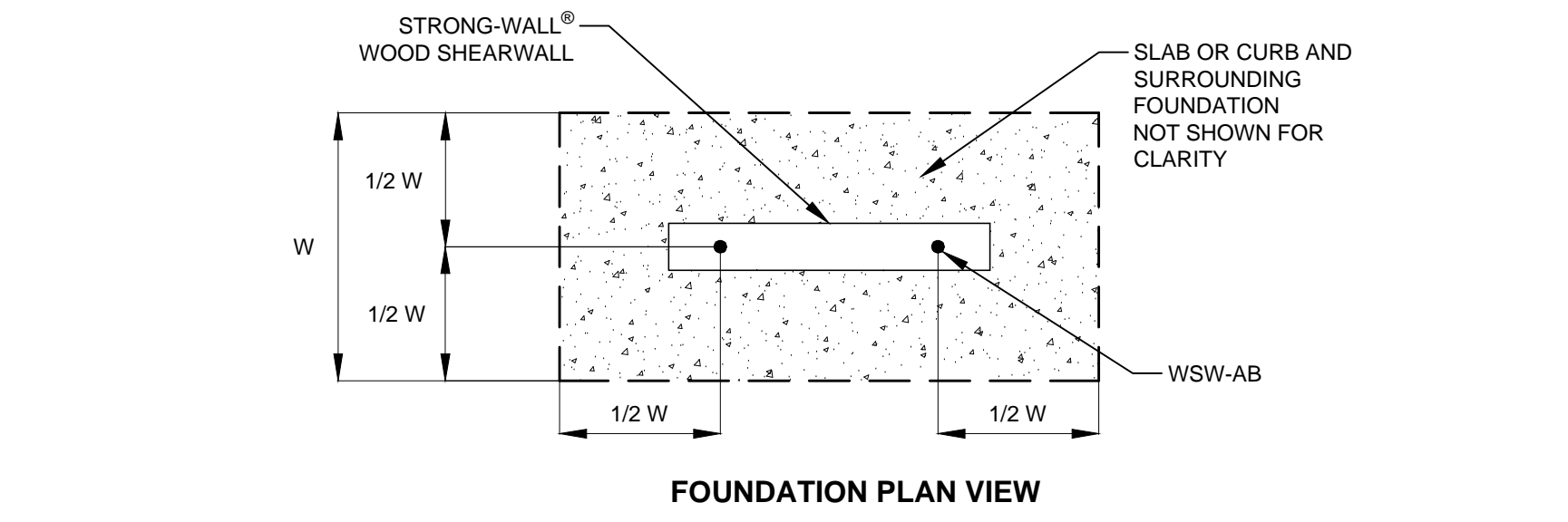


WSW PANEL MODEL	MODEL NO.	DIAMETER	LENGTH	l _e
WSW12 AND WSW18	WSW-HSR7/8x24KT	7/8"	24"	22"
	WSW-HSR7/8x36KT	7/8"	36"	34"
WSW24	WSW-HSR1x24KT	1"	24"	22"
	WSW-HSR1x36KT	1"	36"	34"

WSW ANCHOR BOLT EXTENSION 4



WSW ANCHOR BOLT TEMPLATES 6



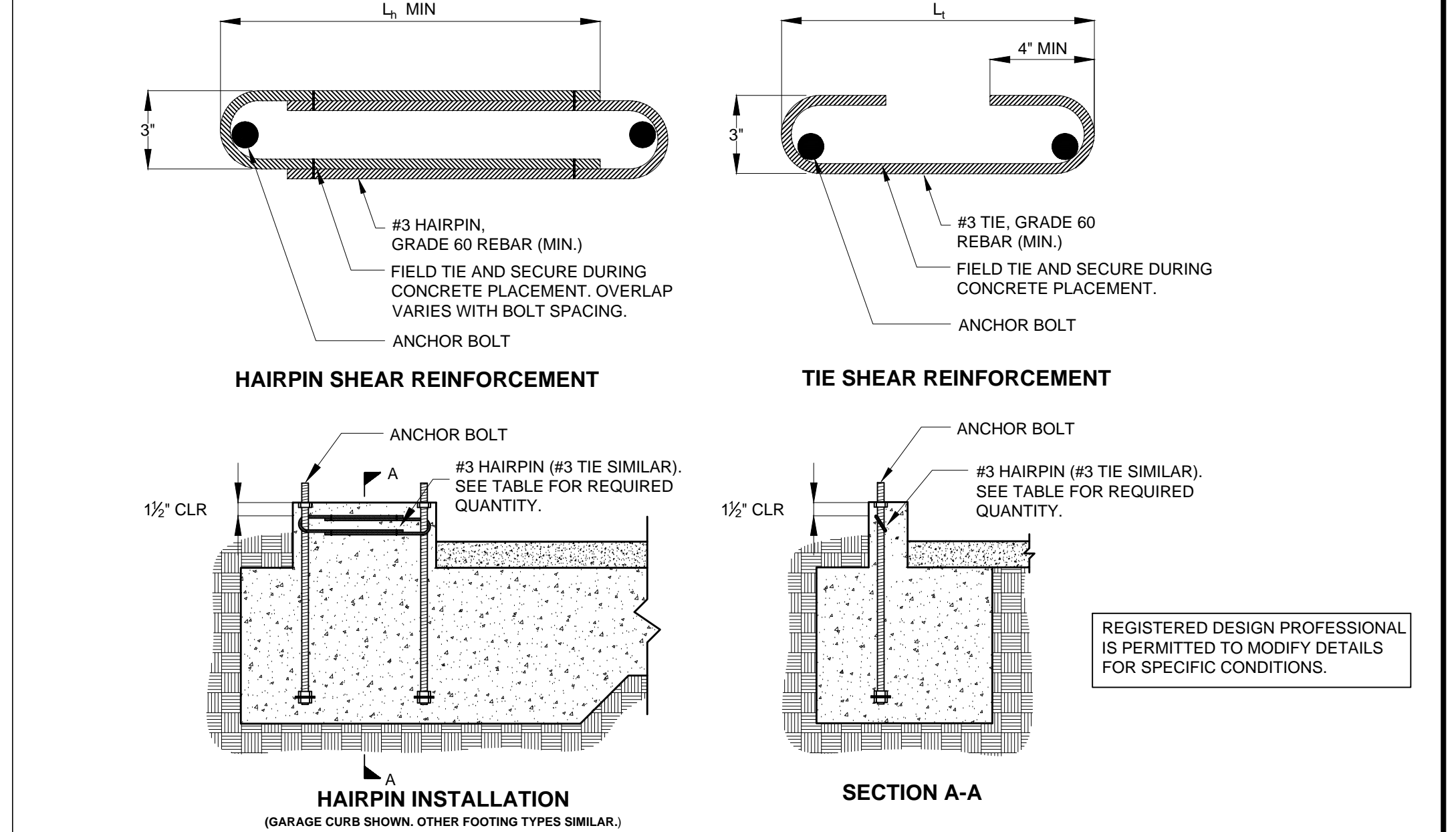
DESIGN CRITERIA	CONCRETE CONDITION	ANCHOR STRENGTH	WSW ANCHORAGE SOLUTIONS FOR 2500 PSI CONCRETE					
			WSW-AB7/8 ANCHOR BOLT			WSW-AB1 ANCHOR BOLT		
			ASD ALLOWABLE TENSION (lb.)	W (in.)	d _b (in.)	ASD ALLOWABLE TENSION (lb.)	W (in.)	d _b (in.)
SEISMIC	CRACKED	STANDARD	11,900	27	9	16,100	33	11
		HIGH STRENGTH	13,100	29	10	17,100	35	12
		HIGH STRENGTH	24,900	43	15	33,000	51	17
	UNCRAKED	STANDARD	12,500	24	8	15,700	28	10
		HIGH STRENGTH	13,100	25	9	17,100	30	10
		HIGH STRENGTH	25,300	38	13	32,300	44	15
WIND	CRACKED	STANDARD	5,100	14	6	6,200	16	6
		HIGH STRENGTH	8,700	20	7	11,400	24	8
		HIGH STRENGTH	13,100	27	9	17,100	32	11
	UNCRAKED	STANDARD	15,900	30	10	21,100	36	12
		HIGH STRENGTH	18,400	33	11	27,300	42	14
		HIGH STRENGTH	23,100	38	13	31,800	46	16

NOTES:
 1. ANCHORAGE DESIGNS CONFORM TO ACI 318-11 APPENDIX D AND ACI 318-14 WITH NO SUPPLEMENTARY REINFORCEMENT FOR CRACKED OR UNCRACKED CONCRETE AS NOTED.
 2. ANCHOR STRENGTH INDICATES REQUIRED GRADE OF WSW-AB ANCHOR BOLT. STANDARD (ASTM F1554 GRADE 36) OR HIGH STRENGTH (HS) (ASTM A449).
 3. SEISMIC INDICATES SEISMIC DESIGN CATEGORY C - F. DETACHED 1 AND 2 FAMILY DWELLINGS IN SDC C MAY USE WIND ANCHORAGE SOLUTIONS. SEISMIC ANCHORAGE DESIGNS CONFORM TO ACI 318-11 SECTION D.3.4.3 AND ACI 318-14 SECTION 17.2.3.4.3.
 4. WIND INCLUDES SEISMIC DESIGN CATEGORY A AND B AND DETACHED 1 AND 2 FAMILY DWELLINGS IN SDC C.
 5. FOUNDATION DIMENSIONS ARE FOR ANCHORAGE ONLY. FOUNDATION DESIGN (SIZE AND REINFORCEMENT) BY OTHERS. THE REGISTERED DESIGN PROFESSIONAL MAY SPECIFY ALTERNATE EMBEDMENT, FOOTING SIZE OR ANCHOR BOLT.
 6. REFER TO 1WSW1 FOR d_b.

STRONG-WALL® WOOD SHEARWALL TENSION ANCHORAGE SCHEDULE 2,500, 3,000 AND 4,500 PSI 2

DESIGN CRITERIA	CONCRETE CONDITION	ANCHOR STRENGTH	WSW ANCHORAGE SOLUTIONS FOR 3000 PSI CONCRETE					
			WSW-AB7/8 ANCHOR BOLT			WSW-AB1 ANCHOR BOLT		
			ASD ALLOWABLE TENSION (lb.)	W (in.)	d _b (in.)	ASD ALLOWABLE TENSION (lb.)	W (in.)	d _b (in.)
SEISMIC	CRACKED	STANDARD	12,300	26	9	16,000	31	11
		HIGH STRENGTH	13,100	28	10	17,100	33	11
		HIGH STRENGTH	25,200	41	14	32,700	48	16
	UNCRAKED	STANDARD	12,000	22	8	16,300	27	9
		HIGH STRENGTH	13,100	24	8	17,100	28	10
		HIGH STRENGTH	25,300	36	12	32,700	42	14
WIND	CRACKED	STANDARD	5,000	13	6	5,600	14	6
		HIGH STRENGTH	8,800	19	7	10,200	21	7
		HIGH STRENGTH	13,100	25	9	17,100	30	10
	UNCRAKED	STANDARD	15,700	28	10	20,100	33	11
		HIGH STRENGTH	19,200	32	11	25,300	38	13
		HIGH STRENGTH	23,200	36	12	32,300	44	15

DESIGN CRITERIA	CONCRETE CONDITION	ANCHOR STRENGTH	WSW ANCHORAGE SOLUTIONS FOR 4500 PSI CONCRETE					
			WSW-AB7/8 ANCHOR BOLT			WSW-AB1 ANCHOR BOLT		
			ASD ALLOWABLE TENSION (lb.)	W (in.)	d _b (in.)	ASD ALLOWABLE TENSION (lb.)	W (in.)	d _b (in.)
SEISMIC	CRACKED	STANDARD	12,600	23	8	16,000	27	9
		HIGH STRENGTH	13,100	24	8	17,100	29	10
		HIGH STRENGTH	24,800	36	12	32,100	42	14
	UNCRAKED	STANDARD	27,100	38	13	35,300	45	15
		HIGH STRENGTH	12,700	20	7	15,700	23	8
		HIGH STRENGTH	13,100	21	7	17,100	25	9
WIND	CRACKED	STANDARD	24,600	31	11	32,500	37	13
		HIGH STRENGTH	27,100	34	12	35,300	39	13
		HIGH STRENGTH	5,400	12	6	6,800	14	6
	UNCRAKED	STANDARD	8,300	16	6	11,600	20	7
		HIGH STRENGTH	13,100	22	8	17,100	26	9
		HIGH STRENGTH	15,300	24	8	21,400	30	10



MODEL	L _s OR L _e (in.)	STRONG-WALL® WOOD SHEARWALL SHEAR ANCHORAGE					
		SEISMIC ³		WIND ⁴			
		SHEAR REINFORCEMENT	MINIMUM CURB/STEMWALL WIDTH (in.)	SHEAR REINFORCEMENT	MINIMUM CURB/STEMWALL WIDTH (in.)		
WSW12	10%	(1) #3 HAIRPIN	8"	SEE NOTE 6	6	1,035	740
WSW18	15	(1) #3 HAIRPIN	8"	(1) #3 HAIRPIN	6	HAIRPIN REINFORCEMENT ACHIEVES MAXIMUM ALLOWABLE SHEAR LOAD OF THE WSW	
WSW24	19	(2) #3 HAIRPINS	8"	(1) #3 HAIRPIN	6		

NOTES:
 1. SHEAR ANCHORAGE DESIGNS CONFORM TO ACI 318-11 AND ACI 318-14 AND ASSUME MINIMUM 2,500 PSI CONCRETE.
 2. SHEAR REINFORCEMENT IS NOT REQUIRED FOR INTERIOR FOUNDATION APPLICATIONS (PANEL INSTALLED AWAY FROM EDGE OF CONCRETE), OR BRACED WALL PANEL APPLICATIONS.
 3. SEISMIC INDICATES SEISMIC DESIGN CATEGORY C THROUGH F. DETACHED 1 AND 2 FAMILY DWELLINGS IN SDC C MAY USE WIND ANCHORAGE SOLUTIONS.
 4. WIND INCLUDES SEISMIC DESIGN CATEGORY A AND B AND DETACHED 1 AND 2 FAMILY DWELLINGS IN SDC C.
 5. WHERE NOTED, MINIMUM CURB/STEMWALL WIDTH IS 6 INCHES WHEN STANDARD STRENGTH ANCHOR BOLT IS USED.
 6. USE (1) #3 TIE FOR WSW12 WHEN PANEL DESIGN SHEAR FORCE EXCEEDS TABULATED ANCHORAGE ALLOWABLE SHEAR LOAD.
 7. #4 GRADE 40 SHEAR REINFORCEMENT MAY BE SUBSTITUTED FOR WSW SHEAR ANCHORAGE SOLUTIONS.

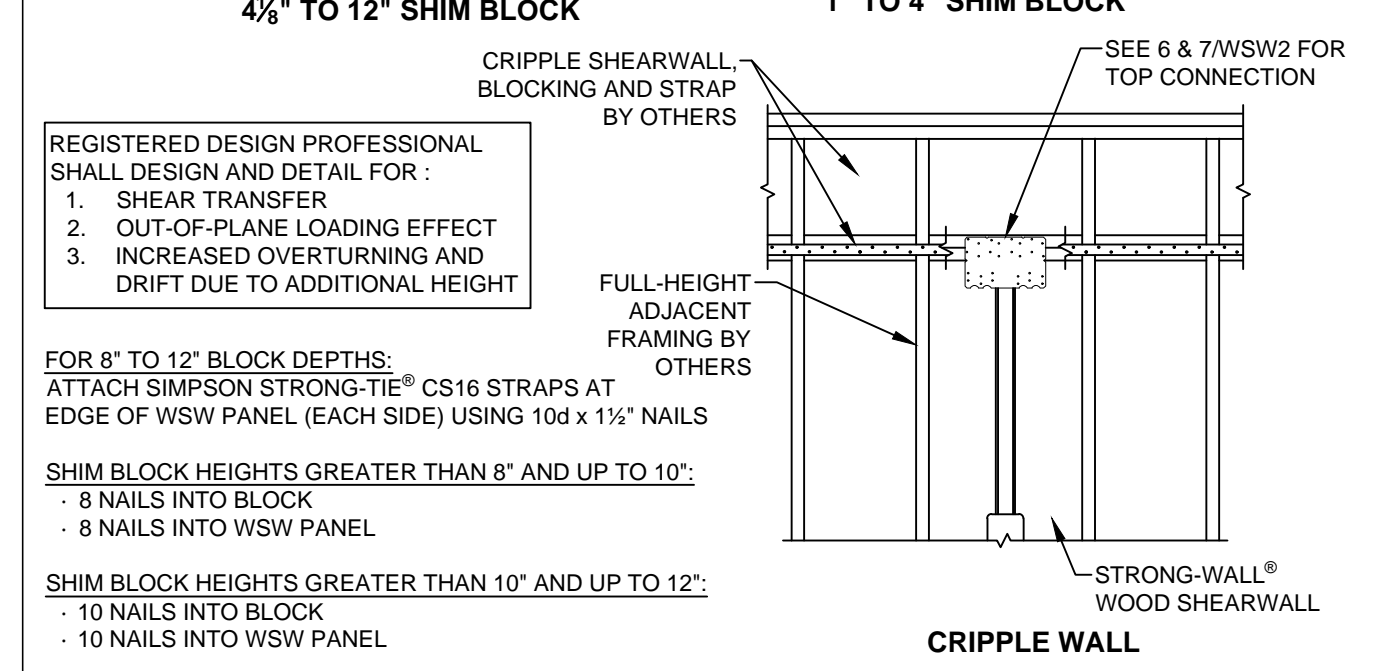
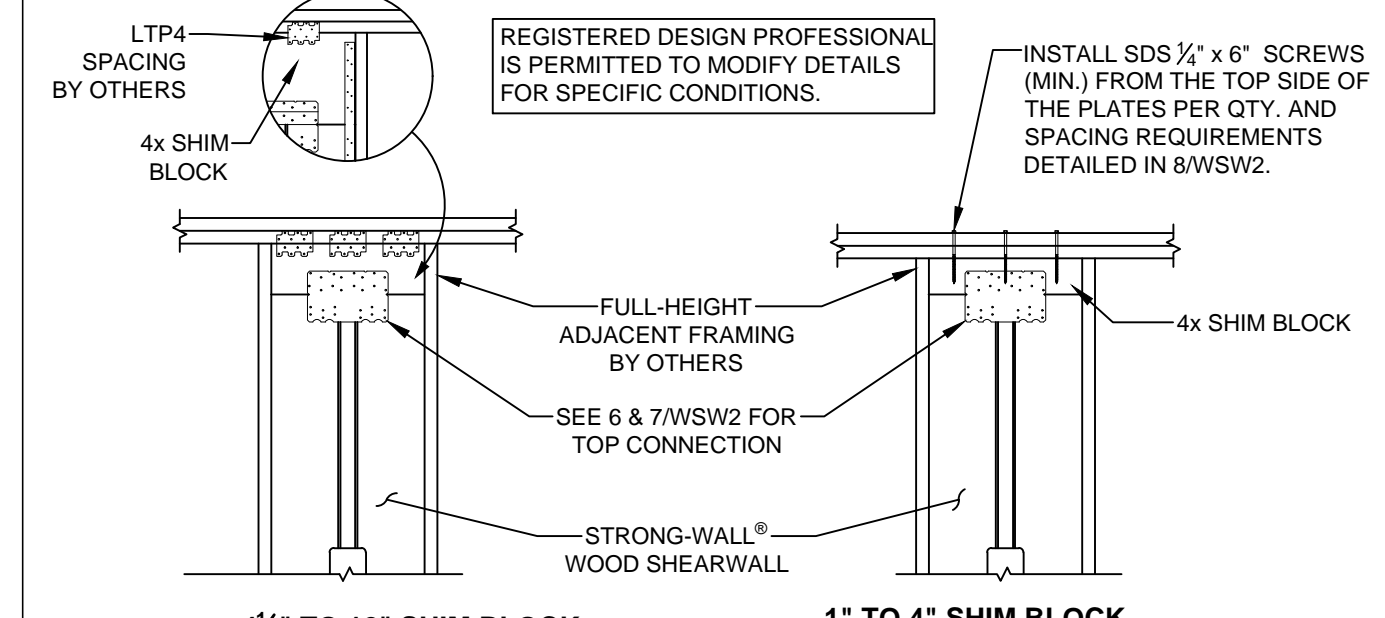
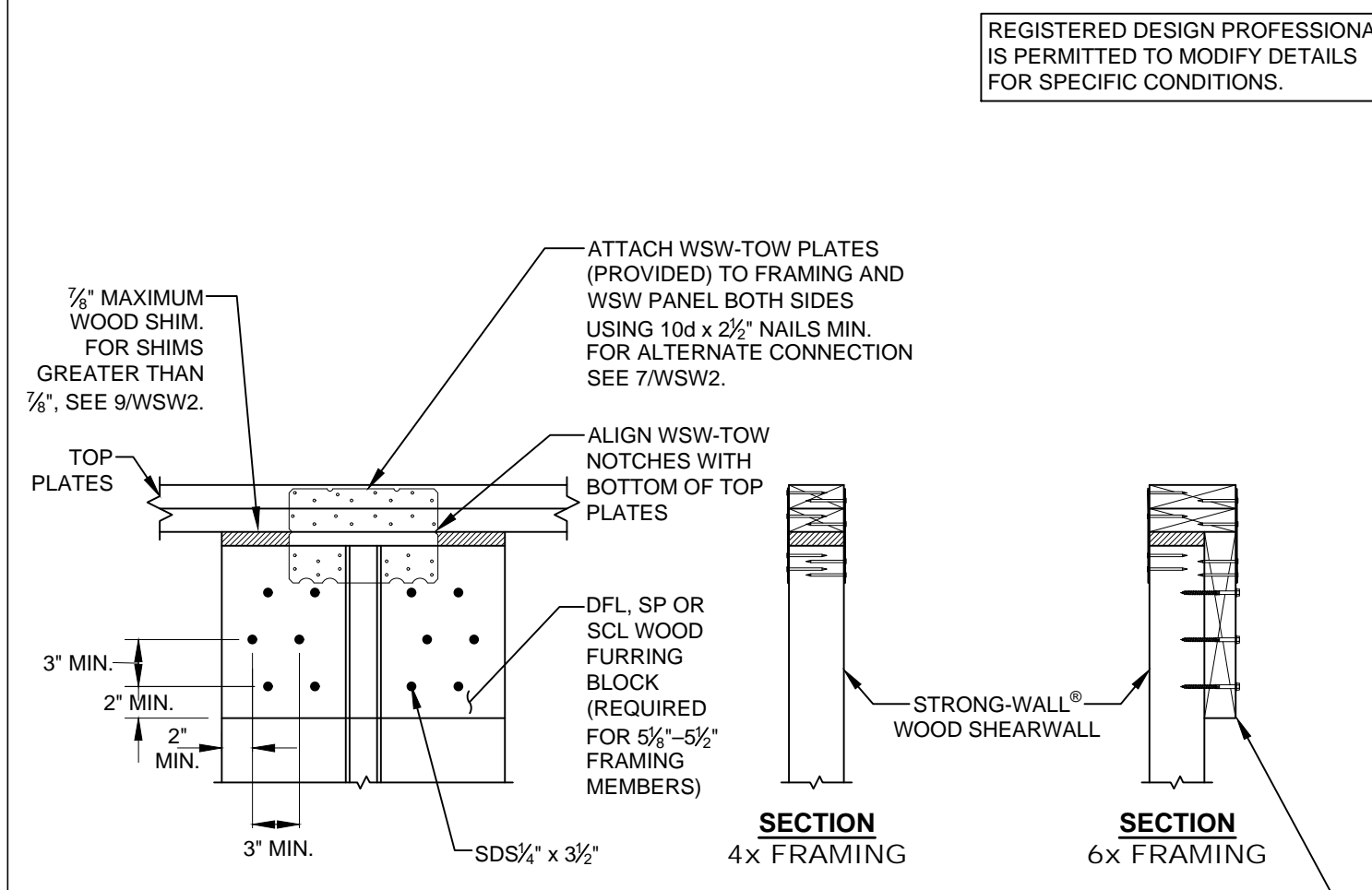
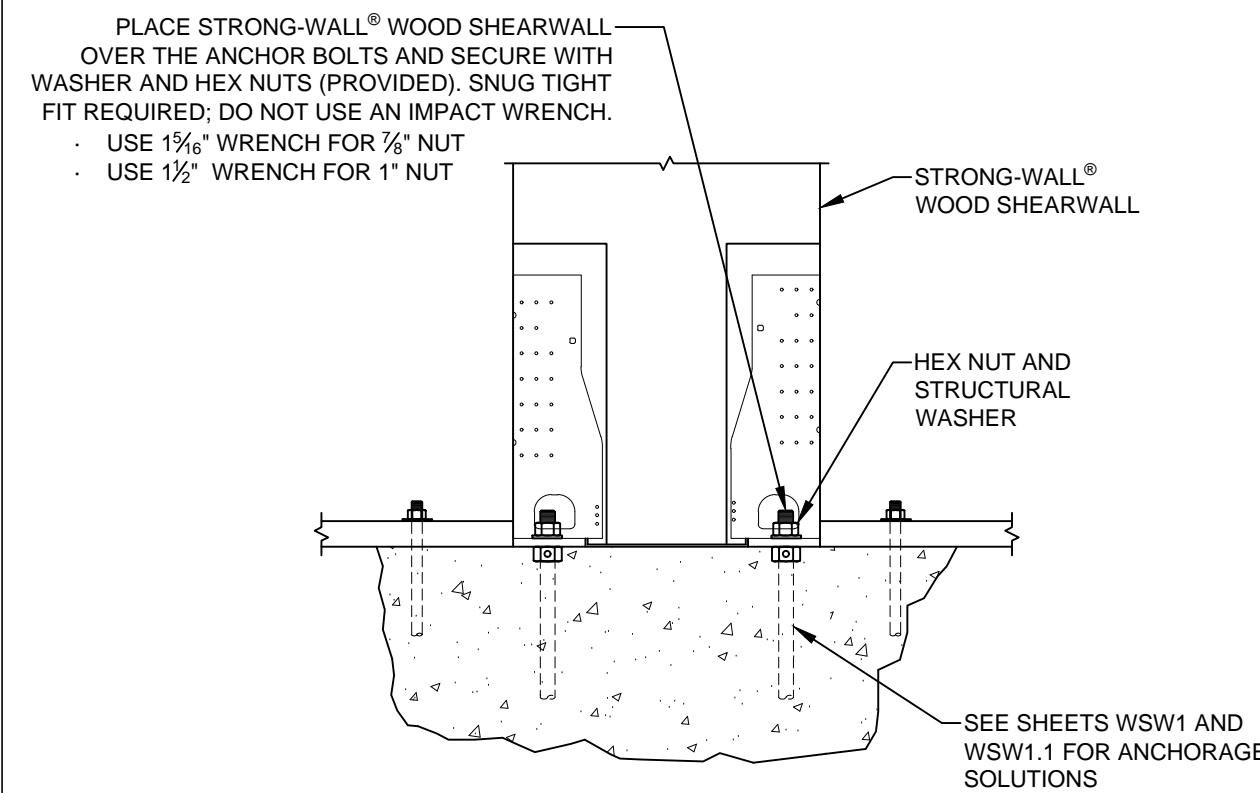
STRONG-WALL® WSW SHEAR ANCHORAGE SCHEDULE AND DETAILS 5

STRONG-WALL® WOOD SHEARWALL MODELS

MODEL NO.	W (in.)	H (in.)	ANCHOR BOLTS QUANTITY	ANCHOR BOLTS DIA. (in.)	TOTAL WALL WEIGHT (lb.)
WSW12x7	12	78	2	7/8	100
WSW18x7	18	78	2	7/8	145
WSW12x7.5	12	85 1/2	2	7/8	110
WSW18x7.5	18	85 1/2	2	7/8	155
WSW12x8	12	93 1/4	2	7/8	115
WSW18x8	18	93 1/4	2	7/8	165
WSW24x8	24	93 1/4	2	1	225
WSW12x9	12	105 1/4	2	7/8	130
WSW18x9	18	105 1/4	2	7/8	185
WSW24x9	24	105 1/4	2	1	245
WSW12x10	12	117 1/4	2	7/8	140
WSW18x10	18	117 1/4	2	7/8	205
WSW24x10	24	117 1/4	2	1	270
WSW12x11	12	129 1/4	2	7/8	150
WSW18x11	18	129 1/4	2	7/8	220
WSW24x11	24	129 1/4	2	1	295
WSW12x12	12	141 1/4	2	7/8	165
WSW18x12	18	141 1/4	2	7/8	240
WSW24x12	24	141 1/4	2	1	320
WSW18x13	18	153 1/4	2	7/8	255
WSW24x13	24	153 1/4	2	1	345
WSW24x14	24	168	2	1	375
WSW24x16	24	192	2	1	425
WSW18x20	18	240	2	7/8	385
WSW24x20	24	240	2	1	520

- NOTES:
 1. FOR HEIGHTS NOT LISTED, ORDER THE NEXT TALLEST PANEL AND TRIM TO FIT. MINIMUM TRIMMED HEIGHT FOR ALL PANELS IS 74".
 2. ALL PANELS COME WITH TWO PRE-ATTACHED HOLD-DOWNS, TWO STANDARD HEX NUTS, TWO STRUCTURAL WASHERS, TWO WSW-TOW PLATES AND INSTALLATION INSTRUCTIONS.
 3. ALL PANELS ARE 3/4" THICK.

REGISTERED DESIGN PROFESSIONAL IS PERMITTED TO MODIFY DETAILS FOR SPECIFIC CONDITIONS.

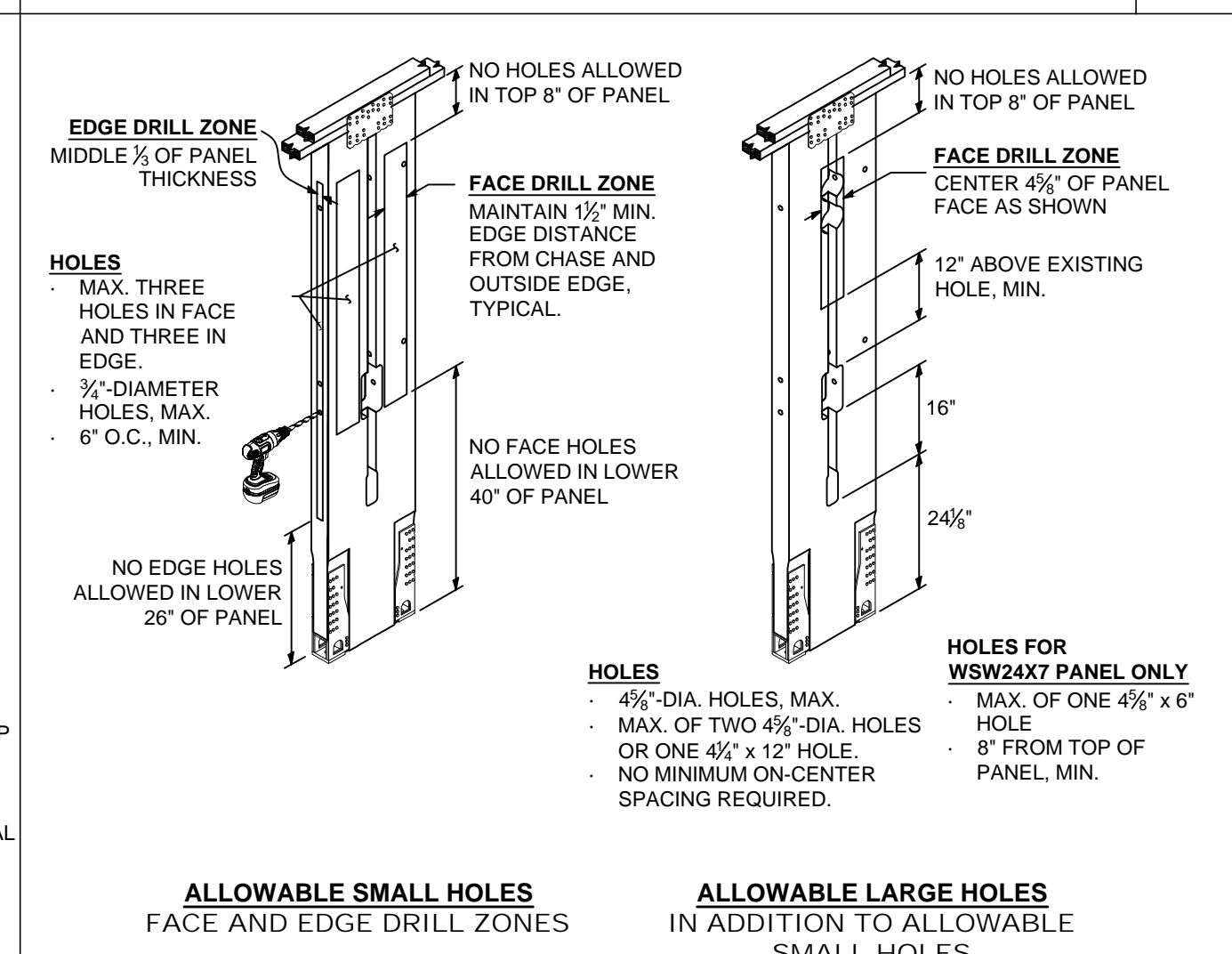
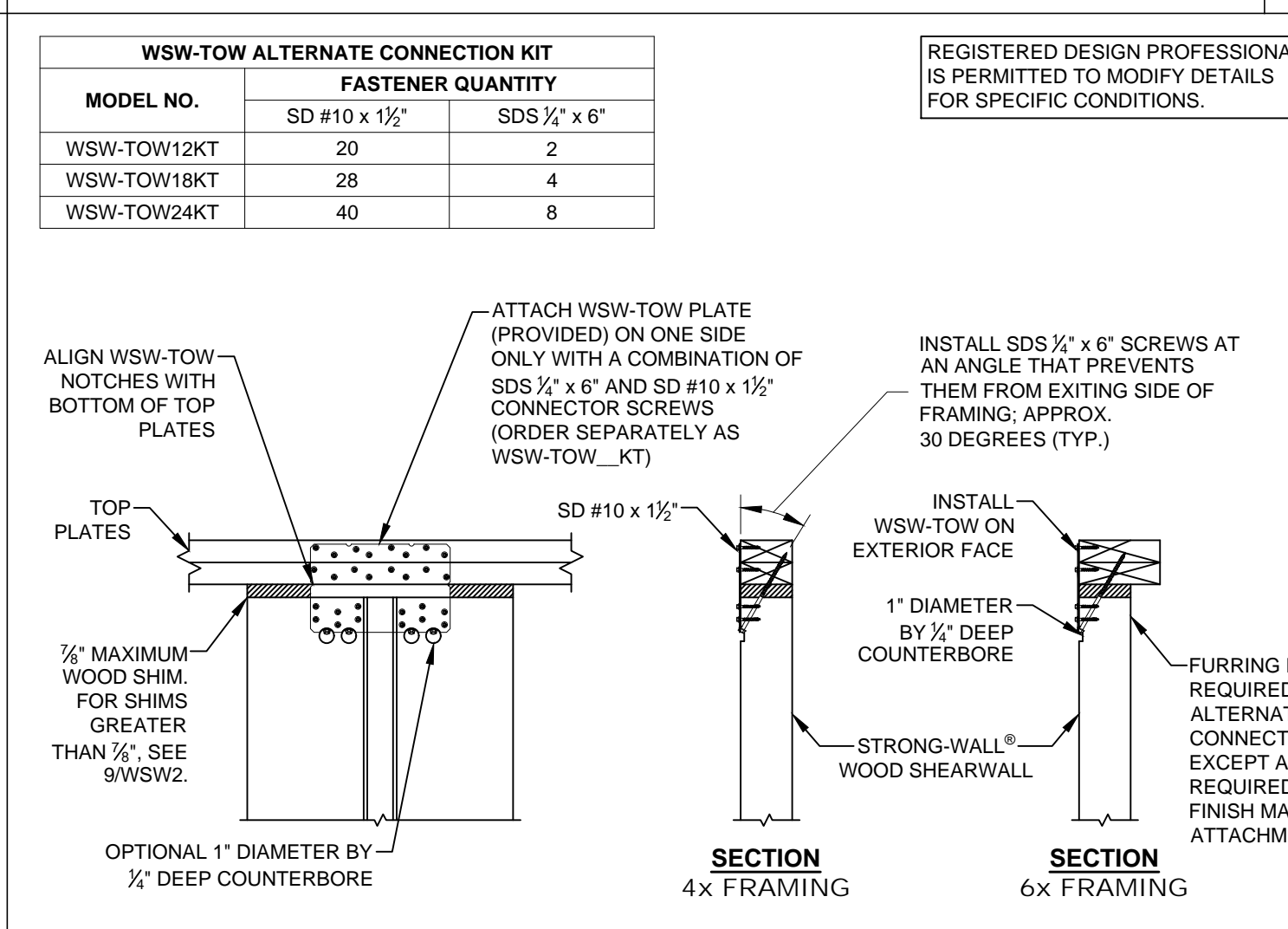
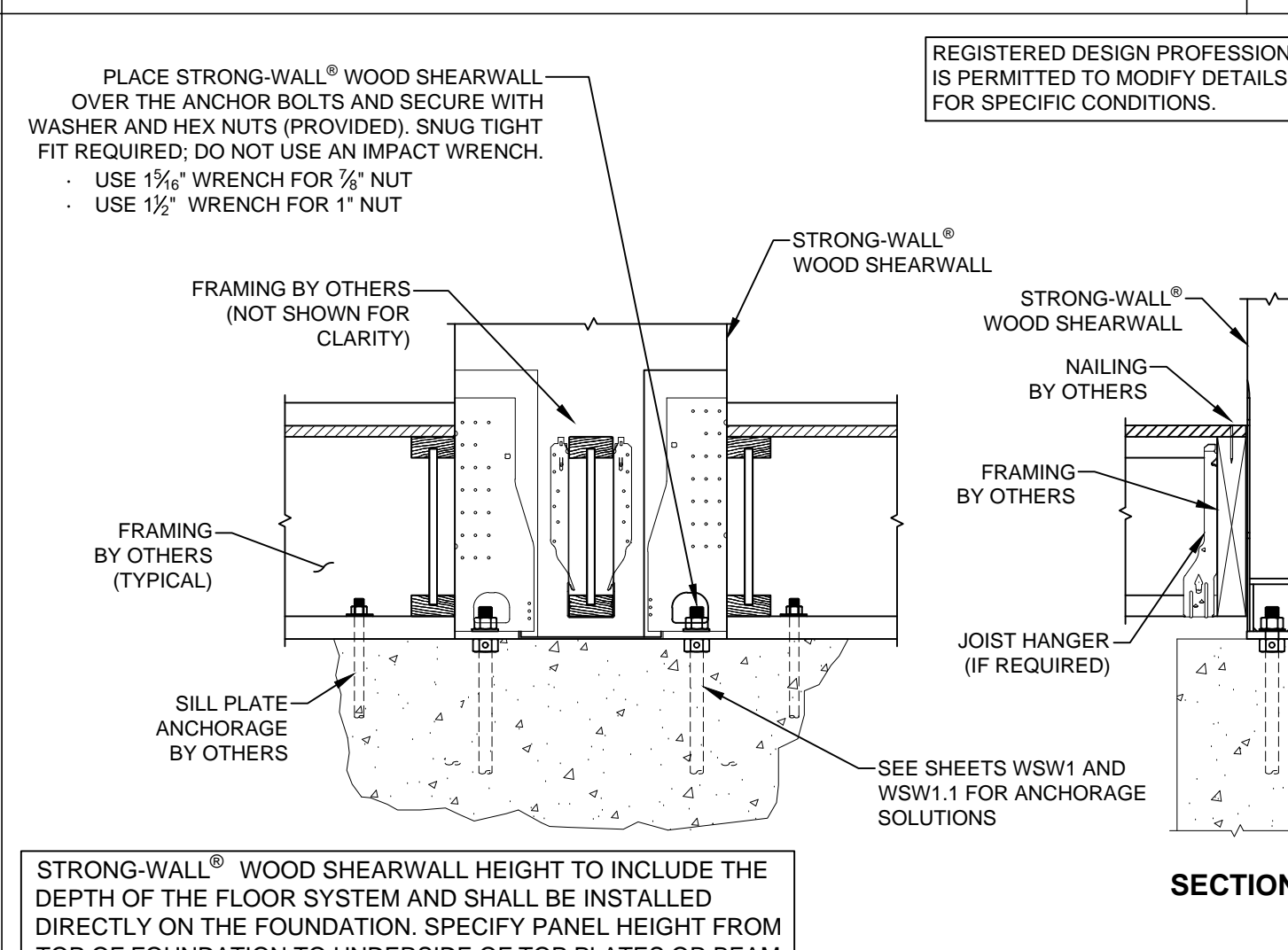
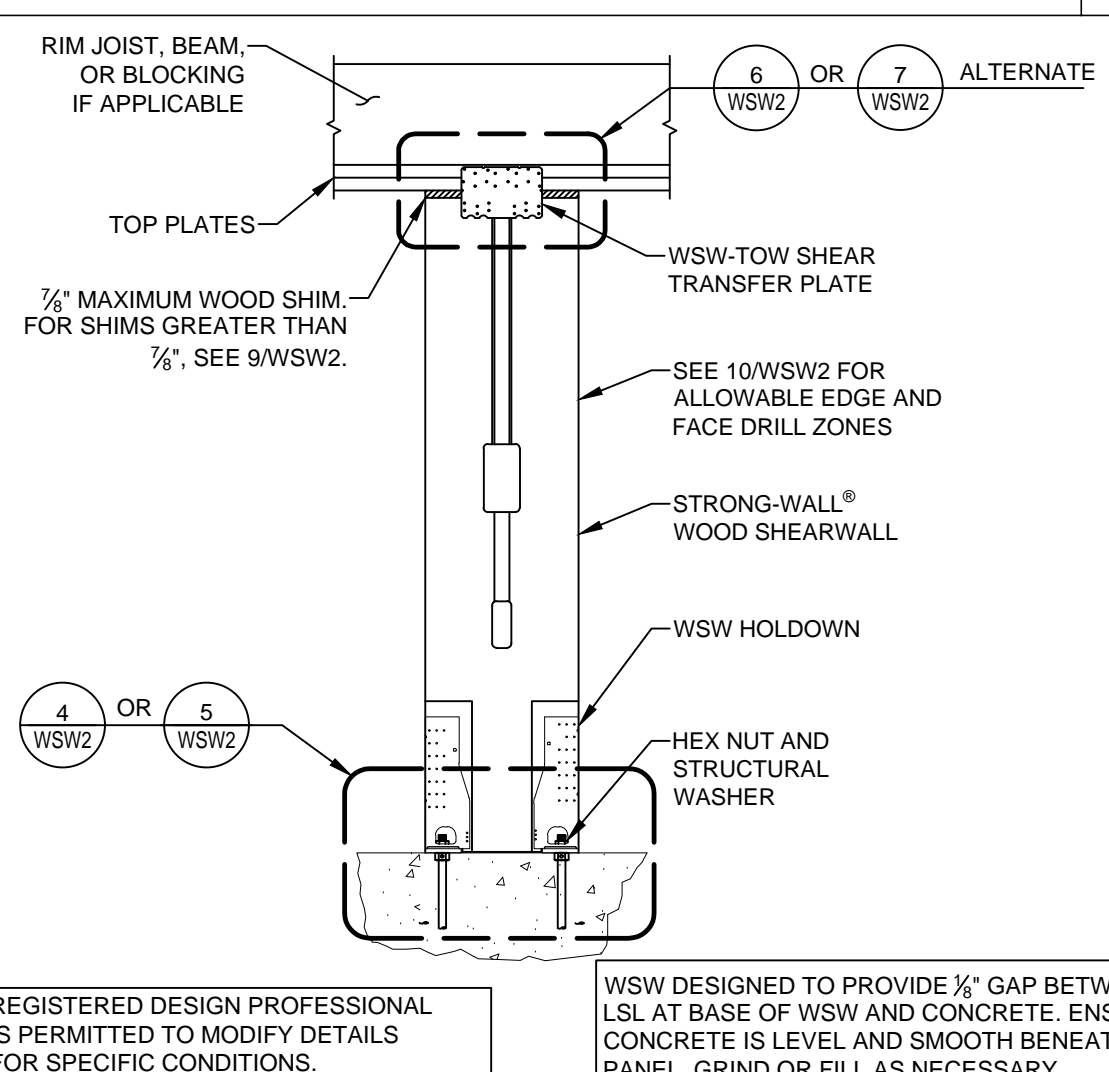


STRONG-WALL® WSW MODELS

1 STANDARD INSTALLATION BASE CONNECTION

4 STANDARD TOP CONNECTION

6 TOP OF WALL HEIGHT ADJUSTMENTS

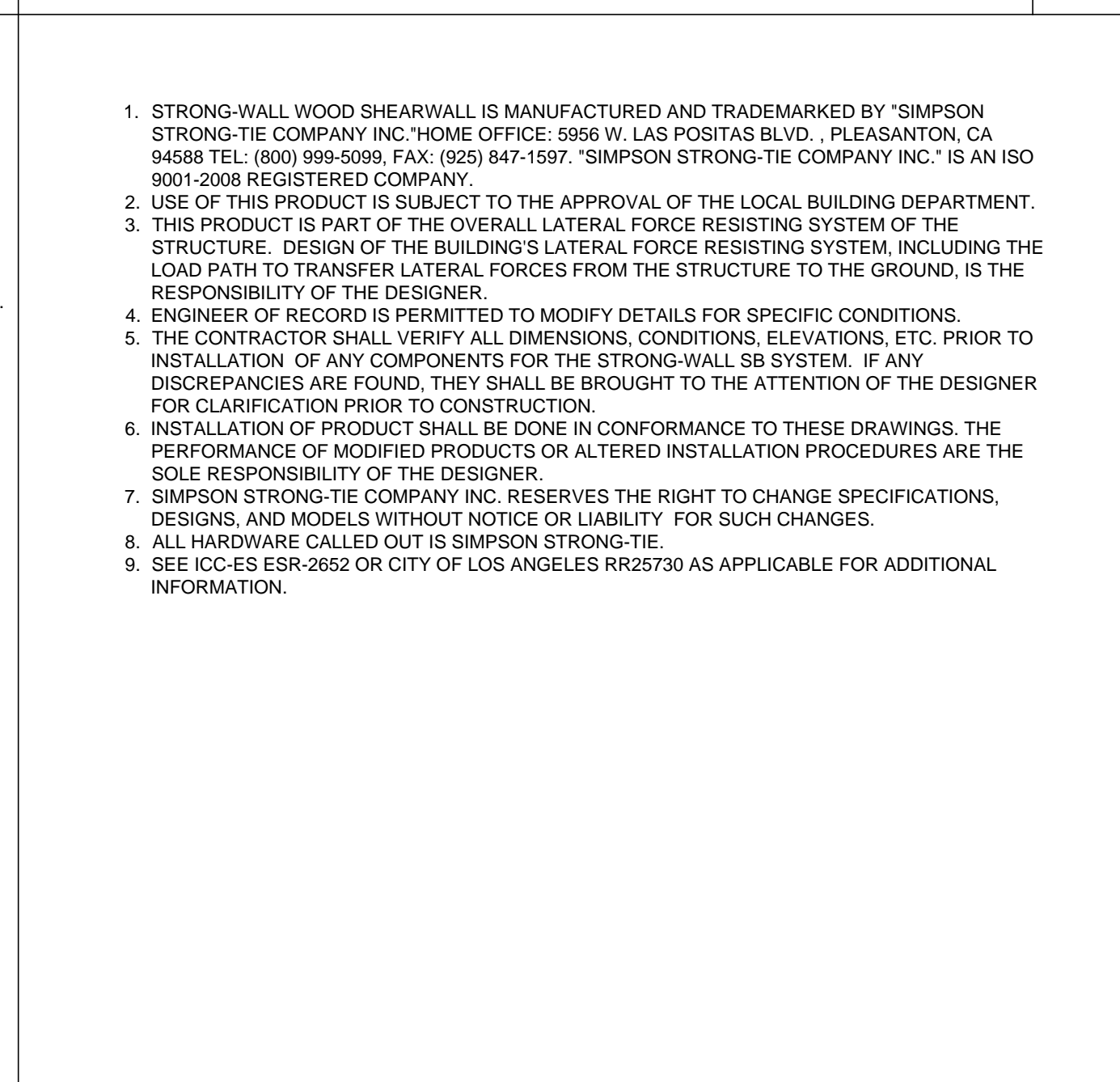
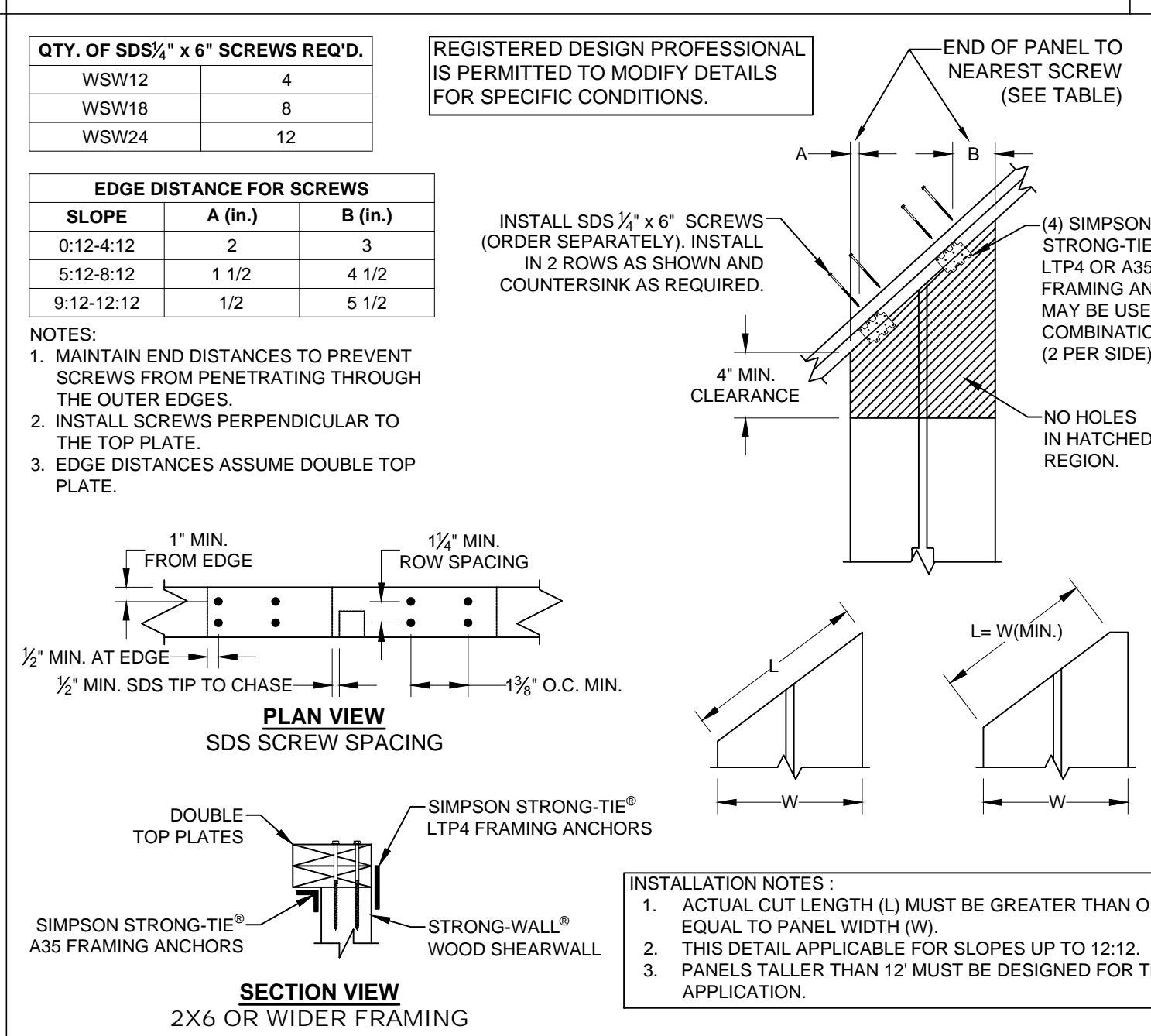
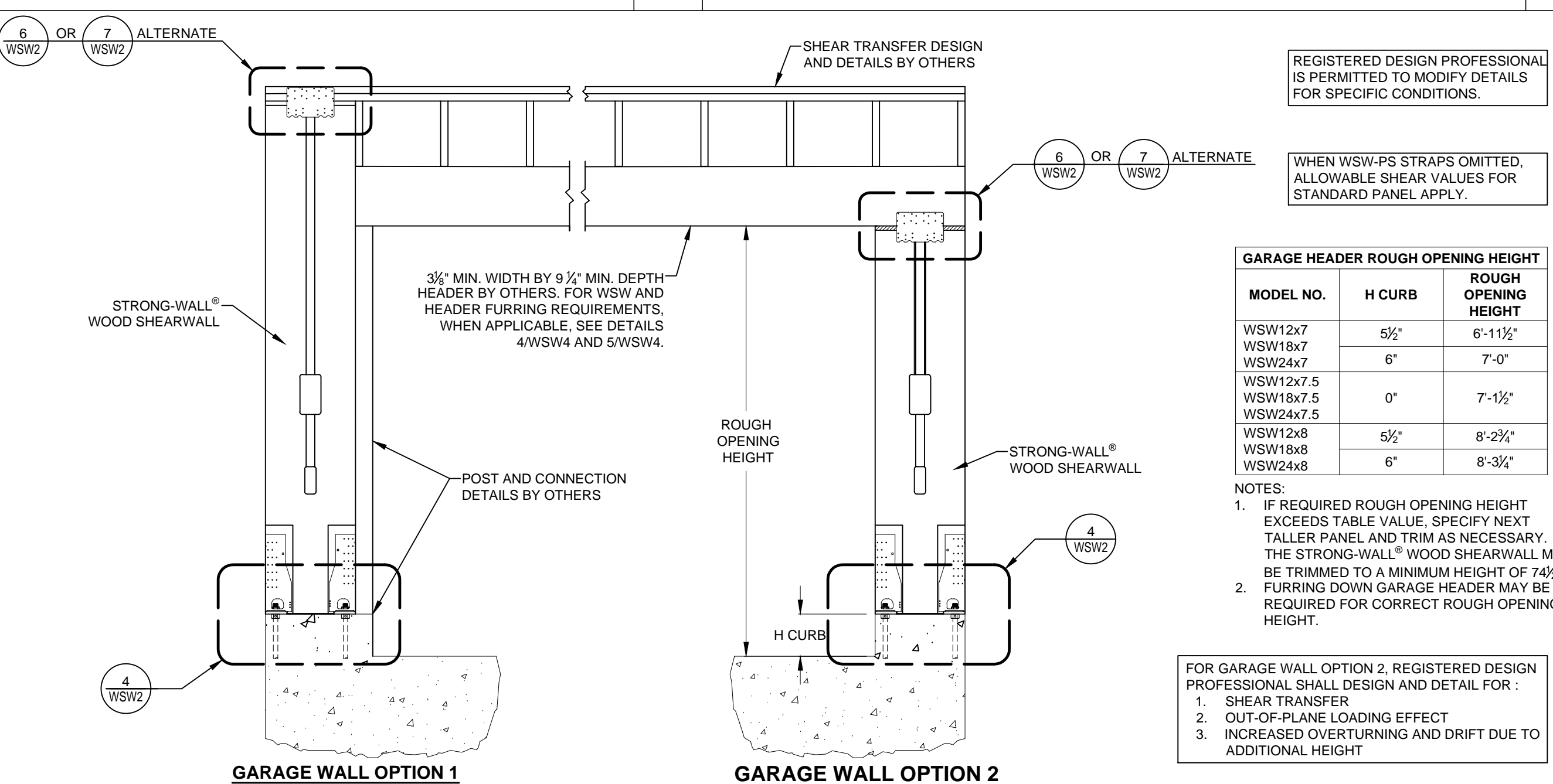


2 SINGLE STORY WSW ON CONCRETE

5 WOOD FLOOR SYSTEM BASE CONNECTION

7 ALTERNATE TOP CONNECTION

10 TRIM ZONE AND ALLOWABLE HOLES



3 ALTERNATE WSW GARAGE FRONT OPTIONS

8 RAKE WALL

11 NOTES



- NOTES:
 1. STRONG-WALL WOOD SHEARWALL IS MANUFACTURED AND TRADEMARKED BY SIMPSON STRONG-TIE COMPANY INC. HOME OFFICE: 6965 W. LAS POSITAS BLVD., PLEASANTON, CA 94588 TEL: (800) 999-5099, FAX: (925) 847-1597. SIMPSON STRONG-TIE COMPANY INC. IS AN ISO 9001-2008 REGISTERED COMPANY.
 2. USE OF THIS PRODUCT IS SUBJECT TO THE APPROVAL OF THE LOCAL BUILDING DEPARTMENT.
 3. THIS PRODUCT IS PART OF THE OVERALL LATERAL FORCE RESISTING SYSTEM OF THE STRUCTURE. DESIGN OF THE BUILDING'S LATERAL FORCE RESISTING SYSTEM, INCLUDING THE LOAD PATH TO TRANSFER LATERAL FORCES FROM THE STRUCTURE TO THE GROUND, IS THE RESPONSIBILITY OF THE DESIGNER.
 4. ENGINEER OF RECORD IS PERMITTED TO MODIFY DETAILS FOR SPECIFIC CONDITIONS.
 5. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, CONDITIONS, ELEVATIONS, ETC. PRIOR TO INSTALLATION OF PRODUCT SHALL BE DONE IN CONFORMANCE TO THESE DRAWINGS. THE PERFORMANCE OF MODIFIED PRODUCTS OR ALTERED INSTALLATION PROCEDURES ARE THE SOLE RESPONSIBILITY OF THE DESIGNER.
 6. SIMPSON STRONG-TIE COMPANY INC. RESERVES THE RIGHT TO CHANGE SPECIFICATIONS, DESIGNS, AND MODELS WITHOUT NOTICE OR LIABILITY FOR SUCH CHANGES.
 7. ALL HARDWARE CALLED OUT IS SIMPSON STRONG-TIE.
 8. SEE ICC-ES ESR-2652 OR CITY OF LOS ANGELES RR25730 AS APPLICABLE FOR ADDITIONAL INFORMATION.

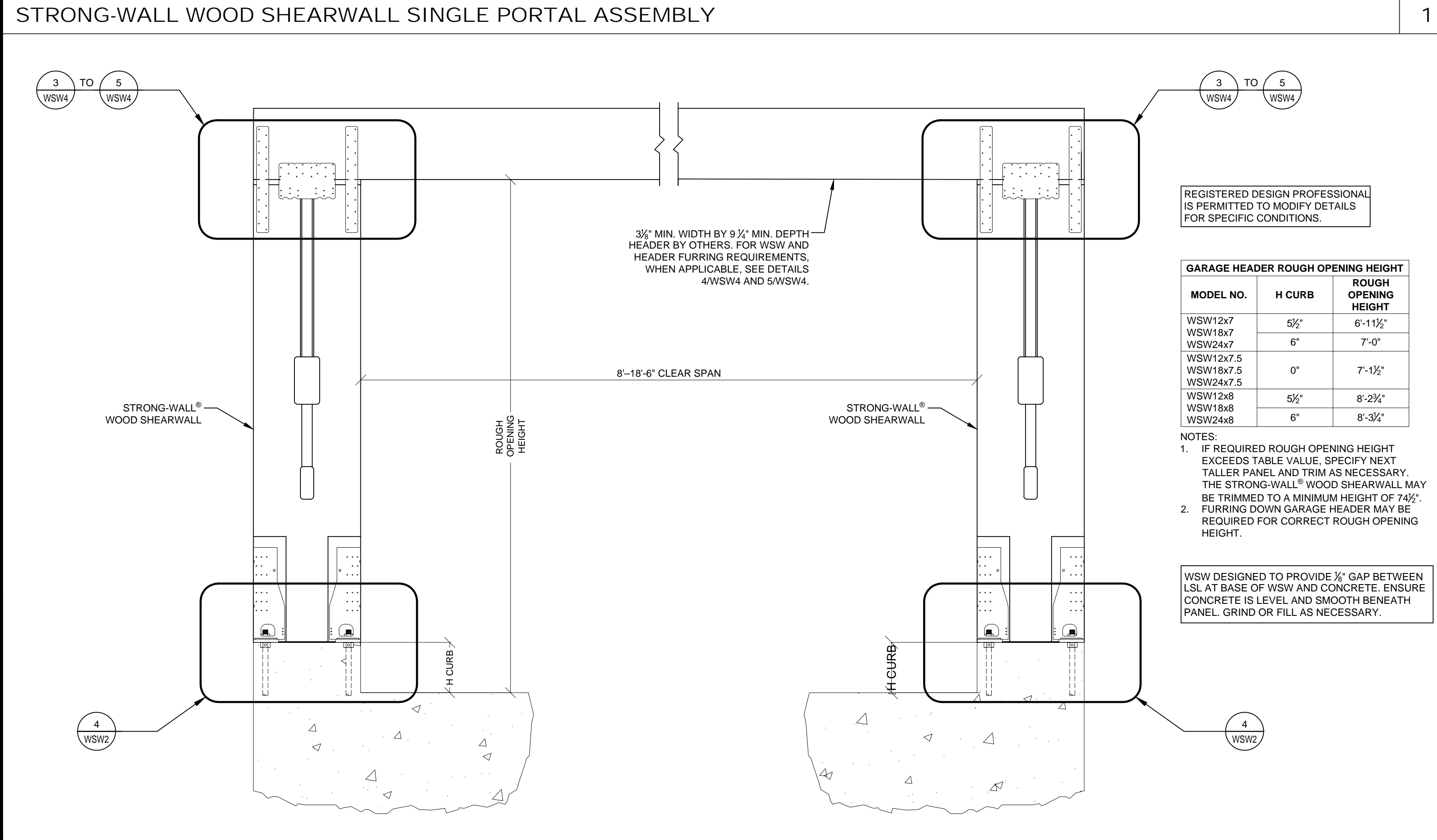
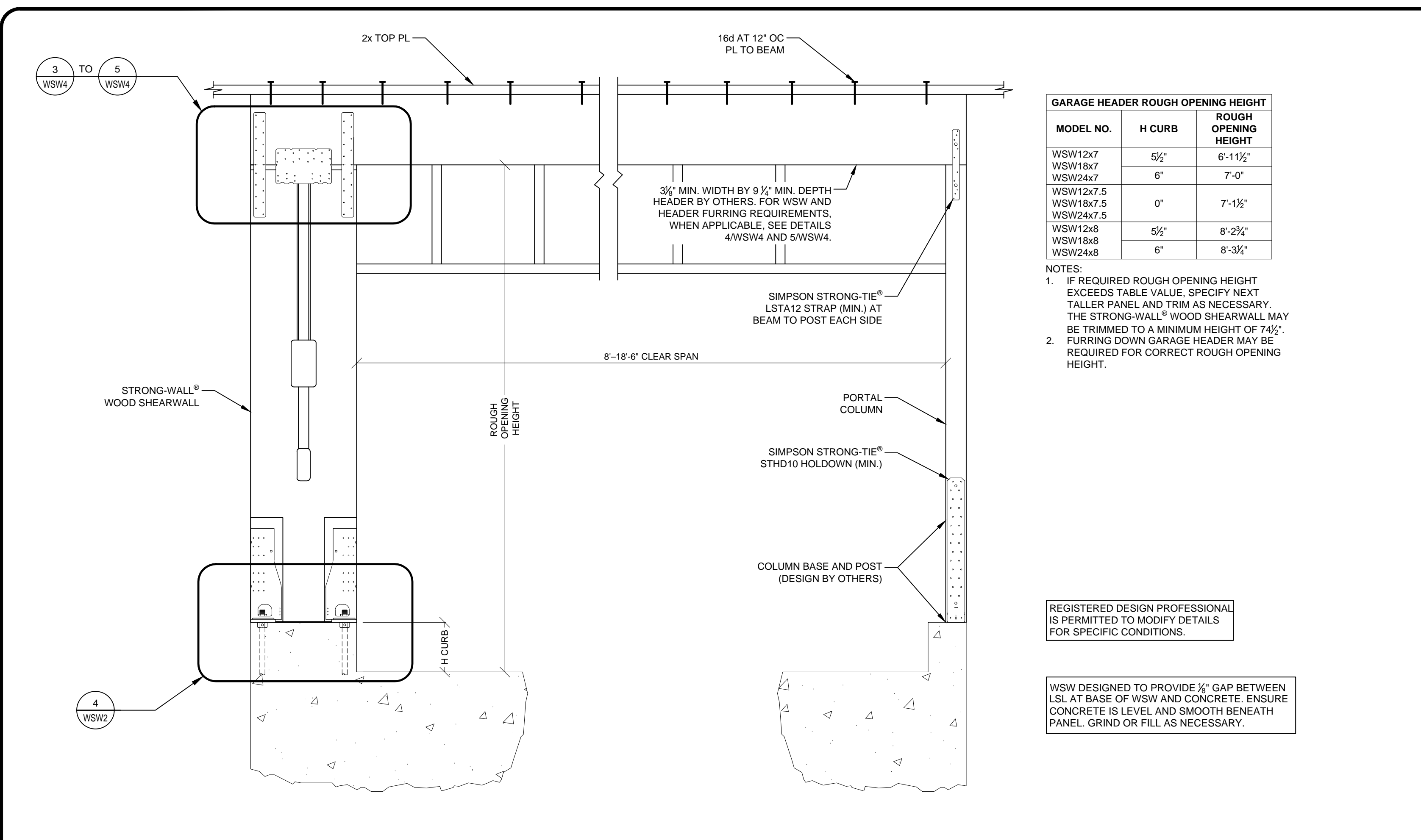
REVISIONS:

L. R. NELSON CONSULTING ENGINEERS, LLC.
 CIVIL FORENSICS PLANNING SURVEY
 6765 West Russell Rd Suite 200
 Las Vegas, Nevada 89118
 (702) 798-7978
 (702) 451-2296 FAX
 www.lrneng.com

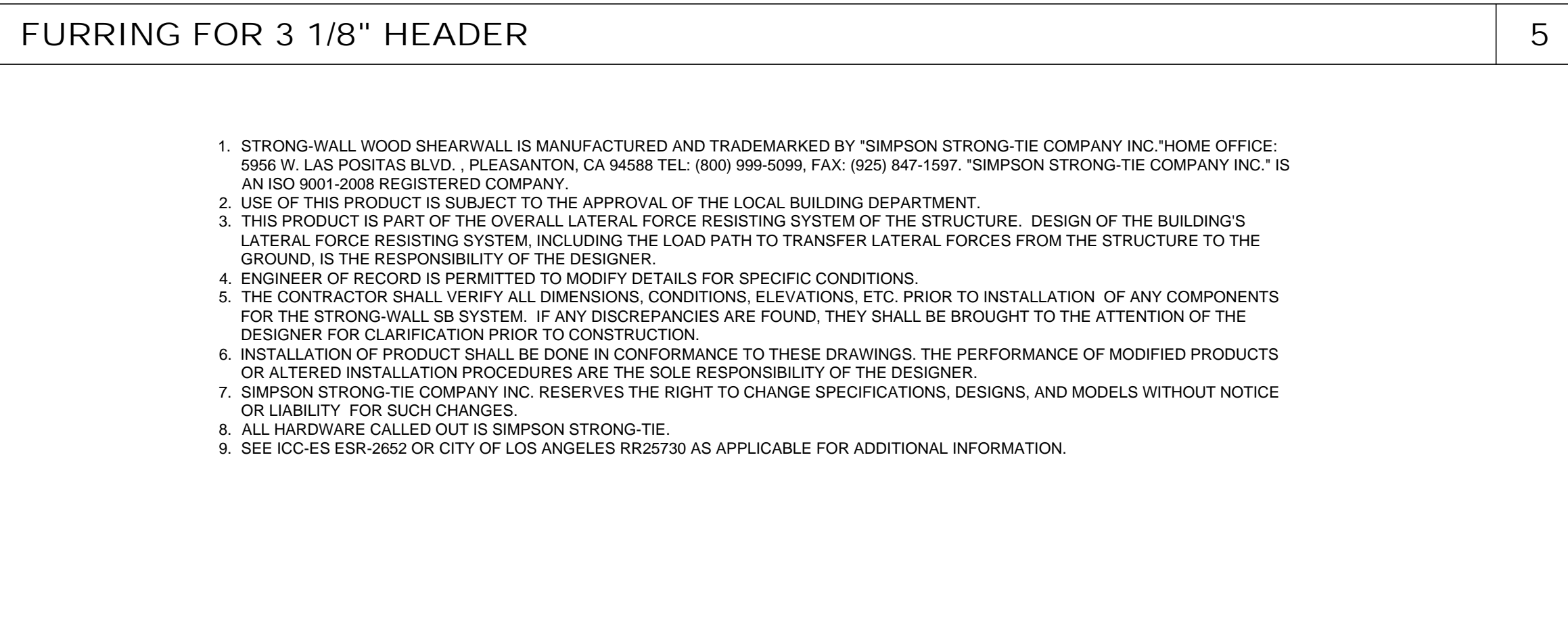
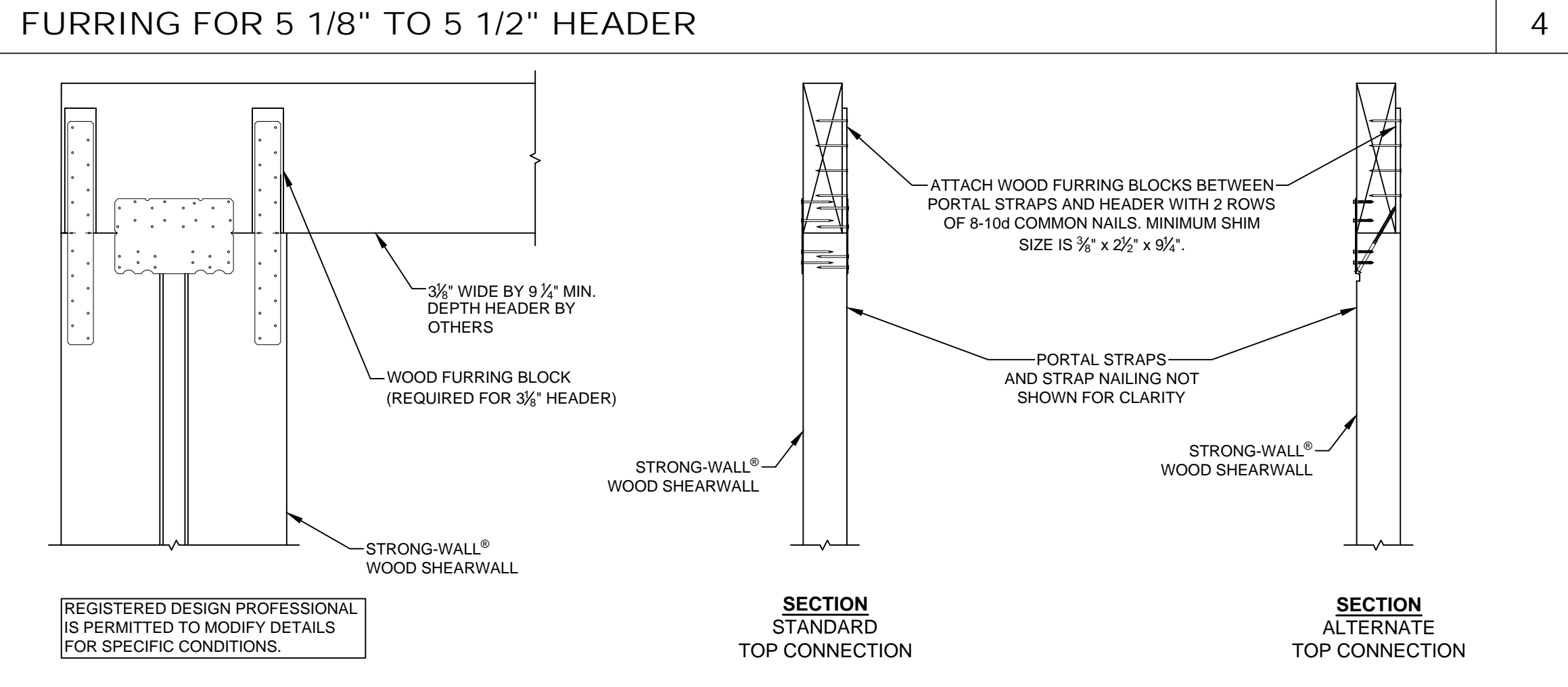
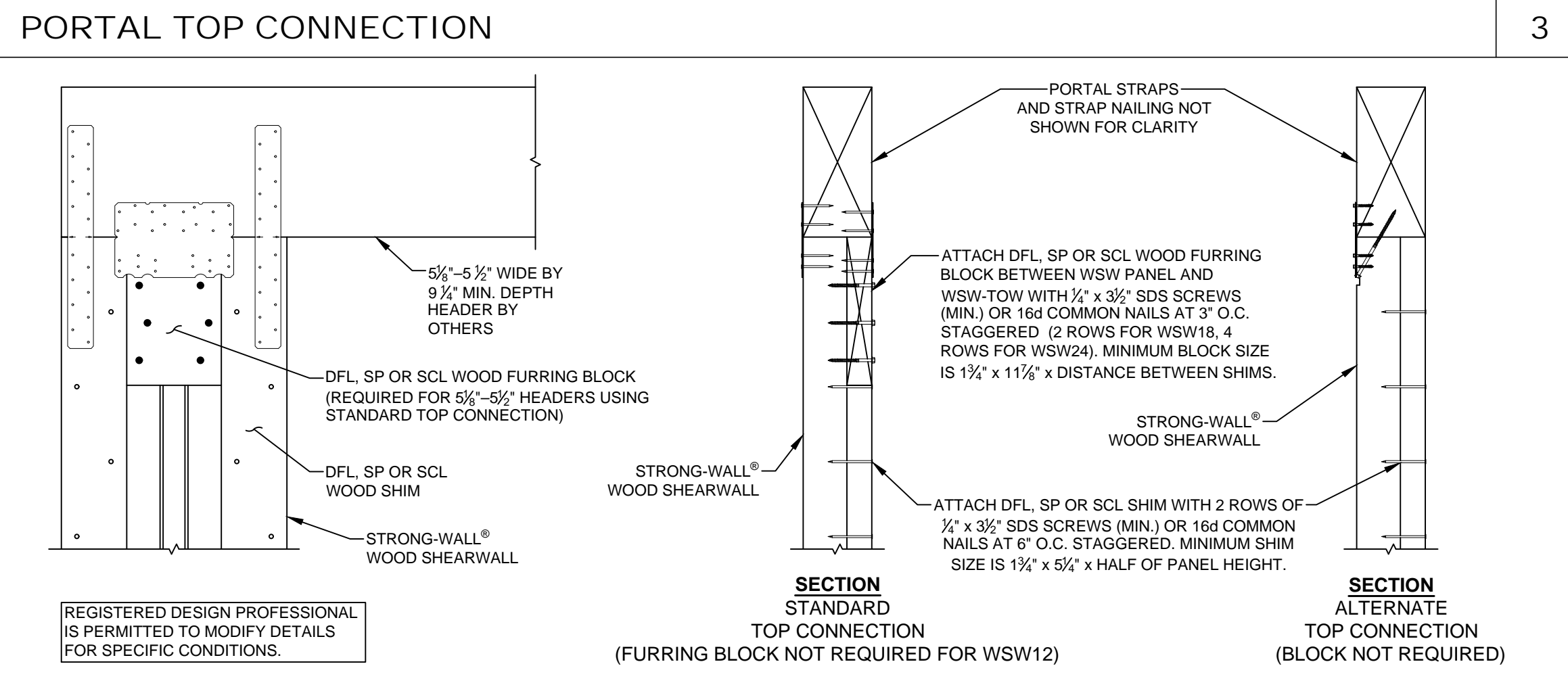
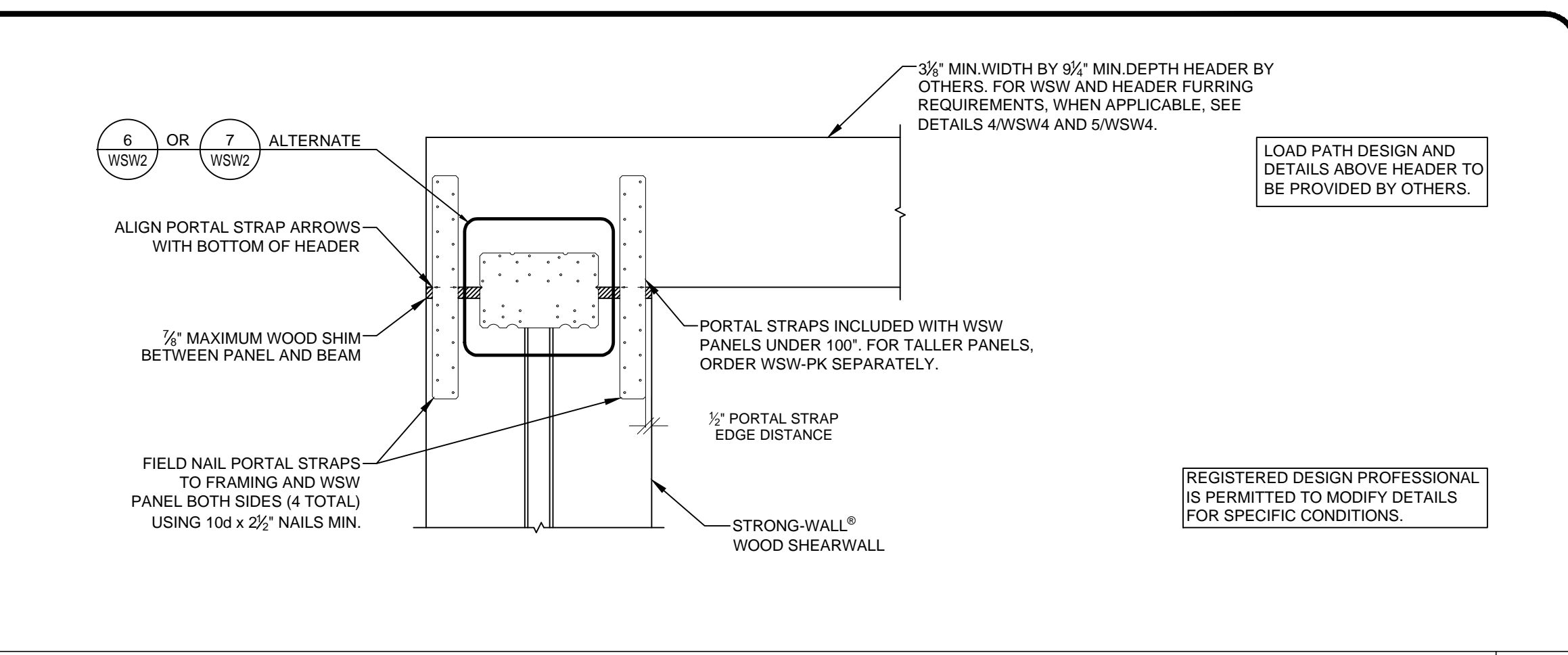
ASSURED DEVELOPMENT
 ATHENS LOT 2
 HENDERSON, NV

REGISTERED PROFESSIONAL ENGINEER STATE OF NEVADA
 SARAH CAMILLE BARENGO
 Exp. 12-31-2020
 CIVIL
 License No. 025375
 07/20/2020

WSW4



STRONG-WALL WOOD SHEARWALL DOUBLE PORTAL ASSEMBLY



NOTES

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- ENGINEER OF RECORD IS PERMITTED TO MODIFY DETAILS FOR SPECIFIC CONDITIONS.
- THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, CONDITIONS, ELEVATIONS, ETC. PRIOR TO INSTALLATION OF ANY COMPONENTS FOR THE STRONG-WALL SB SYSTEM. IF ANY DISCREPANCIES ARE FOUND, THEY SHALL BE BROUGHT TO THE ATTENTION OF THE DESIGNER FOR CLARIFICATION PRIOR TO CONSTRUCTION.
- INSTALLATION OF PRODUCT SHALL BE DONE IN CONFORMANCE TO THESE DRAWINGS. THE PERFORMANCE OF MODIFIED PRODUCTS OR ALTERED INSTALLATION PROCEDURES ARE THE SOLE RESPONSIBILITY OF THE DESIGNER.
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